# A REVIEW AND UPDATE OF THE WATER BALANCE OF LAKE VICTORIA IN EAST AFRICA 

Report to the<br>Overseas Development Administration

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## Acknowledgements

This study makes extensive use of rainfall, flow and lake level data collected in East Africa. We would like to thank the many individuals and organisations who contributed this information.

## Summary

This report describes a review and update of the hydrology of Lake Victoria in East Africa. The aim has been to update previous water balance studies of the lake and to examine the sensitivity of the water balance to possible errors or changes in each of the main components. A preliminary assessment has also been made of the impact of past and projected land use changes on lake levels.

As part of the study, a detailed evaluation has been made of a monthly water balance model developed during a previous study by the Institute of Hydrology. This model covered the period 1925 to 1978 . The simulations have been updated to the present and several aspects of the model have been simplified or improved while still retaining a satisfactory water balance. An annual model has also been developed to extend the water balance back to the start of the century and for use in the sensitivity studies of the lake water balance. This model has also been used in conjunction with a stochastic rainfall generation model to estimate the likely future trends in lake levels.

For use in future modelling studies, revised estimates have been obtained for the historical lake outflow, tributary inflow and lake rainfall series. An attempt has also been made to place the variations in lake levels this century in a regional context by comparisons with rainfall, flow and lake level records for other regions in East Africa. These analyses and the water balance models confirm the results from previous studies which have shown a firm link between the lake rainfall and variations in lake levels this century. The influence on levels of land use changes and the Owen Falls dam - constructed at the lake outlet between 1951 and 1954 - is shown to be negligible.

## Location map



Figure 1.1

## 1. Introduction

This report describes a review and update of the hydrology of Lake Victoria in East Africa. This work has been performed within the framework of the 'Water Balance of African Lakes' project funded by the Overseas Development Administration. The long term objectives of this project are described in Section 1.1 and the specific objectives of the present study are described in Section 1.2.

### 1.1 BACKGROUND

The 'Water Balance of African Lakes' study, which started in 1992, aims to develop methods for assessing the possible impacts of land use changes on lake levels in East and Central Africa. Historical records show that many lakes are subject to large fluctuations in level from year to year and it is generally accepted that these fluctuations are due primarily to natural variations in climate. However, changes in land use may also be important. This is particularly likely to be true over long timescales as vegetation and soil cover are important factors in the transformation of rainfall to runoff. Widespread changes in land use may therefore change not only the depth of runoff but also its timing. This would feed through to the overall water balance of the river catchments affected and where these flow into lakes a long term trend in lake levels may be introduced.

The principal change in land use that has taken place in East and Central Africa over the past century has been the progressive replacement of open savannah woodland with agricultural land. More recently there has been a move towards greater use of irrigation in both large and small scale schemes. From theoretical considerations and the results of experiments conducted worldwide it would be expected that a change from forest to agriculture would result in lower evaporation, higher runoff and higher lake levels. A move towards greater areas of irrigated land would tend to have the opposite effect. The impact on lake levels will depend on both the extent and nature of the changes and the importance of direct runoff in the overall water balance of the lake.

To quantify these effects requires the development of process-based models capable of representing the influence of vegetation and soil cover on runoff and the response of lake levels to variations in runoff. Such models could then be used to assess the impact of land use changes in the past on current lake levels and to estimate the impacts of any future planned or hypothetical changes. This information would be useful for assessing the long term yield and hydropower potential of the major African lakes, as well as assisting lakeside communities in planning future developments such as port facilities and irrigation and drinking water supply schemes.

The development of appropriate process-based models is one of the long term objectives of the study. It is envisaged that the results of this work will be presented in a series of reports covering the development of the models and case studies for individual lakes. Initially, two lakes have been selected for detailed study - Lake Victoria and Lake Malawi - and this report presents the preliminary results for Lake Victoria.

### 1.2 SCOPE OF WORK

The main objective of the present study has been to review and update previous work on the hydrology of Lake Victoria and to examine the sensitivity of the lake water balance to possible errors or future changes in each of the main components. As part of this work, a preliminary assessment has also been made of past and projected changes in land use in the lake catchment and the likely effects of these changes on lake levels. This work should provide a basis for future studies in which process-based models will be used to derive more realistic estimates of the impacts of land use change on lake levels. The results should also be of immediate use in assessments of the water resources and hydropower potential of the Upper Nile.

The starting point for this work has been a major WMO/UNDP funded data collection and analysis exercise for Lake Victoria and the Upper Nile (Hydromet Survey Project; WMO, 1974, 1982). Several subsequent studies have made extensive use of this dataset, including two previous water balance studies by the Institute of Hydrology (1984, 1985; referred to as IH1 and IH2 in this report) and a recent major study by Acres International Ltd. (1990). The WMO and IH studies concluded that the lake water balance this century can be explained satisfactorily solely in terms of natural variations in climate and hence in the inflows and direct rainfall on the lake. By contrast, Acres concluded that the current operating policy at the Owen Falls dam - constructed at the outlet from Lake Victoria between 1951 and 1954 has led to unnaturally high lake levels in recent years. One of the objectives of the present study has therefore been to resolve this issue by making a detailed examination of the outflow record for the periods before and after construction of Owen Falls dam. It is shown that, from the available evidence, there is little reason to accept the Acres hypothesis and that the modelling strategies used in the previous studies remain valid.

A second objective has been to review and update the data used in the previous IH studies. The original simulations covered the period 1925-78. Following a visit to East Africa, sufficient additional data has been collected to update the models to 1990. Also, using an approximate annual model, it has been possible to extend the water balance back to 1900 . The main problem in developing any water balance model for Lake Victoria is the lack of information about the direct rainfall on the lake surface. The estimation methods developed in the IH2 study have therefore been reviewed and an alternative, more easily justified, method has been developed. As part of this work, revised inflow, outflow, lake rainfall and net basin supply series have been derived for use in future modelling studies.

The final objective has been to assess the sensitivity of lake levels to changes in the main components in the water balance and, in particular, to possible changes in inflows due to land use changes. The effect on inflows to date has been estimated from published data for East Africa and the lake catchment and the impact on lake levels estimated using the water balance models. Possible future impacts have also been assessed with the aid of a stochastic rainfall generation model. Based on these studies, suggestions are made for future studies which could improve understanding of the lake water balance and of the likely impacts of land use change on lake levels.

### 1.3 LAYOUT OF THE REPORT

Previous studies on the water balance of the lake are reviewed in Section 2. Section 3 then describes the validation work on the data and attempts to place the observed variations in lake
levels this century in a regional context. The development of the annual and monthly water balance models is described in Sections 4 and 5 together with studies of the sensitivity of the water balance to changes in the individual components. Section 6 then describes how these models were used for a preliminary assessment of the impacts of land use change on lake levels. The main conclusions from this study are given in Section 7 together with suggestions for future work. Finally, Appendix A reviews the hydrological aspects of the Acres study and explains why the conclusions from this study regarding the lake outflow record have been rejected in the present work.

## 2. Previous water balance studies

### 2.1 HYDROLOGICAL BACKGROUND

Lake Victoria, with a surface area of about $67,000 \mathrm{~km}^{2}$, is the largest lake in Africa and one of the largest lakes in the world. It is the main source of water for the White Nile and, since 1954, has provided most of Uganda's electricity through the Owen Falls hydropower scheme constructed at its outlet.

The level of the lake is determined by the balance between direct rainfall, tributary inflows, lake evaporation and lake outflows. By international agreement, releases from the Owen Falls dam are constrained to follow the Agreed Curve, which is a best estimate of the natural outflow which would have occurred if the dam had not been constructed (Figure 2.1). Figure 2.2 summarises levels since measurements first started in 1896. The most striking feature is the sudden and - at the time - unexpected rise of almost 2.5 metres between October 1961 and May 1964, although travellers accounts suggest that levels were also as high or higher in 1878 (Piper, et al., 1986). Clearly, the level of Lake Victoria is highly variable and any process-based model capable of predicting future levels should be able to explain these sudden changes. Key issues include the sensitivity of the lake to natural variations in rainfall, the impact (if any) of the construction of Owen Falls dam on levels, and the extent to which land use changes may have affected levels.

One immediate effect of the 1961-64 rise was to highlight the need for a better understanding of the hydrology of the lake and of its likely behaviour in the future. These concerns resulted in the establishment of the WMO/UNDP Hydromet Survey Project (WMO, 1968, 1974 and 1982) whose main objective was to review and collect hydrometeorological data for the Upper Nile basin and Lake Victoria in particular. During this project, which lasted from 1967 to 1981, the network of raingauges was much improved and the fraction of the catchment which was gauged was increased from about $40 \%$ to $80 \%$. Several experimental catchments were also established and research studies initiated on, for example, lake evaporation and the climatology of the lake. The main data collection work came to an end in 1979-80 with the war in Uganda and since then much of the river gauging network has fallen into disrepair, although rainfall data continue to be collected at many sites. WMO involvement in the project ended in 1981 although the riparian nations of the Nile have continued to fund background work on data collection and processing to the present day.

The main dataset produced by the Hydromet Survey project covered the period 1969-78 and several major water balance studies have made use of this data. The project itself included a modelling and interpretation phase (1975-81) and the results of these studies are discussed in Section 2.2. Between 1983 and 1985, this work was extended and new models developed by the Institute of Hydrology in connection with estimates of the hydropower potential of the Upper Nile. This work is discussed in Section 2.3. Finally, in a major review of future hydropower development options, Acres International Ltd. (1990) presented a detailed evaluation of the outflow record from the lake, which is summarised in Section 2.4 and Appendix A. There have also been a number of research studies on specific aspects of the lake water balance and some of these are discussed in Sections 3 to 6 . Further background information on the hydrology of Lake Victoria can be found in the previous Institute of Hydrology reports (1984, 1985).

## History of the Agreed Curve



Figure 2.1

Lake Victoria end of month lake levels since 1896


Figure

### 2.2 WMO HYDROMET SURVEY PROJECT (1967-81)

The results from the Hydromet Survey project were reported in a series of data yearbooks, consultants' reports and two main reports (WMO, 1974 and 1982). Kite (1981, 1982 and 1984) presents some of the main conclusions from the project regarding the lake water balance, the statistical characteristics of the outflow record and possible alternative lake regulation plans. A mathematical model of the Upper Nile basin was also developed for use in studying alternative lake regulation and hydropower schemes (Brown et al., 1979).

The main use of the mathematical model was in studies of the likely impact of various lake regulation and irrigation/water supply schemes on future lake levels. Attempts were also made to simulate the sudden rise in lake levels of 1961-64 and a smaller rise of about 1.5 m between 1977 and 1980. These simulations showed that the full rise could not be modelled using the observed data alone but that, if the lake rainfall estimates used in the model were increased arbitrarily by $25-30 \%$ in some years, good agreement was obtained between predicted and observed levels. This result suggested that improvements were required in the methods used for estimating lake rainfall from land based rainfall gauges but no further work on this problem was possible within the time available. In these studies, the lake rainfall was estimated from isohyetal maps based on all the available rainfall data at the time and on the results of climatological modelling of the atmospheric circulation over the lake.

In parallel with these simulations, more general water balance studies were carried out to try and understand the cause of the 1961-64 rise in lake levels (Kite, 1981, WMO, 1982). This work, which will be discussed further in Section 3, suggested that the rise must have been due primarily to natural causes (i.e. increased rainfall) and that, over the period for which release records were available (1957-79), releases from Owen Falls dam have closely followed the Agreed Curve. Comparisons with other nearby lakes showed similar jumps in level in the early 1960s and late 1970s, suggesting that a widespread climatological change must have occurred over the region. Again, these results will be discussed further in Section 3.

### 2.3 INSTITUTE OF HYDROLOGY (1983-85)

Between 1983 and 1985, the Institute of Hydrology carried out two reviews of the hydrology of Lake Victoria. The work was performed in association with Sir Alexander Gibb and Partners and Kennedy and Donkin and was funded by the Overseas Development Administration. The overall aims of these studies were (a) to determine whether water balance models alone could explain the observed variations in lake levels this century and (b) to assess the likely future outflows (and hence hydropower potential) at the Owen Falls dam site. Two methods were used for estimating future levels; stochastic modelling of the net basin supply series derived from lake outflows and changes in storage, and stochastic modelling of the lake and catchment rainfall series, coupled with a water balance model for the lake. The results of this work were presented in two main reports (Institute of Hydrology, 1984, 1985) and a technical paper (Piper et al., 1986).

In the IH1 study, the lake rainfall was estimated using a method established by de Baulny and Baker (1970), based on applying seasonally varying weighting factors to data for 8 raingauges sited around the shore of the lake. These gauges were selected since they were the only lakeshore gauges with records available for most of this century. Where possible, total tributary inflows were estimated by developing simple correlations relating flows in the
ungauged catchments to known inflows from gauged catchments. For periods with no gauged inflows, a simple soil moisture accounting model was developed to relate the total inflows to the lake rainfall. Monthly mean values for lake evaporation were taken from WMO (1982) and lake outflows were computed from the Agreed Curve. Using this model, it was found that, over the period 1925-78, reasonable estimates for lake levels could be achieved provided that the lake rainfall estimates were increased by $7.5 \%$ throughout the simulation period and that a single increase of $22 \%$ was made to the rainfall for 1962 . The model was also used to determine equilibrium levels for periods of constant rainfall and the time taken for levels to change between equilibrium levels.

Stochastic modelling studies in the first study were confined to modelling the lake rainfall series alone. Following a detailed examination of the data for the period 1925-78, a model was chosen which incorporated some representation of the observed correlation between October and November rainfall and that between these rainy months and the following March and April rainy season. Probabilities of future levels over a thirty year planning horizon were computed based on 1000 realisations of the lake rainfall record. The simulations suggested that changes in level of the magnitude experienced between 1961 and 1964 are not unique and that, over a thirty year period, there is a serious risk that levels might rise high enough to threaten the safety of the Owen Falls installation.

Following this first study, several sources of uncertainty were identified which could possibly be resolved by more detailed analysis. The main concern was that the stochastic model rarely generated long periods of stable lake levels comparable to those observed between 1900 and 1960. Other concerns were the adjustments needed to the lake rainfall estimates to close the water balance and the use of the lake rainfall in modelling tributary inflows, rather than the catchment rainfalls. It was also considered worthwhile to try stochastic modelling of the net basin supply series, derived from lake levels and the Agreed Curve, for comparison with results from the main studies based on stochastic modelling of rainfall.

The second study was completed in 1985. Several major improvements were made to the lake rainfall and tributary inflow models. In the revised rainfall model, lake rainfalls were still estimated from the same 8 gauges, but using a more sophisticated weighting scheme to combine the individual values (see Section 5.1). Revised estimates for the lake evaporation were also obtained based on Penman estimates for several lakeside stations. Sufficient additional rainfall data was also obtained to derive catchment area average rainfalls for the period 1925-78 and these estimates were used, in conjunction with a conceptual rainfall runoff model, to hindcast the total inflow to the lake back to 1925 . Improvements were also made in the methods used to estimate inflows from the ungauged portion of the catchment.

Two lake water balances were derived, covering the periods 1956-78 and 1925-78 (Figure 2.3). Both showed that the lake behaviour could be explained reasonably well by the water balance model, without recourse to any special adjustments to match the sudden rise of 196164 or the long periods of nearly stable levels. The stochastic modelling studies were based on this revised model. The techniques from the earlier study were developed to model the joint statistics of the catchment rainfall series and the 8 lakeside rainfall series for use with the water balance model. Some serial correlation was still included but on a shorter time scale than in the first study.

The results from the multivariate stochastic modelling suggested that extreme lake levels are less likely than expected from the first study. However, it still remained difficult to generate sequences of lake levels containing both sudden rises in level and longer periods of relatively

Comparison of observed and predicted end of month levels
(a) 1956-78 (b) $1925-78$
(Piper et al., 1986)
(a)

(b)


Figure
stable lake levels like those experienced in the period 1900-60. Similar results were also obtained from a stochastic model of the net basin supply series alone. Some additional simulations were also performed to explore the effects of possible changes in the release policy for Owen Falls dam. The overall conclusion from these studies was that the predicted future lake levels were consistent with historical evidence of past lake levels and - with some reservations - provided a reasonable basis for planning future engineering works on the Victoria Nile.

### 2.4 ACRES INTERNATIONAL LTD. (1990)

Between 1989 and 1990, the Canadian consulting engineers Acres International Ltd. carried out a study of the feasibility of increasing power generating capacity at the Owen Falls site in order to meet anticipated increases in Uganda's power requirements. The work was performed in collaboration with the Uganda Electricity Board and was funded by the World Bank. The overall conclusion was that the least cost option would be to begin by constructing an additional hydropower plant on the right bank of the Nile, supplied by a canal with an offtake just upstream of the Owen Falls dam. The recommended generating capacity of 102 MW would supplement the installed capacity at Owen Falls dam which is currently being uprated to 180 MW . The Acres report covered many different aspects of the proposed scheme, including load demand forecasts, alternative hydropower options, environmental and socio-economic factors, the required spillway capacity at Owen Falls and the overall engineering design. Only the aspects relating specifically to the water balance of Lake Victoria have been considered in this review.

The stated aim of the hydrological studies was to develop a record of monthly streamflows at the Owen Falls dam (a reference hydrology) to be used as the basis for evaluating the firm energy potential of the site and of points downstream. These studies were based mainly on data extracted from previous reports, although the lake level record was updated from 1978 to mid-May 1990. An outflow record was constructed for the period 1900-90 using lake levels and the Agreed Curve for the periods 1900-56 and 1979-90, and the recorded releases from Owen Falls dam for the period 1957-78. The starting point for the analysis was the observation that both this record, and the inflow available for outflow series (i.e. the net basin supply series) show a marked discontinuity from 1961, shortly after the end of construction work and channel improvements at Owen Falls dam (Figure 2.4).

Several reasons for this discontinuity were considered, including errors in the lake level record, changes in land use, climatological causes, errors in the Agreed Curve and discrepancies arising from departures from the Agreed Curve since the start of operations at Owen Falls dam. Of these possibilities, errors in the Agreed Curve were thought to be the most likely explanation, in the sense that the curve does not fully represent the natural rating of Ripon Falls before construction of Owen Falls dam. The conclusion was that the current Agreed Curve considerably underestimates the natural outflows which occurred before the completion of Owen Falls dam in 1954. In effect, this attributes the increase in mean levels since 1964 (compared to the period up to 1954) to construction of the dam. Regional shifts in rainfall regime were ruled out as a possible cause on the basis that, in previous studies (e.g. Kite, 1981), extensive adjustments to the lake rainfall were required to fully simulate the 1961-64 rise in levels and that stochastic models were not able to fully reproduce the range of levels observed in the historic record (e.g. Institute of Hydrology, 1984).

A full history of the Agreed Curve is given in Section 3.3 but, briefly, the curve was first

Lake Victoria net basin supply 1900-90
(Acres, 1990)


Figure 2.4
established on the basis of simultaneous measurements of lake levels at Jinja and discharges at the Namasagali gauging station which is about 75 km downstream of the original outlet from the lake at Ripon Falls. Acres concluded that the Namasagali discharge measurements could not be relied upon because the river section at the site is prone to changes in size and shape and because the site is subject to backwater effects from Lake Kyoga (indeed, the site was abandoned in 1969 for this reason). Adjustments using data from stations further down the Nile were also ruled out because "their ratings were found to be as variable as that at Namasagali" and "appear to have been adjusted to maintain consistency with the outflow recorded at Owen Falls". Cumulative mass plots and double mass plots were presented which were interpreted as confirming these conclusions. Also, on the basis of analytical hydraulic modelling studies, Acres estimated that the current Agreed Curve underestimates the actual outflows pre-1954 by between $19 \%$ and $65 \%$, according to the channel shape assumed for the Ripon Falls before the start of construction work at Owen Falls dam (Figure 2.5). It was concluded that "there is no question of reverting to the hydrologic regime defined by the pre1954 data set as this data set is not representative of the true hydrology during that period but contains serious data errors".

Having established a reference hydrology, least cost and risk assessment studies were performed using a reservoir simulation model. It was generally assumed that releases will continue to be determined by the Agreed Curve, although some preliminary studies were also performed using alternative regulation plans. The results of these simulations will not be reported here but some mention should be made of the data series used to drive the model as these will be referred to again in Appendix A of this report. Our understanding is that three alternative sequences were used to represent the net basin supply. The first, called the reference hydrology, was based on the measured releases from Owen Falls dam from 1961 to 1989 (possibly extended by using the 1949-60 flows adjusted by some unspecified correction method). The other two series were used to represent the worst case low-flow hydrology and were based on the minimum 40 year period in the corrected and uncorrected 1900-89 flow sequences. The reference and low-flow hydrology were initially assigned recurrence probabilities of $99 \%$ and less than $1 \%$ respectively. In the final simulations, the full range of probabilities $(0-100 \%)$ was considered in order to identify the point at which each project would become economically viable. The preferred development option was chosen on the basis of these simulations.

## Further studies

Acres' conclusions regarding the outflow record had major implications on existing views of the water resources and hydropower potential of the Upper Nile. An independent review of the hydrological aspects of the study was therefore commissioned in 1990. This review (Cassidy, 1991) focussed on the accuracy of the discharge measurements at Namasagali, the synthetic Agreed Curves which Acres derived by hydraulic modelling, Acres' preliminary assessments of the need for a spillway at Owen Falls dam and, finally, the methods used by Acres to assess the risk associated with a return of the low flows observed in the period 19001960. Again, the flood and risk assessment aspects will not be discussed here.

Cassidy began by reviewing the independent modelling checks on the Agreed Curve which Acres performed using a one dimensional hydraulic model called HEC-2. Cassidy concluded that this model has some technical limitations when applied to a complex non-uniform flow such as existed at Ripon Falls before construction of Owen Falls dam. Independent calculations by Sir Alexander Gibb and Partners (Gibb) were also cited which showed that by adopting an alternative, and equally plausible, compound weir model the opposite conclusion could be reached i.e. that the Agreed Curve is substantially correct. Cassidy

therefore placed a low weighting on the evidence from these modelling studies. Instead, his main conclusions were based on a set of simultaneous measurements of releases from Owen Falls dam and discharges at Namasagali made during the commissioning of Owen Falls dam (1955-56). These seemed to show beyond reasonable doubt that the flows measured at Namasagali at that time were to within a few percent of flows computed at the dam from the turbine and sluice ratings. Cassidy therefore concluded that errors in the Agreed Curve are small and that the pre-1954 lake outflows computed from this curve must also be regarded as correct to within a few percent.

A second review of the Acres report (Gibb, 1990) dealt mainly with the conclusions reached by Acres on the accuracy of the Agreed Curve and the hydrology of the Upper Nile and Lake Victoria before 1954. Again, the main conclusion was that the suggested errors in the Agreed Curve are not consistent with the evidence available and that the full outflow record based on this curve is correct and should be used in any future hydropower assessments. Climatological studies were cited (Farmer, 1981) which suggest that lake levels have remained high since 1961 due to an increase in rainfall during the October/November rainfall season resulting from a shift in the regional atmospheric circulation. The increased lake levels since 1961-64 could therefore result from an observed climatological phenomenon. Also, work by Nicholson (1980) suggests a link between the high lake levels in the late 1800s and above normal rainfall in the period 1875-95. Acres also noted that this change appears in the net basin supply series although they attributed the apparent shift to data errors rather than climatological causes.

A detailed study was also made of the Namasagali discharge measurements on which the Agreed Curve is based. From the descriptions in the "Nile Year Books" (Hurst, various years) - the authoritative source of data on the Nile - it was concluded that the methods used to measure flows at Namasagali were valid and, by implication, were in accordance with current hydrometric practice. Also, the original measurements were abstracted from the yearbooks and plotted against simultaneous measurements of lake levels at Jinja obtained from the same source. This analysis (Figure 2.6), which was restricted to a period (1923-1950) before the start of construction at Owen Falls, seems to confirm that the Agreed Curve is valid, provided of course that the Namasagali discharge measurements are correct.

To check the Namasagali measurements, a monthly comparison was made between flows at Namasagali and flows at Mongalla further down the Nile during the period 1905-79. Both records were computed from river level observations and the rating equations valid at the time of the observations. The two records agree well during low flow periods with, as expected, higher flows at Mongalla during peak flow periods. In particular, the 1961-64 rise in flows is mirrored at both stations. A similar comparison between lake outflows and flows at Mongalla is shown on a cumulative basis in Figure 2.7. While this does not prove conclusively that either record is correct, it does improve confidence in the rating equations and hence the discharge measurements on which they are based. Similar good agreement is also obtained by comparing monthly mean lake outflows and flows at Murchison Falls for the period 1912-71 (Figure 2.8). It was noted that the rating curve for Kamdini, on which the 1940-50 portion of the Murchison Falls record is based, is excellent, unlike the rather less stable curve for Namasagali.

Gibb then consider the main implications of the Acres conclusions regarding the Agreed Curve. According to this hypothesis, releases must have been artificially low from the start of operations at Owen Falls dam in 1954 in order to match the supposedly incorrect Agreed Curve. Only after the rains of 1961-64 did lake levels rise high enough for releases, again

# Comparison of the Agreed Curve <br> with Namasagali discharge measurements 1923-50 

(Gibb, 1990)


Figure 2.6

Comparison of cumulative flows at Jinja and Mongalla 1905-77
(Sutcliffe and Lazenby, 1990)


Figure 2.7


Figure 2.8
made according to the Agreed Curve, to reach pre-1954 values. If this view of events is correct, then flow records for Namasagali and other gauging stations on the Upper Nile should show a sharp decrease in the 1954-60 period compared to flows pre-1954, and the pre1954 flows should be of a similar magnitude to flows after the end of the 1961-64 wet period. Also, downstream lake and river levels should show similar trends.

Gibb concluded that the Namasagali discharge estimates alone seem to disprove this sequence of events and that the pre-1954 and 1954-60 measurements were very similar. This conclusion would apply even if there was a gross systematic error in the Namasagali rating curve as Acres imply. Other evidence includes (a) the comparisons shown in Figures 2.7 and 2.8 (b) the unique floods of the early 1960s in the Sudd swamps together with a trebling of the area covered between 1952 and 1980 (Sutcliffe and Parks, 1982) (c) the drastic increase in water levels at Murchison Falls from 1961, as evidenced by survey work, tourist photos and movie clips from the early 1950s and the 1960s and (d) greatly increased levels at Lake Albert and Karuma falls after the 1961-64 rise in levels in Lake Victoria. Further evidence is cited from the construction phase of Owen Falls dam. During this time, flows were monitored on a small scale hydraulic model and, at times, were routed through openings of known dimensions in the dam, allowing accurate flow estimates to be made. Gibb conclude that any discrepancies between the actual flows, the Agreed Curve and the results from the hydraulic model would have been immediately apparent to engineers working on the site. It is also pointed out that flows of the magnitude Acres imply (an average of about $1300 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ ) could not have been coped with during the construction phase.

## Concluding remarks

The Acres report, together with the Cassidy (1991) and Gibb (1990) reviews, raise a number of interesting questions about the hydrology of the Upper Nile basin and the accuracy of the Lake Victoria outflow record. This has led us to perform an unusually detailed investigation of the lake outflow data. Also, the Acres hypothesis has been investigated further with the aid of a monthly water balance model. Like Cassidy and Gibb, our overall conclusion is that, from all the data available, the Agreed Curve appears to be correct to within a few percent and the pre-1954 outflows based on this curve are a reasonable estimate of the actual flows which occurred. The analyses leading to this conclusion are distributed throughout this report so, for convenience, our findings relating to the Acres report are summarised in Appendix A.

## 3. Regional analysis of data

This section reviews the data available for Lake Victoria and attempts to place the observed variations in lake levels this century in a regional context. Comparisons are made with levels observed at other lakes in the region and with area average rainfall estimates for East Africa. Also, the outflow record from Lake Victoria is examined in detail and compared with flows observed at stations further down the Nile. On the basis of these studies, new outflow and observed net basin supply series are derived for use in assessing the performance of the water balance models presented later in this report.

### 3.1 LAKE LEVELS

Regular measurements of lake levels first began in 1896 at Entebbe and were later extended to Jinja (1912) and many other stations. For most of this century, these readings have been published in the Nile Basin Yearbooks (Hurst, various years) as 10 day means and monthly means. Monthly mean levels for Jinja for the period 1957-79 were also published by the Hydromet Survey Project (WMO, 1982). Several independent checks have been made on the consistency of these records. For example, Kite (1981) found reasonable agreement between the levels for four gauges - Entebbe, Jinja, Kisumu and Mwanza - for the period 1950-80. In particular, the 1961-64 rise was recorded at all four gauges. Acres (1990) also confirmed that the values for 1900-78 from these sources agreed with values obtained independently from the Ugandan Water Development Department in Entebbe.

For computing the end of month levels required to estimate storage changes, the method described by de Baulny and Baker (1970) is normally used which is to average the final 10 day mean for the previous month and the first 10 day mean of the current month (e.g. WMO, 1982, Institute of Hydrology, 1984). In this report, all lake levels shown are actual or estimated end of month values for the Jinja gauge. For the period 1900-78 these values were taken from the IH2 report and, for the period 1896-99, values were estimated by correlation with levels at Entebbe. Acres (1990) extended these records up to 1990 and an almost complete set of measurements was also obtained independently for use in the present study. In both of these records, missing values at Jinja were infilled where possible by correlation with levels at Entebbe. A time series comparison (Figure 3.1) shows that there is reasonable agreement between these two records.

In discussions of the sensitivity of the lake water balance, it is helpful to have an idea of the usual range of variation of lake levels, both from month to month and over a whole year. Figure 3.2 shows the distributions of level changes for the period 1896-1992. The median month to month variation is typically about 0.07 m with a maximum of about 0.3 m . The corresponding results for the annual variations are 0.4 m and 1.1 m . Taking a longer term view, several studies have shown that the level of Lake Victoria has fluctuated significantly in the past. From travellers' accounts, in August 1878 the lake is estimated to have risen to a similar level to the 1964 peak of 13.3 metres (Lamb, 1966, Piper et al., 1986). A compilation of observations from the 1800s for Lake Victoria (Nicholson, 1989; see Figure 3.3) suggests that levels rose from a minimum around 1860 to the peak in 1878, declined gradually to about 1925 (though with several short lived rises) and were then fairly constant until the 1961-64 rise. Flohn and Burkhardt (1985) reach similar conclusions by estimating the approximate lake outflows and hence levels from observed flows at Aswan in the late

## Comparison of end of month lake levels 1979-90



Figure 3.1

## Frequency plots of month to month and within year variations of lake levels, for the period 1896-1992



Figure 3.2

1800s and early 1900s.
Figure 3.3 also shows some comparisons with other African lakes. It can be seen that several lakes in the region have similar long term trends in levels to Lake Victoria. For example, Kite (1981) showed that three other lakes - Lakes Malawi, Mobutu Sese Seko and Tanganyika - experienced similar rises in the early 1960s and late 1970s and even matched the smaller rises and falls in the period 1950-80. This is not surprising for Lake Mobutu Sese Seko since this lake is part of the Upper Nile system, but the variations in level for the other two lakes indicate the influence of widespread climatological features. From Lema (1980), Lakes Rudolf (Turkana) and Naivasha also experienced peaks in the late 1800s, followed by a gradual decline, a stable period then a sudden rise in the early 1960s. These comparisons all suggest that the 1961-64 rise in levels at Lake Victoria was caused by a regional increase in rainfall, following a long period of stability in the preceding years. A similar rise appears to have resulted from a rainy period in the late 1800 s. This result is considered further in the following section.

### 3.2 RAINFALL DATA

Rainfall in East Africa is linked primarily to the seasonal migrations of the Intertropical Convergence Zone (ITCZ) whose maximum range of movement is between about $15^{\circ} \mathrm{S}$ and $20^{\circ} \mathrm{N}$, or from Malawi to northern Kenya. At the latitude of Lake Victoria, this results in two rainy seasons which typically lie between April and May and October and December. The large scale atmospheric circulation over Lake Victoria is approximately from east to west but is strongly influenced by the on-shore and off-shore breezes generated by the lake itself. This local circulation frequently results in the formation of cumulonimbus clouds over the southwestern portion of the lake leading to a significant increase in rainfall over the lake and in a narrow strip of land some 30 km wide around the lake shore (Channon, 1968). The impact of the lake on rainfall can be seen from the only measurements of rainfall ever made near the centre of the lake. These measurements, which were made during a 6 month period by the Hydromet Survey project (WMO, 1982), indicated a total rainfall roughly 30\% higher than that observed at any other lakeside or island station during the same period.

These observations suggest that rainfall variations over the lake basin should be linked to variations over the whole of East Africa. Nicholson (1989) gives an interesting account of rainfall trends over the East African plateau. Historical accounts and observations point to a period of frequent, but short, dry episodes in the mid 1800s, a relatively wet period between the 1870s and 1890s and an abrupt continent-wide decrease starting in 1895 and lasting until the 1910s, but with a wet spell around 1905 in East Africa. Since then, dry periods have been experienced around 1920, in the 1940s and 1950s and the early 1970s, and wet periods have occurred in the 1960s and late 1970s. The Lake Victoria region seems to have largely escaped the effects of the Sahelian drought in recent years. Many reasons have been suggested for these changing patterns of rainfall, including the influence of sea surface temperatures in the Pacific (i.e. the El Nino Southern Oscillation), greenhouse effects and atmospheric feedback from land use changes, but no consensus has yet been reached.

These trends in rainfall are largely confirmed by Figure 3.4, which shows a compilation of regional rainfall estimates taken from several sources. The values for the Kagera catchment and the northeastern lake tributaries have been taken from the IH2 study and updated from 1979 to 1990 in the present study. The values for the lake rainfall are approximate values implied from the lake water balance and are derived in Section 4 of this report. This

## Compilation of lake level comparisons from various sources



Figure 3.3

Compliation of regional rainfall trends from various sources

compilation confirms the regional nature of the rainfall pattern and, in particular, shows that the increased rainfall in the early 1960s and late 1970s appears in all records, irrespective of the region selected, the number of raingauges used and the area averaging procedure.

These trends also have many similarities to the variations in the level of Lake Victoria this century; for example, the short lived rise in levels around 1905 and the long period of low, roughly constant levels in the 1940s and 1950s. The region-wide increases between 1961 and 1964 are of particular interest because of the sudden rise in the lake levels in this period. Previous studies have shown that, during this period, rainfall increased in both rainy seasons, but particularly in the October-December season (Lamb 1966, Farmer, 1981). Flohn (1987) refers to the 'catastrophic' rains of late 1961 and early 1962 and estimates that, in the Lake Victoria region, rainfall was 1.8 times the normal for the period Aug 1961 to Jan 1962, and almost 4 times normal for November alone. Grundy (1963) describes the widespread flood damage caused throughout Kenya by these rains. Farmer (1981) suggests that 1961 marked a shift in the climate of East Africa and that the mean rainfall since then has been significantly above the mean for the preceding 30 year period, possibly due primarily to an increase in rainfall in the October-November rainy season. The review by Nicholson (1989) suggests that such shifts are not unusual and have often occurred on a timescale of several decades. A comparison of the records for the 8 long term rainfall stations around the shore of Lake Victoria (Figure 3.5) suggests that the heaviest rainfall was to the south and east of the lake. Also, most gauges show a succession of relatively dry years preceding the 1961-64 increase in rainfall.

It is now interesting to ask whether the observed changes in lake level from 1961-64 are consistent with the observed increases in rainfall. The additional rainfall between 1961 and 1964 can be estimated by examining the data for raingauges around the lake shore but first it is necessary to derive an estimate for the relationship between the rainfall observed at these gauges and the total rainfall over the lake. The mean rainfall for these 8 gauges was about 1560 mm for the period 1956-78. For the same period, the IH2 water balance studies suggest that the total lake rainfall was about 1850 mm which in turn suggests that the lake rainfall is on average a factor $1850 / 1560=1.19$ higher than the average rainfall recorded at the 8 lakeshore stations. The additional rainfall between 1961 and 1964, compared to the previous 4 year period, was about 1.35 m at the 8 lakeside stations, equivalent to an increase of about 1.6 m over the lake surface. For the same period, the observed additional inflow compared to the previous 4 years was equivalent to roughly a 0.9 m increase in levels and the additional outflow was equivalent to roughly a 1.1 m drop in levels. Again, all values are based on data tabulated in the IH2 report.

These estimates suggest the net increase in levels between Jan 1961 and Dec 1964 was about 1.4 m compared to the observed change of 2.0 metres in the same period. This agreement seems reasonable given the approximate nature of these calculations, and several possible reasons could explain the difference. For example, the total lake evaporation was quite possibly lower in the period 1961-64 as a result of the increased cloudiness and humidity associated with the increased rainfall. A reduction of only 150 mm per year, or about $9 \%$, would explain most of the difference. Another possible explanation is that the seasonal distribution of rainfall changed during the wetter period, resulting in a change in the relationship between total lake rainfall and rainfall at the lakeside stations. These issues will be discussed further in the water balance studies in Sections 4 and 5 of this report. However, as in the previous IH studies, the preliminary conclusion is that the observed rise in levels is consistent with the observed increase in rainfall and - more generally - that there seems to have been a consistent link between lake levels and regional variations in rainfall since lake

Annual departures from mean for 8 lakeshore rainfall stations (1900-90)







Figure3.5
level observations first began in the late 1800s.

### 3.3 OUTFLOWS AND THE AGREED CURVE

## The Agreed Curve

Until 1950, the outflow from Lake Victoria was regulated naturally at Ripon Falls. In 1951, temporary river regulation works began at Ripon Falls in preparation for construction of the dam at the Owen Falls site some 3 km further downstream. Maximum flows were limited to about $750-800 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ (nominally) during this period and, by agreement with Egypt, flows were not to be reduced below $600 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ in periods when the lake level was high enough to maintain such a flow (Westlake et al., 1954). The dam was closed in 1953 and the water level in the forebay was gradually increased to match the lake level, in the process drowning out the original Ripon Falls. A full account of the design and construction of the dam up to this time is given by Westlake et al. (1954) and Bertlin and Olivier (1954).

Since 1954, releases have, by international agreement, been constrained to match the natural outflows from the lake which would have occurred if the dam had not been built. The operating rule which is followed to achieve this is the Agreed Curve. In practice, the dam operators compute releases on a 10 day average basis and adjust flows in subsequent periods if there have been any departures from the Agreed Curve (Cassidy, 1991). This presumably gives the operators the flexibility to vary releases to suit maintenance schedules and load demands. Between 1957 and 1960, further channel improvements were carried out to ensure that flows could still be controlled from the dam during low flow periods.

Starting in late 1961, lake levels began to rise rapidly and, by early 1962, had risen above the upper limit ( 12.0 metres) of the original Agreed Curve. For the next few years, releases were determined by a straight line extension to the curve, fitted by matching the gradient of the original curve at the point 12.2 metres on the Jinja gauge. In 1968, a new Agreed Curve was adopted based on the results of hydraulic model tests (Hydraulics Research Station, 1966). This new curve is assumed to give a reasonable estimate of the flows which would have occurred naturally for lake levels over the wider range of 9.9-16.3 metres on the Jinja Gauge and has been used to determine the dam releases up to the present day.

These variations of the Agreed Curve are summarised in Figure 2.1. The original curve was established before construction of Owen Falls dam from a comparison of lake levels at Jinja and simultaneous discharge measurements at Namasagali, some 75 km downstream of the dam site. The implicit assumption made was that, at any given time, flows at Ripon Falls are similar to flows at Namasagali. This assumption is reasonable since there are no major inflows or outflows in the reach down to Namasagali, the runoff from the surrounding catchment is negligible compared to the flow in the main channel and major variations in flow normally occur over timescales of several weeks. Direct flow measurements at Ripon Falls were not possible due to the absence of a suitable gauging site. The calculations underlying this curve have been checked many times (e.g. Acres, 1990, Gibb 1990; see Figure 2.6) and the curve is generaily accepted to be a good fit to the available data for lake levels up to about 12 m . Of course, much depends on the accuracy of the discharge measurements at Namasagali but the review by Gibb (1990) and our own review (see below) suggests that these measurements are valid.

For the period of artificial regulation since 1954, the accuracy of the Agreed Curve is of less interest from the narrow viewpoint of modelling the water balance of the lake. Provided the
dam operators follow the curve exactly, then the lake outflows are defined completely. Any departures from the Agreed Curve can be identified from the recorded releases from the dam computed from the turbine and sluice ratings. Acres (1990) have recently reviewed these ratings by deducing the turbine flows from the recorded monthly energy production over the period 1968-90, and the sluice discharges from the sluice ratings established from model tests. They conclude that the recorded turbine/sluice releases were, on average, about $2.4 \%$ too high over this period, primarily because no allowance had been made for the variations in discharge with the operating head over the turbines. Cassidy (1991) also describes tests between 1955 and 1956 in which the turbine/sluice estimates of discharge were found to be some $4-5 \%$ lower than measured flows at Namasagali for averaging periods of 1,2 and 3 days. The preliminary conclusion must be that, to within a few percent, the recorded dam releases since 1954 give a reasonable estimate of the actual outflow from the lake.

Taking a wider view, the accuracy of the Agreed Curve is important in assessing the impact, if any, of Owen Falls dam on lake levels especially in view of the Acres hypothesis (see Section 2.4) that the high levels since 1961-64 are largely a consequence of construction of the dam. Two effects need to be considered, namely (a) the impact on levels if the operators do not follow the Agreed Curve and (b) the impact on levels if the Agreed Curve itself is not a good representation of the natural flows which would have occurred in the absence of the dam. An assessment of both these effects is given in the following sections.

## The outflow record

Using the Agreed Curve, it is possible to estimate the outflows from Lake Victoria for the whole period over which lake levels are available. In addition, actual monthly total releases from Owen Falls dam were compiled for the period 1957-79 inclusive by the Hydromet Survey project and values for the period 1980-87 are given in the Nile Year Books. Values for the period 1989-91 were kindly provided by the Uganda Water Development Department (WDD) in Entebbe. Also, discharge measurements were made at Namasagali at frequencies of up to 10/year from 1940-50, 1/week from 1951-57 and up to $1 /$ month from 1958 to 1970. From 1970, measurements were also made at the Mbulamuti gauging station some 20 km upstream and were continued up to about 1978. As will be shown, for the slowly varying flows on the Victoria Nile, the Namasagali and Mbulamuti constitute a valuable cross-check on the outflow estimates from other sources.

From these data, we have computed three estimates for the outflows from the lake. The first estimate (Record A) is based on the observed lake levels and Agreed Curve throughout. Monthly outflows have been estimated by applying the Agreed Curve to daily values of lake levels estimated by linear interpolation between the end of month lake levels. The second record (Record B) consists of the turbine/sluice releases from 1957 to 1991. A third, novel, record (Record C) has been computed directly from the Namasagali and Mbulamuti discharge measurements. The method used is to interpolate linearly between individual measurements and then sum the resulting daily flows to obtain monthly total flows (Figure 3.6). This method should provide excellent flow estimates provided that the interval between measurements is much less than the time over which significant variations in flow occur. This is certainly true in the period 1951-57 when discharges were measured every week and also seems to give reasonable results for the earlier period when the interval between measurements was longer. In the calculations, the maximum interval allowed between consecutive measurements was 2 months; for longer periods, values in the resulting gaps were set missing.

Figure 3.7 compares Records A, B and C. The comparison between the Agreed Curve

## Interpolation method for estimating monthly total flows

 from individual discharge measurements

$\begin{array}{cl}\begin{array}{c}\text { 菼留 }\end{array} &$|  Monthly flows (actual)  |
| :--- |
| $\bullet$ | <br>

$\begin{array}{l}\text { Daily flows (interpolated) } \\
\text { Discharge measurements }\end{array}\end{array}$

Figure 3.6

Comparison of monthly total outflow records $A, B$ and $C$
(a) A vs B

(b) B vs C

(c) A vs C

estimates and the turbine/sluice releases (A vs B) suggests that there have been significant departures from the Agreed Curve on several occasions but that usually compensatory releases have been made at a later date. The differences in the period 1961-67 are expected as releases were determined by a straight line extension to the Agreed Curve at this time. Other significant differences also occurred in the late 1970s and early 1980s. Following Kite (1981), these differences can be converted to equivalent changes in lake level, for convenience assuming a constant lake area of $67,000 \mathrm{~km}^{2}$. These calculations, shown in Figure 3.8, confirm Kite's conclusion that the cumulative effect on levels was negligible in the period 1957-80. The calculations also show that the effect continued to be small in the period up to 1991.

The comparison of the turbine/sluice releases and the Namasagali record (B vs C) seems to confirm the conclusion (Cassidy, 1991) that the Namasagali discharge measurements were consistently about $5 \%$ higher than the releases recorded at the dam during the 1950s. The small random scatter in the differences between the two stations can be attributed to random measurement errors in either record and minor losses/inflows in the reach down to Namasagali. Again, the cumulative effect of these differences on lake levels is very small. After the 1961 rise in levels, the difference between the two records is larger, with the Record C estimates some $10 \%$ higher than the estimates based on the recorded releases. From 1970, when the discharge measurements were switched to Mbulamuti, the errors are smaller and vary randomly about zero. These results suggest a possible problem with the Namasagali discharge measurements in the period 1961-70 as a result of the increased water levels/flows at that time. This point is discussed further below.

The final comparison, between the Agreed Curve estimates and the discharge measurement record (A vs C), seems to confirm that, from 1940 to 1951, the Agreed Curve gives a good estimate of the actual flows. This, of course, is not surprising since the Agreed Curve was based on the Namasagali discharge measurements in that period. However, it does increase confidence in the validity of estimating outflows from the Namasagali discharge measurements for the period when only 10 measurements were made per year. A more interesting feature of this comparison is that, for the period 1952-53, the actual outflows were well below the estimates given by the Agreed Curve, presumably due to the temporary reductions in flow associated with construction of the Owen Falls dam. The cumulative effect on levels in this period was about 0.15 m , which agrees reasonably well with an estimate of 0.11 m made at the time (Bertlin and Olivier, 1954). From the start of dam operations in 1954, it is more difficult to interpret the results of this comparison since the discharge measurements only give an indication of the dam releases on the day the measurement was made. On a daily basis, it is unlikely that the dam operators can always follow the Agreed Curve exactly, although the objective is to achieve this agreement over longer timescales (Cassidy, 1991). However, it again seems clear that the over-estimates in the Namasagali discharge measurements increased after 1961, since Record C is consistently above Record B from this time.

Our overall conclusions from these comparisons are:
(a) Since the start of operations in 1954, releases from the dam have on average matched the Agreed Curve but departures lasting several weeks have often occurred followed by compensatory releases, particularly in the mid 1960s and late 1970s/early 1980s. However, the cumulative effect of these departures on lake levels is small.
(b) Flows at Ripon Falls were significantly restricted in the period 1952-53 to allow construction work to proceed.

## Implied cumulative error in lake levels from Figure 3.7



Figure 3.8
(c) Until the rise in levels in 1961, the Namasagali discharge measurements agreed to within about $5 \%$ with the turbine/sluice estimates of releases from the Owen Falls dam. After 1961, it seems that the Namasagali measurements were about $10 \%$ higher than the recorded releases.

These results suggest that with the data available the best estimate which can be obtained for the outflow record is as follows:

$$
\begin{array}{ll}
\text { 1896-1939 Agreed Curve+lake levels } & \text { (Record A) } \\
\text { 1940-1956 Namasagali discharge measurements } & \text { (Record C) } \\
\text { 1957-1991 Turbine/sluice releases } & \text { (Record B) }
\end{array}
$$

Using this approach, revised 'observed' outflow and net basin supply series have been calculated for use in evaluating the water balance models described later in this report. Any missing values in the final record have been infilled using the Agreed Curve. The general principle has been to use measured values at Owen Falls in preference to values measured at Namasagali, and to use measured values at Namasagali in preference to values estimated from the Agreed Curve. It could perhaps be argued that, for the period 1940-50, the Agreed Curve would provide slightly better estimates; however, the comparison of Records A and C shows that there is little difference between these records in this period and the use of Record C does have the advantage of using direct observations of flows rather than estimates of flows from levels and a rating curve.

Further conclusions about the accuracy of these records cannot be reached without an absolute check on the Namasagali discharge measurements since these underpin both the Agreed Curve and the Record C estimates. The main interest is in the accuracy of the measurements before 1957 since the turbine/sluice releases are available from this time as a more reliable alternative. These checks are especially important in the light of the Acres (1990) suggestion of a $19-65 \%$ under-estimate before 1954 in the conventional outflow record based on the Agreed Curve (Record A).

## The Namasagali discharge measurements

Discharge measurements were first made at Namasagali in 1923 and then were made regularly from 1940 until about 1970, when the site was closed down in favour of the Mbulamuti gauging station about 20 km upstream. The reason for this change was the increasing difficulty of establishing a stable rating curve at Namasagali, due mainly to backwater effects from Lake Kyoga at the higher river and lake levels since 1961. However, this should not have greatly affected the accuracy of the discharge measurements themselves, provided the measurements were made carefully using suitable equipment.

The Nile Year Books (Hurst, various years) show that discharges at Namasagali have always been estimated using the 'half depth' method, in which the total discharge estimated from the 'half depth' velocities is multiplied by a correction factor of 0.96 to account for the variations in velocity with depth. Berg (1953) gives an interesting account of a discharge measurement at this site in which the full velocity profile was measured using a new Ott current meter calibrated in Germany. This study, made on 19/11/52, suggested that the correction factor for this site should be 0.91 rather than 0.96 , which in turn suggests that the discharge measurements reported in the Nile Year Books up to that time were some $5-6 \%$ too high.

During this discharge measurement, the measured discharge was $564 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ compared with a value $594 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ obtained using the usual half depth method and the 0.96 factor. For
comparison, the lake level at that time was about 11.4 metres, which would have corresponded to about $814 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ on the original Agreed Curve. Outflows at this time were, of course, reduced to aid construction work at Owen Falls dam. An interesting comment in Berg's account states that the current meter calibration was also checked against a Gurley meter owned by the Egyptian Physical Department, who had been responsible for most of the Namasagali discharge measurements up to that time. These two instruments and a third Swiss Amsler instrument all agreed to within $0.6 \%$. It is believed that the 0.96 factor continued to be used throughout the 1950s, so this result seems to confirm the result from comparisons with the turbine/sluice estimates (see above) that discharges at Namasagali were some $5 \%$ too high during the 1950s. The 5\% difference noted in Figure 3.7 can therefore be attributed almost entirely to the Namasagali records rather than the turbine/sluice release record. This conclusion does not necessarily apply to the measurements made after 1961, however, due to the sudden increase in river levels and flows at that time. Indeed, the comparisons shown in Figure 3.7 suggest that the error increased to about $10 \%$ from 1961, most probably due to a change in the velocity profile at the site and backwater effects from Lake Kyoga. However, further data (e.g. full velocity profiles) would be needed to prove conclusively that this increase in errors did occur.

As a final check on the Namasagali measurements and the lake outflows based on these measurements, comparisons have been made of the revised lake outflow record (Record D) with stations further downstream. Monthly comparisons for 3 stations - Jinja, Murchison Falls and Mongalla - have already been shown in Figures 2.7 and 2.8 and seem to confirm the Namasagali record. Many other such comparisons have been made before in research studies on the hydrology of the Nile. However, this interesting subject falls outside the scope of the present study and, for the limited objective of checking the Namasagali record, the compilation of results shown in Figure 3.9 is probably sufficient. The map shows annual total discharges at a number of sites and has been deliberately compiled from a number of different sources to avoid the possibility of bias due to systematic transcription, unit conversion or other errors in any one source. The sources used are:

| Lake Victoria outflows | - Record D from this report |
| :--- | :--- |
| Masindi Port | - UNDP/World Bank (1987) |
| Mongalla | - Nile Year Books (Hurst, various years) |
| Malakal | - Cairo Univ./MIT from Shahin (1985) |

These four records are also plotted together in Figure 3.10. It is worth noting that the Masindi Port record was completely revised during the UNDP/World Bank study and is based on the levels ( $2 /$ day) taken from the original record sheets and converted using new rating curves developed during the study. Figure 3.9 also shows levels for Lake Kyoga and Lake Victoria and the total annual inflows to the lake estimated in Section 5.2 of this report.

The most notable feature of these records is that all stations downstream of the lake registered the sudden rise in flows around 1915 and in the period 1961-64. As expected, the rise at Malakal is delayed due to the influence of the Sudd swamps. The general pattern of flows is similar over the whole century and there is no evidence of a sudden change in flows in 1954 at the start of operations at Owen Falls dam as Acres (1990) imply. The overall conclusion from this comparison, the other comparisons presented in this section, and Figures 2.7 and 2.8, must be that the Agreed Curve provides a good estimate for the lake outflows over the whole period of record.

The main uncertainty which remains is the interesting question of whether the extended


Figure 3.9

Comparison of annual total flows at 4 gauging stations in the Upper Nile basin


Figure 3.10

Agreed Curve for levels above 12.0 m gives a reasonable estimate of the natural outflows which would have occurred if the Owen Falls dam had not been built. This question is only of interest from the narrow viewpoint of determining whether lake levels since 1961 have been significantly altered by construction of Owen Falls dam (and, by implication, whether more or less flow should be released in order to reproduce the behaviour of the natural lake).

From Figure 2.1, the form of the extrapolation above 12.0 m seems physically reasonable, and the model tests on which the extrapolation is based seem to have been made carefully using accurate survey data for the higher water levels since 1961 (Hydraulic Research Station, 1966). However, as Acres (1990) point out, there is only limited data available on the shape of the natural channel before construction work started, so the form of the extrapolation will always be open to doubt. Indeed, hydraulic calculations by Acres suggest a significant error in the Agreed Curve both above and below levels of 12.0 m although, for reasons discussed in Appendix A, these calculations are also open to doubt.

It seems unlikely that this issue can ever be completely resolved. The natural lake is only known to have reached levels above 12.0 m in the 1870s long before outflow measurements were started. Perhaps the main argument in favour of the extended Agreed Curve is that the original curve is known to be accurate for levels up to 12.0 m (see above), the extrapolation looks reasonable and only a small portion of the extrapolated part of the curve has ever been used. The highest level reached since 1961 is only about 13.3 m and, in the 32 years up to 1992, the annual mean level exceeded 12.5 m in only 7 years. Errors in the extrapolation over this limited range are therefore unlikely to be large. Also, the water balance models presented in the next two sections suggest that the behaviour of the lake can be explained satisfactorily mainly in terms of natural variations in rainfall, without hypothesising any significant influence from Owen Falls dam.

## 4. Annual water balance model

Quantitative estimates of past and future lake levels are best obtained using a water balance model. In this study, two models have been evaluated; an annual model and a monthly model. The advantage of an annual timestep is that seasonal variations in rainfall, inflow and evaporation need not be considered and the lag between catchment rainfall and inflow to the lake can reasonably be neglected. The development of the model is described in Section 4.1 and the results are presented in Section 4.2. The main aim has been to map out the broad features of the lake response and to identify trends in lake level over the period for which data are available. The second model is described in Section 5 and has arguably the best performance of any of the many water balance models for Lake Victoria so far developed. This model works on a monthly time step and was developed during the second (IH2) Institute of Hydrology study (1985).

### 4.1 DEVELOPMENT OF THE MODEL

The main factors which determine the level of the lake are direct rainfall on the lake surface, lake evaporation, tributary inflows and the natural (pre 1952-53) or regulated (1954-) outflows. On an annual basis, the lake evaporation can reasonably be assumed to be constant, tributary inflows are related to the lake rainfall and lake outflows can be estimated from the Agreed Curve. Hence it should be possible to develop a water balance model for the lake in which the lake rainfall is the only input variable. Several models of this type have been developed previously and have generally given reasonable results (e.g. Kite, 1981, Institute of Hydrology, 1984). The aim in this study has been to develop a new model using as few calibration factors as possible in order to illustrate the main features of the lake response. The main interest is in the sensitivity of lake levels to changes in individual components in the water balance and, in particular, to possible changes in inflows which might, for example, be caused by land use changes.

The annual water balance for the lake is given by:

$$
\begin{equation*}
\Delta h=P-E+\left(\frac{Q_{i}-Q_{o}}{A}\right) \tag{4.1}
\end{equation*}
$$

where $\Delta h$ is the change in water level over the year, $P$ is the annual rainfall on the lake, $E$ is the annual evaporation, $Q_{i}$ is the annual total tributary inflow, $Q_{0}$ is the annual total outflow and $A$ is the surface area of the lake. As usual, unknown terms such as seepage from the lake or minor abstractions have been neglected, although in practice these may account for part of the inevitable differences between the predicted and observed levels and outflows.

To estimate the lake rainfall, it is necessary to derive a relationship between the total rainfall over the lake and the rainfall measured at the lakeshore stations. Over this century, the number of lakeshore stations has been gradually increased, particularly during the Hydromet Survey project. However, for long term estimates, it is best to use a constant number of stations to avoid possible bias in the results and de Baulny and Baker (1970) correctly identified that only the following 8 stations have reliable long term records:

| Jinja | $1902-91$ |
| :--- | :--- |
| Entebbe | $1900-91$ (and some data from 1896) |
| Kalangala | $1926-77$ |
| Bukoba | $1906-12,1922-92$ |
| Kagondo | $1925-83$ |
| Mwanza | $1902-11,1922-92$ |
| Musoma | $1922-92$ |
| Kisumu | $1903-90$ |

The years indicate the periods for which mainly complete records were obtained in the present study. For this simple model, no attempt has been made to apply weighting factors to the data for these stations. Instead, the average lakeshore rainfall has been estimated as simply the average of all 8 stations in years in which all stations have data, and by the following relationship:

$$
\begin{equation*}
P=\frac{\bar{P}}{N} \sum_{i=1}^{N} \frac{P_{i}}{\bar{P}_{i}} \tag{4.2}
\end{equation*}
$$

in years in which one or more stations has missing data. Here $P_{i}$ is the annual rainfall for station $i$, and $P_{i}$ and $P$ are the mean rainfalls for station $i$ and for the average record respectively calculated for the years in which all stations had data. This method minimises the bias introduced in years when there are missing values for one or more stations. Obviously, it is desirable to use as many stations as possible in computing the lakeshore average rainfall, so the accuracy of these estimates will not be so good before 1925. However, the accuracy should still be sufficient to give an indication of the lake response in this period. Annual lakeshore rainfalls have been computed for the period 1900-91 for use in the water balance simulations.

To calibrate the relationship between lake rainfall and the rainfall measured at the shore, the main data collection period of the Hydromet Survey project (1969-78; see Section 2.3) has been chosen. In this period, approximately $80 \%$ of the lake catchment was gauged and good estimates for the remaining ungauged portion were obtained in the IH2 study. Releases from Owen Falls dam were also computed directly from the turbine/sluice ratings in this period and several methods for estimating or measuring the lake evaporation were evaluated. Following a review of these evaporation studies, Piper et al. (1986) suggested a best estimate of 1600 mm for the average annual evaporation. Thus, for this period, the implied lake rainfall can be estimated to reasonable accuracy using measured values for the other main components in the water balance and the computed changes in lake storage. The resulting annual water balance is:

| Lakeshore rainfall | 1537 mm |
| :--- | ---: |
| Implied lake rainfall | 1811 mm |
| Tributary inflows | 336 mm |
| Outflows | 567 mm |
| Assumed lake evaporation | 1600 mm |

Figure 4.1 shows a comparison of the annual lakeshore values and the implied lake rainfall. A best fit straight line suggests that the lake rainfall is on average about $1.18\left(=\mathrm{k}_{1}\right.$, say) times the lakeshore value. This value has been assumed in the water balance simulations presented below and is close to the estimate of 1.19 presented in Section 3.2 based on the estimated

Relationship between annual implied lake rainfall and rainfall measured at the lakeshore stations (1969-78)


Figure 4.1
water balance for the period 1956-78.
To estimate the total tributary inflows, it is convenient to develop a relationship between the annual total inflow and the lake rainfall. For this comparison, a best estimate of the inflow record for the period 1925-90 has been used. The derivation of this record is described in Section 5.2 of this report but, briefly, the record is based as far as possible on measured inflows with missing values infilled using a conceptual rainfall runoff model. Figure 4.2 shows that there is a reasonably consistent relationship between annual inflows and the lake rainfall, and that the runoff coefficient appears to increase with increasing lake rainfall. For the sensitivity studies, two representations have been used for the inflows and both are plotted in Figure 4.2. The first assumes a constant runoff coefficient, k say, of 0.16 and the second a runoff coefficient which varies with the lake rainfall, with the inflow given by:

$$
\begin{equation*}
Q_{i}=\frac{k A P^{4}}{P_{k}^{3}} \tag{4.3}
\end{equation*}
$$

where $P_{k}$ is a constant. A best fit was obtained for $P_{k}=1.8 \mathrm{~m}$.
The lakeshore rainfall series, the three constants $\mathrm{k}, \mathrm{k}_{1}$ and $\mathrm{P}_{\mathrm{k}}$ and the assumed evaporation are sufficient to define the lake water balance completely provided that the remaining component in the balance - the lake outflow - can be estimated from the Agreed Curve. The results presented in Section 3.3 indicate that, although there have been some minor departures from the Agreed Curve since the start of operations at Owen Falls dam in 1954, the curve seems to have been followed fairly closely in terms of annual total releases. No attempt has been made to represent these minor departures in the simulations. To represent the Agreed Curve, a cubic spline approximation from the IH2 study has been used. The accuracy of this curve is excellent; for example, on a plot with the same scales as Figure 2.1, the curve could not be distinguished from the actual Agreed Curve. The remaining unknown in Equation (4.1) - the lake surface area - has been estimated from the elevation/area relationship established by land surveys during the Hydromet Survey project.

As a first check on the model, Figure 4.3 compares the estimated net basin supply ( $\mathrm{N}_{\mathrm{i}}$ ) for the two inflow models and the 'observed' net basin supply ( $\mathrm{N}_{\mathrm{o}}$ ) calculated from the observed lake levels and the best estimate of the outflow record (Record D) derived in Section 3.3. These two quantities are defined by:

$$
\begin{equation*}
N_{i}=P-E+\frac{Q_{i}}{A} \quad \text { and } \quad N_{o}=\Delta h+\frac{Q_{o}}{A} \tag{4.4}
\end{equation*}
$$

The agreement is reasonable for both inflow models, although the polynomial model (Equation 4.3) gives a better representation of the increase in net basin supply during the 1961-64 period. Both models seem able to represent the large year to year variations in the observed record. The errors in net basin supply vary randomly in many periods, and are typically in the range $10,000-20,000 \mathrm{MCM} /$ year, equating to about $0.15-0.30 \mathrm{~m}$ per year in terms of depth over the lake surface. Larger cumulative errors can build up in periods when several consecutive years of positive or negative error occur. From Figure 4.3, there are several such periods, notably around 1910, 1920, the early 1940s and 1950s and around 1980. The largest errors in predicted lake levels can therefore be expected in these periods. Expressed in terms of depth over the lake, the estimated records have a mean of about 0.43 m $(0.44 \mathrm{~m})$ and a standard deviation of $0.27 \mathrm{~m}(0.40 \mathrm{~m})$, where the values are for the linear and (polynomial) inflow models respectively. For comparison, the observed record has a mean

## Relationship between annual total inflow to the lake

 and the annual implied lake rainfall (1925-90)


Figure 4.3
of 0.40 m and standard deviation of 0.31 m over the same period.
These results are largely confirmed by the lake level simulations. To estimate lake levels, it is necessary to use an iterative method to solve Equation (4.1). An approximate scheme has been used here in which lake outflows are computed using the mean of the start and end of year water levels. Figures 4.4 and 4.5 compare predicted and observed start of year levels for the period 1900-91 for the linear and polynomial inflow models respectively. The agreement is reasonable, and the observed and predicted levels at the end of the simulation are very close, showing that the overall water balance is approximately correct. The predicted rise between 1961 and 1964 is about 1.5 m for the linear model and about 1.9 m for the polynomial model, compared to the observed change of about 2.0 m in start of year levels. The largest errors are about 0.5 m and, as expected, occur mainly in the periods when the net basin supply estimates were worst. To put these errors in context, it should be remembered that the average annual lake rainfall is about 1.8 m , so a cumulative error in lake level of 0.5 m , built up over several years, represents only a small fraction of the input rainfall which drives the model. The two models therefore seem to give a reasonable indication of lake levels and, in some periods, such as the last 20 years, give an excellent representation of observed levels. The differences between the linear and polynomial inflow models are small except during periods of high rainfall such as the period 1961-64.

### 4.2 Sensitivity studies

The main motivation for developing the annual water balance model was to assess the sensitivity of lake levels to variations in the individual components in the water balance. To assist with interpretation of the results, these studies have been performed using various idealised forms for the rainfall series which drives the model. Except in a few cases (noted below), the form used for the inflow model is not important so, for convenience, the linear inflow model has been used in most of the simulations.
(a) Equilibrium response

The first set of simulations concerns the response of the lake during long periods of more or less constant rainfall or net basin supply. Since the lake rainfall is the only input variable to the model, it might be expected that, in such periods, the lake will reach a stable level dependent on the magnitude of the rainfall or net basin supply. This behaviour has been illustrated previously using a rather more complicated annual water balance model developed during the IH 1 study. Figure 4.6 shows the variation in lake levels for a range of values of net basin supply and starting level. The assumed values of net basin supply ( $0.0 \mathrm{~m}, 0.5 \mathrm{~m}$, 1.0 m ) correspond to annual lake rainfalls of $1380 \mathrm{~mm}, 1810 \mathrm{~mm}$ and 2240 mm respectively for the linear inflow model and $1470 \mathrm{~mm}, 1810 \mathrm{~mm}$ and 2080 mm for the polynomial inflow model.

The simulations show that typically, after a period of 10-20 years, a stable level is reached irrespective of the starting level, but dependent on the value specified for the rainfall/net basin supply. Previous, more complicated, water balance studies have also obtained similar results. For example, the IH1 study indicated a response time of 8-10 years for a start level of 12.0 m . Using a stochastic model, Salas et al. (1982) again obtained a value of about 10 years and Evans (1990), using a simple monthly water balance model, suggests a value of 612 years.

Comparison of observed and predicted start of year levels for the period 1900-91
( Linear inflow model )


Figure 4.4

## Comparison of observed and predicted start of year levels

 for the period 1900-91( Polynomial inflow model )


Figure 4.5

Idealised lake response for start levels of 10.5 m and 12.0 m and annual net basin supply values of $0 \mathrm{~m}, 0.5 \mathrm{~m}$ and 1.0 m


Figure 4.6
(b) Response to a rainfall impulse

Changes in lake level arise from natural variations in lake rainfall so it is interesting to consider the response of the lake to temporary changes in rainfall occurring during a long period of constant rainfall. For these simulations, the background annual lake rainfall has been chosen to yield an equilibrium lake level of 12.0 m , which is typical of the observed levels in recent years. The short lived increases and decreases in rainfall assumed represent a $10 \%$ increase or decrease for 1 or 5 years in a row, and a $50 \%$ change for 3 years. Figure 4.7 shows the predicted lake response using the linear inflow model. During the period of change, the level rises or falls at a rate dependent on the size of the change. The annual rate of change is most rapid in the first year but progressively decreases due to the compensating increase or decrease in outflows as levels rise or fall. A $10 \%$ change for 5 years causes a change in levels of about 0.5 m and a change of $50 \%$ for 3 years causes a change of more than 2 m . It is interesting that, according to this model, levels drop more rapidly for a given change than they rise as a result of the increased slope of the Agreed Curve at low lake levels. This result may at first seem surprising and, in reality, some modification to this response may occur due to the lag between changes in rainfall and changes in inflow. However, the observed lake level record does seem to confirm that, with the exception of the 1961-64 rise, falls in level can occur as rapidly as rises.

To place these results in context, it is helpful to consider the magnitude of the natural variations in the lake rainfall series. Figure 4.8 shows the annual departures from the mean in the implied lake rainfall series calculated from the observed net basin supply and the linear inflow model. The annual mean of this series is about 1730 mm and the standard deviation about 270 mm . It can be seen that sustained changes of $10-20 \%$ lasting up to 5 years are common, so that changes of more than 1 m in levels are also likely to be common. The largest change occurred between 1961 and 1964 when the implied lake rainfall increased to about 2300 mm from an average of about 1600 mm in the preceding 4 years, representing an increase of more than $40 \%$ for 4 years. There is some evidence of a long term periodicity in this series, although, as will be shown in Section 6.2, this is much less evident in the lake rainfall series derived directly from the shore based stations. In part, this periodicity may arise from the simple inflow model used to derive this series.
(c) Transition between equilibrium levels

A shift in climate or a progressive change in land use may, over long periods, result in a change in the mean net basin supply. It is therefore interesting to consider the transition of the lake between two periods with different net basin supply. Again, a start level of 12.0 m equivalent to a net basin supply of about 0.5 m - has been assumed in the simulations, and inflows have been estimated from the linear model. The changes considered are increases or decreases of $10 \%$ and $20 \%$ in rainfall, and a $50 \%$ change in the runoff coefficient k .

The results of these simulations are shown in Figure 4.9 and suggest that a long term shift of about $10 \%$ in rainfall causes a change of about 0.5 m in equilibrium lake level. A further similar rise occurs for an additional $10 \%$ increase, and an even greater fall of about 1 m for an additional decrease of $10 \%$. The effect of a $50 \%$ change in runoff coefficient is similar to that of a $10 \%$ change in rainfall. This indicates that the lake water balance is about 5 times more sensitive to long term changes in rainfall than in runoff coefficient, and suggests that a major change in land use has only the same effect as a minor change in climate. Similar results are obtained using the polynomial inflow model. A more rigorous assessment of the comparative sensitivity to runoff and climate is presented in the next section.

Idealised lake response to changes in rainfall of $10 \%$ for 1 year and 5 years and $50 \%$ for 3 years, for a start equilibrium level of 12.0 m


Figure 4.7

Departure from mean series for the implied lake rainfall in the period 1900-91. Also shown are lines representing a $10 \%$ departure from the mean.


Figure 4.8

Idealised lake response to long term changes of $10 \%$ and $20 \%$ in rainfall and $50 \%$ in runoff coefficient for a start equilibrium level of 12.0 m


Figure 4.9

By definition, the equilibrium level is the level at which the outflow matches the net basin supply to the lake. Figure 4.10 shows the implied relationship between equilibrium level and net basin supply for the current Agreed Curve. As mentioned earlier, the current lake level of about 12.0 metres would correspond to an equilibrium net basin supply of about 0.5 m . This would imply an annual lake rainfall of about 1810 mm for both the linear and polynomial inflow models.

Figure 4.11 shows estimates for the sensitivity of the equilibrium lake level to sustained changes in rainfall and net basin supply assuming the linear inflow model. The multiplier varies between about 2 and 15 over the range of levels over which the Agreed Curve is defined. So, for example, for an equilibrium level of 12.0 m , the multiplier is about 5 for rainfall and 4 for net basin supply, so a sustained change of 0.1 m in either component would cause a change of about $0.4-0.5 \mathrm{~m}$ in equilibrium level. For the implied current annual average lake rainfall of about 1.8 m , a 0.1 m change would represent a shift of about $5 \%$. The simulations presented earlier suggest that this new level would eventually be reached provided the change lasted at least 5-10 years. The corresponding change in outflow would in fact be independent of the equilibrium level and is roughly 2500 times the change in annual rainfall if the outflow is in $\mathrm{m}^{3} \mathrm{~s}^{-1}$ and the change in rainfall is in metres. So, for example, a sustained change of 0.1 m in rainfall would cause approximately a $250 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ change in outflows, or roughly $25 \%$ at current lake levels.

These results help to confirm the extreme sensitivity of lake levels and outflows to minor shifts in rainfall and net basin supply. This sensitivity also affects estimates of the sustainable long term power output from Owen Falls dam. The turbines at the dam are currently rated at about $0.15 \mathrm{MW} / \mathrm{m}^{3} \mathrm{~s}^{-1}$, which suggests a sensitivity of about 375 MW per metre change in long term annual rainfall'. A long term shift of about 0.1 m in rainfall, equivalent to about $5 \%$ of the current annual average rainfall, would therefore cause a change of about 37.5 MW in the power output from the dam. This represents about $25 \%$ of the current installed capacity at the dam. This interesting result could possibly be developed further but this falls outside the scope of the present study.

Similar arguments can also be used to estimate the sensitivity of lake levels to changes in runoff coefficient. For a given fractional change in the runoff parameter $k$, and the same fractional change in rainfall, simulations show that the ratio of the changes in lake levels would be about $14 \%$ for the linear inflow model using the best estimate of 0.16 for the runoff coefficient obtained earlier. For the polynomial inflow model, the comparative sensitivity would be slightly less at about $10 \%$, since changes in rainfall have an even greater effect according to this model. These results therefore suggest that a small change in rainfall has about 7-10 times the effect of an equal change in runoff coefficient. For the above example of an equilibrium level of 12.0 m , and a fractional change in rainfall of about $5 \%$, the same percentage change in runoff to the lake would cause a change of less than 0.1 m in levels compared to the change of 0.5 m due to the $5 \%$ change in rainfall. Alternatively, a $35-50 \%$ change in runoff would be required to achieve the same change in equilibrium level.
(e) The lake response time

To estimate the response time of the lake, two approaches seem reasonable. The first is to define the response time as the time taken for the lake level to approach to within an arbitrary distance from the equilibrium level, where this distance is given by a fraction, $p$ say, of the

Relationship between lake equilibrium levels and the annual net basin supply


Figure 4.10

## Estimated sensitivity of lake levels to changes in rainfall and net basin supply



Figure 4.11
original difference between the start level and the equilibrium level. Values for this measure of response time have been computed for $p=1 \%$ and are plotted in Figure 4.12. As an example, the response time would be about 18 years for a net basin supply of 0.5 m and an equilibrium level of 12 m .

A second, more useful, approach is to estimate the time taken for the lake level to approach within an arbitrary distance in metres, $z$ say, of the equilibrium level. If this distance is defined as the typical month to month variation in lake levels, then this gives an estimate of the time taken for lake levels to reach a value which cannot be easily distinguished from the lake level by field measurements. From Figure 3.2, the median month to month variation is about 0.1 m , and this provides a rational length scale for estimating the response time of the lake.

Estimated response times calculated using this approach are shown in Figure 4.12 for a range of starting levels and assuming $z=0.1 \mathrm{~m}$. The curves show the expected behaviour, with the response time increasing as the equilibrium lake level decreases. As expected, as the start level approaches the equilibrium level, the response time drops to zero. For equilibrium levels much greater than the start level, all curves tend towards a common response time of about 10 years. For very low equilibrium levels, the response times tend towards 30 years or more. As an example, if the start level is 12 m , the time taken to reach an equilibrium level of 14 m would be about 9 years and the time taken to reach an equilibrium level of 10 m would be about 21 years.

## (f) Application of results

The annual water balance model, together with the idealised equilibrium results obtained in this section, allow a novel interpretation to be made of the response of the lake this century. For the equilibrium results to be valid, the net basin supply should be constant, or varying over a time scale much less than the typical lake response time. Given that the response time is about 10 years, then it might be expected that equilibrium levels will be reached in periods when the net basin supply is either constant for periods of 10 years or more, or varying rapidly about a mean over timescales of much less than 10 years, say 1-2 years.

An inspection of the observed net basin supply series (Figure 4.3) shows that there have been several such periods since records first began, and these are shown in Figure 4.13, together with the implied equilibrium levels. Towards the end of long periods of constant net basin supply, the lake might be expected to be approaching the corresponding equilibrium level, while, near the start of these periods, or in periods when the net basin supply is varying, the lake will be in transition towards these levels from the most recent peak or minimum level reached. Predicted transition lake levels, calculated using the annual water balance model, are shown for several such transition periods, and appear to agree reasonably well with observed levels in these periods.

This interpretation of the lake behaviour suggests that, in the early part of this century, up to about 1925, the net basin supply only reached a constant value for short periods of up to about 5 years. Consequently, the lake level was constantly in transition between equilibrium states which were never reached. From the mid 1920s to the early 1940s. the net basin supply varied rapidly about a mean of about 0.33 m , equivalent to an equilibrium level of about 11.2 m , and then, up to about 1960 , dropped to about 0.29 m , equivalent to an equilibrium level of 11.0 m . The increased rainfall between 1961 and 1964 then caused the net basin supply to rise to about 1.08 m , with a corresponding equilibrium level of about 13.9 m . This

Estimated lake response times for $p=0.01$ and for $z=0.1 \mathrm{~m}$, (start levels $11.0 \mathrm{~m}, 12.0 \mathrm{~m}, 14.0 \mathrm{~m}$ )


Figure

Idealised interpretation of the relationship between lake levels and net basin supply for the period 1900-91


Figure 4.13
level was never reached, however, because, in the period 1965-77, the net basin supply dropped to a mean of about 0.56 m , with the result that the lake level approached the corresponding equilibrium level of about 12.2 m towards the end of this period. Since then, with the exception of a short lived rise around 1979-80, the net basin supply has entered a rapidly varying phase with a slightly lower mean of about 0.48 m , resulting in an apparent equilibrium level of about 11.9 m under present conditions.

This view of the lake response suggests that lake levels are constantly in transition between a succession of equilibrium states, which are only ever reached if the net basin supply, or lake rainfall, can be considered constant for periods of several years or more. Significantly, the lake has only approached equilibrium for a few short periods since the rains of 1961 to 1964 and levels appear to have been on a downwards trend since that time. These results have clear implications for estimates of future lake levels and these are discussed further in Section 6.2. For the moment, the good agreement between predicted and observed levels can be taken as further evidence that the annual water balance model developed in this section gives a good representation of the actual behaviour of the lake and, in particular, can be used to estimate the sensitivity of lake levels to possible changes in runoff caused by land use change. Estimates of the actual changes in runoff which may have occurred due to land use change are given in Section 6.2.

## 5. Monthly water balance model

The main outcome of the previous Institute of Hydrology studies $(1984,1985)$ was a monthly water balance model which seemed to give excellent predictions of water levels in the main calibration period 1956-78 and reasonable predictions in the period 1925-55. This model was used in conjunction with a stochastic rainfall generation model to estimate the likely future behaviour of the lake and, in particular, the risks of overtopping at Owen Falls dam.

One of the main aims of the present study has been to assess the suitability of this model for use in future studies of the impacts of land use change on lake levels. Using additional data collected during a visit to East Africa, the simulation period has been extended from 1979 up to 1990 , providing a useful test of the model outside the period for which it was calibrated. Also, it has been possible to make a more detailed evaluation of the performance of the model than was possible in the time available in the earlier studies.

The evaluation work is described in Section 5.1 together with some possible improvements which could be made to the model. These include simplifying the method used to estimate the lake rainfall and using an alternative rainfall runoff model to estimate tributary inflows. The effects of these changes are discussed in Section 5.2 and recommendations are made for the best model to use in assessments of the likely future impacts of land use change on lake levels. Some first estimates for the impacts of land use change are given in Section 6.

### 5.1 EVALUATION OF THE MODEL

## Data and constraints

The IH2 model had two main components; a model for the lake rainfall and a rainfall runoff model for the tributary inflows. The resulting lake rainfall and tributary inflow series, combined with Penman estimates for the average monthly evaporation, gave an estimate for the monthly net basin supply series. Lake levels were then calculated assuming that outflows could be estimated from the Agreed Curve. The lake rainfall model was based on equal weighting of the scaled rainfalls for the eight lakeshore rainfall stations discussed in Section 4.1. The rainfall runoff model related observed flows in the main tributaries to rainfall in the respective catchment areas and was used to hindcast flows in these tributaries back to 1925. Flows in ungauged tributaries and from areas below the main gauging stations were derived by ratio from the gauged and partially gauged areas assuming typical runoff coefficients. The annual evaporation was assumed to be constant on the grounds that the only way of hindcasting a variation from year to year would be as an inverse function of rainfall; wetter years would have lower lake evaporation and vice-versa. It was argued that if this function is reasonably linear, the result would be the same as a linear transformation of rainfall.

The structure of the model was governed primarily by the data available for the lake catchment at the time. This restricted the simulations to the period 1925 to 1978. Before 1925, only a limited amount of rainfall data was available for the lakeshore stations and hardly any data was available for the tributary catchments. The main calibration period chosen was the period 1956-78 for which measured outflows were available as were measured tributary inflows for the Kagera and for the four main tributaries in the northeastern part of the lake catchment. These tributaries accounted for about $40 \%$ of the total lake catchment area and about $50 \%$ of the total inflow to the lake in this period.

The most difficult quantity to estimate was the lake rainfall. Rainfall is the most important and variable item in the lake balance, being about five times larger than tributary inflows and about four times larger than the net basin supply. If a consistent, homogeneous dataset was to be used for the whole 1925-78 simulation period, then only the eight lakeshore stations were available to estimate the lake rainfall. This procedure was likened to attempting to estimate the rainfall for an area the size of southern England from only eight raingauges situated around the perimeter. Unfortunately, the spatial distribution of rainfall over the lake is unknown although several attempts have been made to estimate this distribution using atmospheric circulation models and satellite data (see Sections 3.2 and 5.2). In recent decades, many more rainfall records have become available but the use of a lake rainfall series based on a variable, and generally increasing, number of records would make the description of fluctuations over time more difficult to interpret. In deciding to use only eight lakeshore stations, the IH2 study took the view that continuity of record is more important than use of all the information available in any given period. A consistent measure of lake rainfall should therefore rely only on those gauges that have been in operation for most, if not all, of the years since 1925.

The lake rainfall model was based on a method first proposed by de Baulny and Baker (1970). De Baulny and Baker derived a set of monthly weighting factors which accounted for both the differences between the shore based stations and the lake rainfall and the seasonal variations in the distribution of rainfall around the lake. In the IH2 study, this approach was rejected on the grounds that these factors had been distorted by the need to scale the rainfall whilst simultaneously describing the relative influence of the different stations on the lake rainfall. It was concluded that the scaling and combining processes should be separated while accepting that the same eight stations must form the basis of the lake rainfall series. The term scaling here means converting the monthly rainfall values for each station into ratios representing the departures from normal. The term combining means the process of averaging these ratios for all eight stations and then applying a correction factor which represents the differences between the rainfall measured at the lakeshore stations and the actual rainfall over the lake.

After much experimentation, a procedure was used in which overall monthly combination factors were derived from the statistics (mean and standard deviation) of the implied rainfall over the period of the simulations. The implied rainfall is simply the rainfall implied by the observed net basin supply and the predicted tributary inflows and evaporation. This method ensures that the water balance over the whole simulation is satisfied and that the predicted start and end levels match the observed levels exactly. However, there is no constraint on the differences which may develop between observed and predicted levels over the period of the simulations. This approach effectively treats the lake as a giant raingauge and provides perhaps the only rational means of estimating the long term statistics of the lake rainfall from the limited data available. One possible disadvantage is that all errors and omissions in the water balance are lumped into the estimated rainfall; however, this can also be an advantage as the model calibrations based on this rainfall allow implicitly for these errors.

In the final simulations, equal rather than monthly varying weighting factors were used for all eight stations as this caused only a slight decrease in the performance of the model. This is equivalent to saying that the seasonal distribution and monthly variance of the true lake rainfall are equally represented by the records from each of the eight lakeshore stations. This procedure also avoids problems associated with the combination of subsets of the data in periods when records are missing for one or more of the lakeshore stations. Several scaling methods were considered and the final method used was to divide the departures from the
monthly means for each station by the monthly standard deviations for each station. These scaled values were then combined by averaging and then reversing the scaling procedure using the monthly mean and standard deviation of the implied rainfall over the same period. Some enhancement of the standard deviation of the final series was made to allow for the tendency of this process to produce less variable rainfall than was indicated by the implied series.

Using this method, two rainfall series were produced for use in the lake water balance simulations. The first was based on the statistics of the implied rainfall in the period 1956-78 for which most of the other components of the lake balance were known to a reasonable accuracy. The second covered the full period 1925-78 and was based solely on the modelled inflows and outflows. These two series have been updated to 1990 in the present study using additional data collected during a visit to East Africa but using the original scaling and combination factors from the IH2 study. For convenience, the series will be referred to as Rain $\mathbf{A}$ and Rain $\mathbf{B}$ respectively in some of the following discussion.

## Accuracy of the water balance

Since the lake rainfall model ensures that the lake levels at the start and end of each simulation period are the same as the actual levels, the main test of the water balance model is that the simulated response of the lake in the intermediate months and years resembles the actual lake level fluctuations. Simulated levels up to 1978 have already been shown in Figure 2.3 for the two rainfall series. Because of the method used to calculate the two series, there were inevitably differences in the common period 1956 to 1978 and this is one possible objection to this approach. Also, the lake level predictions based on the implied rainfall for the whole period did not reproduce the sharp rise in lake levels in the early 1960s as well as that based on the more accurate implied rainfall for the later period only.

The two simulations have been updated to 1990 in the present study using the full Rain A and Rain B series. Figure 5.1 shows a comparison of the observed and predicted levels for the two periods. It can be seen that the model continues to provide reasonable estimates of the fluctuations in lake levels, but that levels are underestimated by some $0.1-0.5 \mathrm{~m}$ in the early to mid 1980s. The 1956-78 calibration factors give slightly better results. Part of the reason for these underestimates may be that less rainfall data has been available in recent years than in the time the Hydromet Survey project was operating, and this lack of data may have introduced some bias into the results. In particular, the most recent data located for Kagondo was 1983 and for Kalangala was 1977.

As expected, better results are obtained if the scaling and combination factors in the lake rainfall model are recalculated from the statistics of the implied rainfall for the whole of the periods 1956-90 and 1925-90. A slight improvement also results if seasonal weighting factors are used and the stations Kagondo and Kalangala are omitted from the analysis throughout. Figure 5.2 shows the results for two such further simulations. The 1925-90 simulation used all eight lakeshore stations and the optimum seasonal weighting factors from the IH2 study whilst the 1956-90 simulation used a revised version of these factors which omitted Kagondo and Kalangala. Both simulations gave better predicted levels for the 1980s and, as remarked in the IH2 study, inclusion of the seasonal weighting factors improves the modelling of the 1961-64 rise in levels.

These results show that the model continues to perform reasonably well in the extended period 1979-90. However, a more stringent and arguably more useful presentation of the results would be to make a direct comparison of the predicted and observed net basin supply

Comparison of observed and predicted levels (a) 1956-90 using the Rain B series (b) 1925-90 using the Rain A series



Figure 5.1

Simulations of Figure 5.1 repeated after re-scaling over the whole period and using seasonally varying weighting factors
(a)

(b)


Figure 5.2
series. The problem with a comparison of levels alone is that, due to the link between levels and outflows implied by the Agreed Curve, some of the errors in the calculations also appear in the predicted outflow series. The predicted levels alone therefore do not give a full picture of the performance of the model. Figure 5.3 shows the annual differences between the predicted and observed net basin supply for the Rain A and Rain B series. When compared in this way, the differences between the two simulations are much less marked than in a comparison of predicted levels alone. Note that, for these preliminary comparisons, only approximate estimates were used for the tributary inflows in recent years (1979-90).

By contrast, the cumulative errors in each series (Figure 5.4) follow noticeably different patterns in the two cases. As expected, the cumulative error is zero over the period used to derive the respective implied rainfalls and rainfall scaling factors. However, the series based on Rain B does not define the lake balance well over the period up to 1955 although it of course does better than the series based on Rain A for the period 1956 to 1978. Both series perform equally well for the extended period up to 1990. The periods showing the highest rate of accumulation of error appear to be 1925 to 1934, when the predicted net basin supply is lower than the observed values, and from 1935 to 1955 when the opposite is true. Tables 5.1 to 5.3 show the mean values of the main components in the water balance over these and various other periods as well as the errors in the water balance expressed as a percentage of the individual components. In the tables, the observed net basin supply is referred to as the Basin Supply (outflow) series and the predicted value is referred to as the Basin Supply (inflow) series. As an example, the error over the period 1935 to 1955 when using the Rain A series amounts to an average of about $6,000 \mathrm{MCM} / \mathrm{year}$ over this 21 year period. The total error of about $126,000 \mathrm{MCM}$ would be equivalent to a rise in lake level of about 1.9 m assuming a typical lake area of about $67,000 \mathrm{~km}^{2}$ and no compensating adjustment to outflow. In terms of outflow alone the disparity would be equivalent to an underestimate of about $29 \%$ and, in terms of lake rainfall or evaporation alone, the disparity would be equivalent to an overestimate of only about $5-6 \%$.

From the data available, it is impossible to determine the main source of these errors. However, possible explanations include:
(1) The concept of a single set of monthly average rainfall scaling factors does not always work because in the past there have been changes in the rainfall regime in some months affecting only part of the lake and covering different numbers of years.
(2) Unidentified or unexplained errors in the period 1925 to 1934 are hindering a more effective analysis of the rest of the record.
(3) Errors in rainfall are too small to draw any realistic conclusions, and the apparent trends in the series of errors are fortuitous.

Changes in land use around the lake or in the contributing basins have caused variations in the total tributary inflow to the lake between the period 1925-55 and the period 1956-78 over which the rainfall runoff model was calibrated.
(5) The hypothesis advanced by Acres (1990) is correct and outflows before 1954 have been underestimated by $30 \%$ or more (see Section 2.4 and Appendix A).

The last two explanations do not seem to be supported by results presented elsewhere in this report. For example, an explanation in terms of errors in the outflow series seems unlikely

Annual errors in the predicted net basin supply series
calculated using the Rain A and Rain B series (1925-90)


Figure
5.3

Cumulative errors in the predicted net basin supply series calculated using the Rain A and Rain B series (1925-90)


Figure 5.4

| Period |  | 1900-24 mean | $\begin{gathered} 1925-34 \\ \text { mean } \end{gathered}$ | 1935-55 mean | 1956-60 mean | 1961-64 mean | $1965-78$ <br> mean | 1979-89 mean | 1965-89 mean | $\begin{gathered} 1956-78 \\ \text { mean } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lake level (end Dec) Lake area | $\begin{array}{r} \mathbf{m} \\ \mathbf{k m 2} \end{array}$ | $\begin{array}{r} 10.95 \\ 66772 \end{array}$ | $\begin{aligned} & 11.05 \\ & 66829 \end{aligned}$ | $\begin{aligned} & 11.01 \\ & 66819 \end{aligned}$ | $\begin{aligned} & 10.92 \\ & 66740 \end{aligned}$ | $\begin{array}{r} 12.53 \\ 67798 \end{array}$ | $\begin{array}{r} 12.26 \\ 67790 \end{array}$ | $\begin{aligned} & 11.92 \\ & 67542 \end{aligned}$ | $\begin{aligned} & 12.11 \\ & 67681 \end{aligned}$ | $\begin{aligned} & 12.01 \\ & 67563 \end{aligned}$ |
| Change in lake storage | mcm | -826 | 2198 | -254 | 400 | 33911 | -1557 | -4004 | -2634 | 5037 |
| Outfiow | mcm | 20395 | 21453 | 21184 | 19611 | 38606 | 39671 | 34863 | 37556 | 35125 |
| Tributary inflow | mcm |  | 15253 | 19591 | 16396 | 31248 | 22906 | 24000 | 23387 | 22942 |
| Lake rainfall ( $\mathrm{IH}-$ Rain $A$ ) | mm |  | 1614 | 1706 | 1642 | 2076 | 1793 | 1713 | 1758 | 1809 |
| Lake rainfall ( H - Rain B) |  |  | 1653 | 1750 | 1682 | 2111 | 1834 | 1748 | 1796 | 1849 |
| Lake rainfall (de Baulny \& Baker) | mm |  | 1587 | 1668 | 1586 | 1922 | 1734 | 1808 | 1739 | 1734 |
| Lake evaporation | mm |  | 1595 | 1595 | 1595 | 1595 | 1595 | 1595 | 1595 | 1595 |
| Basin supply (Inflow) Rain A | mcm |  | 16520 | 26992 | 19544 | 63799 | 36315 39141 | 31947 34313 | 34393 | 37449 |
| Basin supply (outfiow) | mcm | 19569 | 23651 | 20930 | 20011 | 72517 | 38114 | 30859 | 34922 | 40162 |

Table 5.1

Lake balance errors expressed in terms of the component variables

| Period |  | $1900-24$ mean | 1925-34 mean | $\begin{gathered} 1935-55 \\ \text { mean } \end{gathered}$ | $\begin{gathered} \text { 1956-60 } \\ \text { mean } \end{gathered}$ | 1961-64 mean | $\begin{gathered} 1965-78 \\ \text { mean } \end{gathered}$ | $\begin{gathered} \text { 1979-89 } \\ \text { mean } \end{gathered}$ | 1965-89 <br> mean | 1956-78 mean |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Basin supply (Inflow) | mcm |  | 16520 | 26992 | 19544 | 63799 | 36315 | 31947 | 34393 | 37449 |
| Basin supply (outflow) | mcm | 19569 | 23651 | 20930 | 20011 | 72517 | 38114 | 30859 | 34922 | 40162 |
| Error in water balance BS(Inflow) - BS (outfiow) | mcm |  | . 7131 | 6062 | -467 | -8718 | -1799 | 1088 | 529 | -2713 |
| Error as percentage of: |  |  |  |  |  |  |  |  |  |  |
| Outtiow | \% |  | 33 | -29 | 2 | 23 | 5 | 3 | 1 | 8 |
| Lake ralnfall (IH - Rain A) | \% |  | -7 | 5 | -0 | -6 | -1 | 1 | -0 | -2 |
| Tributary Inflow | $\%$ |  | -47 | 31 | 3 | -28 | -8 | 5 | -2 | -12 |
| Lake evaporation | \% |  | 7 | -6 | 0 | 8 | 2 | -1 | 0 | 3 |
| Notes: <br> A positive percentage error indicates a possible overestimate of the relevant variable. In each case all other variables are assumed to be accurate. |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |

Lake balance errors expressed in terms of the component variables

| Period |  | $\begin{gathered} 1900-24 \\ \text { mean } \end{gathered}$ | $\begin{gathered} \text { 1925-34 } \\ \text { mean } \end{gathered}$ | $\begin{gathered} \text { 1935-55 } \\ \text { mean } \end{gathered}$ | 1956-60 mean | 1961-64 mean | 1965-78 mean | 1979-89 mean | 1965-89 mean | 1956-78 mean |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Basin supply (Inflow) | mcm |  | 19122 | 29942 | 22167 | 66140 | 39141 | 34313 | 37017 | 40146 |
| Basin supply (outfiow) | mcm | 19569 | 23651 | 20930 | 20011 | 72517 | 38114 | 30859 | 34922 | 40162 |
| Error in water balance BS(Inflow) - BS(outflow) | mcm |  | -4529 | 9011 | 2157 | -6377 | 1027 | 3455 | 2095 | -15 |
| Error as percentage of: |  |  |  |  |  |  |  |  |  |  |
| Outflow | \% |  | 21 | -43 | -11 | 17 | -3 | -10 | -6 | 0 |
| Lake rainfall ( IH - Rain B) | \% |  | -4 | 8 | 2 | -4 | 1 | 3 | 2 | - |
| Tributary Inflow | \% |  | -30 | 46 | 13 | -20 | 4 | 14 | 9 | - |
| Lake evaporation | \% |  | 4 | -8 | -2 | 6 | -1 | 3 | -2 | 0 |

Notes:
A positive percentage error indicates a possible overestimate of the relevant variable.
In each case all other variables are assumed to be accurate.
in view of the results of the comparisons presented in Section 3.3, which seem to show beyond reasonable doubt that the pre-1954 outflow record is substantially correct (see also Appendix A). An explanation in terms of inflow also seems unlikely because, as will be shown in Section 6, the impact to date of land use changes is probably negligible. Furthermore, the implied errors in the inflow and outflow components are not consistent throughout the period from 1925. For example, if outflows were underestimated for 1935 to 1955, it seems odd that they appear to be overestimated from 1925 to 1934. The same Agreed Curve was used to derive outflows from known, and accurate, lake levels for both periods. It is therefore much more likely that the major part of the error lies in the estimates of lake rainfall.

An idea of the natural variations in lake rainfall can be obtained from Table 5.4, which compares the mean monthly rainfall at the eight lakeshore stations for two consecutive periods of about 20 years. These comparisons show that the mean lakeshore rainfall can differ markedly over this timescale, and seem to confirm the theory (see Section 3.2) that, since 1961, there has been a significant shift in the climate over the lake. These results suggest that this change has been mainly in the climate of the northern and eastern part of the lake, resulting in an increase in rainfall during the months of October, November and March. However, Table 5.4 also shows that there have been contemporary reductions in rainfall at several stations in some other months. There is as yet no physical explanation for these changes, nor is it possible to determine whether the representativeness of each station in terms of lake rainfall is stable or varying over the period 1925-90. Given these problems in estimating the rainfall over the lake, implied errors of only $5-10 \%$ could be considered small.

These results therefore suggest that the most likely cause of the errors in the predicted net basin supply is small yet persistent errors in estimates of the lake rainfall. Tables 5.1-5.3 confirm that errors of only a few percent in rainfall can cause quite significant errors in the lake water balance. This result was also found from the sensitivity studies described in Section 4.2. The sensitivity to rainfall may be further enhanced if, as might be expected, evaporation is lower in wetter years and higher in drier years. The variation in the difference between rainfall and evaporation would then be greater than the variation in rainfall alone from one period to another. Thus the nominal errors in rainfall alone could be less than those shown yet give the same result in terms of lake water balance error.

These arguments suggest the following conclusions:

1. Errors in the lake water balance arise primarily from relatively small errors in the estimates of lake rainfall.
2. As the errors in rainfall estimates appear to be consistent from year to year within periods of several years, they probably arise from shifts of climate whose effect on lake rainfall cannot be effectively defined.
3. If these conclusions are correct, there is again little support for the Acres hypothesis that outflows were substantially underestimated in the years before 1954. Also, if the Acres hypothesis is valid, some explanation must be found for the contrary result obtained for the period 1925 to 1934.
4. The water balance of the lake is uncertain to the extent that it does not, and possibly cannot, provide an unequivocal judgement on the likely accuracy of the lake rainfall, tributary inflows or the Agreed Curve. Most hydrologists would concur with a

Comparison of mean monthly rainfall in different periods (mm)

| JINJA |  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | Year |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1935-55 | 56 | 63 | 108 | 212 | 166 | 70 | 54 | 101 | 89 | 87 | 111 | 103 | 1221 |
|  | 1956-78 | 64 | 85 | 141 | 195 | 140 | 69 | 70 | 83 | 100 | 141 | 161 | 87 | 1336 |
|  | difference | 8 | 22 | 33 | -17 | -26 | -1 | 15 | -17 | 12 | 53 | 50 | -16 | 114 |
| ENTEBBE | 1935-55 | 68 | 82 | 162 | 268 | 281 | 114 | 73 | 78 | 83 | 77 | 139 | 117 | 1541 |
|  | 1956-78 | 88 | 101 | 179 | 260 | 238 | 121 | 69 | 79 | 72 | 126 | 178 | 112 | 1623 |
|  | difference | 20 | 19 | 17 | -9 | -43 | 7 | -4 | 1 | -11 | 49 | 39 | -4 | 82 |
| KALANGALA | 1935-55 | 115 | 138 | 232 | 373 | 352 | 234 | 139 | 116 | 113 | 123 | 187 | 197 | 2332 |
|  | 1956-78 | 135 | 137 | 239 | 340 | 322 | 162 | 96 | 94 | 114 | 159 | 210 | 200 | 2214 |
|  | difference | 21 | -1 | 7 | -33 | 30 | . 72 | -43 | -22 | 1 | 35 | 23 | 4 | -118 |
| BUKOBA | 1935-55 | 144 | 153 | 229 | 366 | 311 | 83 | 51 | 75 | 117 | 141 | 175 | 204 | 2049 |
|  | 1956-78 | 150 | 180 | 254 | 398 | 316 | 89 | 51 | 66 | 102 | 153 | 195 | 193 | 2147 |
|  | difference | 6 | 27 | 25 | 32 | 6 | 6 | -0 | -9 | -15 | 12 | 20 | -11 | 97 |
| KAGONDO | 1935-55 | 132 | 132 | 216 | 311 | 244 | 29 | 24 | 55 | 91 | 122 | 173 | 160 | 1690 |
|  | 1956-78 | 119 | 152 | 219 | 362 | 234 | 47 | 26 | 40 | 94 | 115 | 201 | 161 | 1770 |
|  | difference | -13 | 20 | 3 | 51 | -10 | 18 | 2 | -16 | 3 | -7 | 28 | 0 | 80 |
| MWANZA | 1935-55 | 95 | 103 | 143 | 172 | 101 | 17 | 9 | 20 | 43 | 41 | 122 | 147 | 1012 |
|  | 1956-78 | 102 | 114 | 156 | 177 | 71 | 16 | 15 | 21 | 25 | 99 | 158 | 146 | 1100 |
|  | difference | 8 | 11 | 12 | 5 | -30 | -1 | 7 | 1 | -18 | 57 | 36 | -1 | 88 |
| MUSOMA | 1935-55 | 62 | 63 | 120 | 189 | 107 | 30 | 17 | 18 | 29 | 35 | 65 | 69 | 804 |
|  | 1956-78 | 59 | 84 | 123 | 182 | 101 | 24 | 21 | 22 | 31 | 53 | 117 | 78 | 895 |
|  | difference | -3 | 21 | 3 | -7 | -6 | -6 | 4 | 4 | 2 | 18 | 52 | 9 | 91 |
| KISUMU | 1935-55 | 55 | 76 | 152 | 196 | 177 | 99 | 68 | 92 | 73 | 63 | 91 | 106 | 1248 |
|  | 1956-78 | 71 | 98 | 155 | 234 | 175 | 79 | 63 | 90 | 84 | 87 | 139 | 102 | 1376 |
|  | difference | 16 | 22 | 3 | 38 | -2 | -20 | -5 | -2 | 11 | 24 | 47 | -4 | 128 |
| LAKE | 1935-55 | 136 | 142 | 207 | 290 | 227 | 71 | 38 | 78 | 99 | 118 | 148 | 197 | 1750 |
|  | 1956-78 | 147 | 159 | 226 | 295 | 209 | 62 | 37 | 69 | 96 | 147 | 210 | 192 | 1849 |
|  | difference | 11 | 18 | 20 | 4 | -18 | -9 | -1 | -9 | -3 | 29 | 62 | -4 | 99 |
| Notes: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| This comparison is based on the IH lake rainfall series defined in their second report. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Table 5.4
judgement that a small error in lake rainfall is more likely than a large error in inflows or outflows. In practice there are likely to be minor errors in all these variables.

## Possible improvements to the model

Despite these uncertainties, the IH2 water balance model continues to give a good representation of the observed lake levels and the net basin supply in recent years. The main constraints on modelling the historical water balance remain the quality and availability of data, rather than the sophistication of the modelling techniques used. However, with the benefit of hindsight, some aspects of the model might be simplified or improved while still obtaining a satisfactory water balance.

The main questions concern the lake rainfall model. Two possible objections might be made to the method of using the implied rainfall to calibrate the model. The first is that the resulting estimates of lake rainfall vary according to the period picked to estimate the implied rainfall. Although the differences are very small, this method is difficult to justify to non specialists. The second objection is the use of the arbitrary factor applied to the overall standard deviation in order to retain the observed variability in the series. In the following section, an attempt is made to derive an alternative net rainfall model which estimates both the rainfall and evaporation together.

There are fewer possibilities for improving estimates for the remaining terms in the input side of the water balance. As remarked earlier, there seems little chance of hindcasting a plausible lake evaporation record, and the monthly average Penman estimates are probably the best that can presently be achieved. The main constraints on improving the tributary inflow estimates are the lack of observed flow data for calibration and the quality of the catchment rainfall estimates. The difficulty in estimating the catchment rainfalls is again lack of suitable long term data and it seems unlikely that these estimates can be much improved. However, to provide a cross-check on the IH2 estimates, some alternative tributary inflow estimates have been derived using a different rainfall runoff model. Also, a new best estimate of the total inflow record has been derived using additional observed flow data located in the present study.

### 5.2 REVISED MODEL

## Rainfall and evaporation

Rainfall data for the lakeshore and lake island stations suggest that a successful monthly lake rainfall model must include some allowance for seasonal variations in rainfall and multiplying factors to allow for the differences between rainfall at the shore based stations and over the lake. The most convincing way to combine the data from the lakeshore stations would be to derive weighting factors based on seasonal isohyetal rainfall maps for the lake and the surrounding catchment, However, despite many attempts, no truly convincing isohyetal maps have yet been constructed for the region. The main difficulties are that rainfall gradients are high near the lake shore (of the order $10 \mathrm{~mm} / \mathrm{km}$ in the northwestern sector of the lake) and that very little data is available from over the lake surface.

During the Hydromet Survey project, several attempts were made to construct seasonal isohyetal maps from the limited data available from island stations and satellite pictures of cloud cover of the lake. The resulting estimates of lake rainfall generally seemed much too low in the main test period of 1950-79 (Kite, 1981). The other main approach has been to use
the results of atmospheric circulation models as a guide (e.g. Datta, 1980). However, these models necessarily give a much simplified, average view of the interactions between the lake circulation and the larger scale atmospheric movements over the region. A recent example (Flohn and Burkhardt, 1985) relies heavily on the results of a limited six month run of rainfall data obtained from Nabuyonge island during the Hydromet Survey project (Figure 5.5). This map, if correct, shows that the peak annual rainfall approaches 3 m over the southwest portion of the lake.

There is even less information on the variation in evaporation over the lake surface. The Penman equation provides probably the most reliable way of estimating lake evaporation over timescales of 1 month or more. In the absence of better information, the IH2 study estimated the total lake evaporation as the average of Penman estimates of evaporation from several of the lakeshore stations. A better method would be to account for the variations in evaporation over the lake surface and for the influence of cloud cover and rainfall on evaporation. From the data available for the lakeshore stations, there do appear to be significant variations in evaporation around the lake, with much higher evaporation in the drier, warmer regions to the south and east. There also appears to be some link between evaporation and rainfall. An indication of this trend can be obtained by comparing the monthly average rainfall and Penman evaporation at each of the lakeshore stations. Comparisons for two stations at opposite sides of the lake are shown in Figure 5.6 and suggest that the evaporation may drop by up to $10 \%$ in wet months compared to drier months. However, with current knowledge, there is no obvious way of generalising these results to the whole lake or over time.

In the future, it may be possible to improve estimates of the rainfall and evaporation distribution using satellite imagery and improved measurements over the lake. Until that time, the only alternative approach is to infer the implied values from the remaining components in the lake water balance. Since the evaporation cannot be specified with any certainty, it seems sensible to model both the rainfall and evaporation together as net rainfall. This is the approach taken for the revised model. To calibrate the method, the period 1969-78 has been chosen as this period has the most complete data coverage, with outflows measured at Owen Falls dam and about $80 \%$ of the lake catchment gauged (see Section 4.1). Figure 5.7 shows the average seasonal variations in the implied net rainfall for this period together with the average rainfall for the lakeshore stations. Here, the lakeshore average rainfall has been calculated simply by averaging the monthly data from the lakeshore stations, but omitting Kagondo and Kalangala due to the lack of data for these stations in recent years. It can be seen that there is a close link between the implied net rainfall and the rainfall measured at the 6 lakeshore stations. This result is confirmed by correlation plots of the individual monthly values, some examples of which are shown in Figure 5.8. Best fit straight lines fitted to these correlation plots suggest the following values for the slope and intercepts:

Isohyetal map based on a model of the atmospheric circulation over the lake
(Fiohn and Burkhardt, 1985)


Figure 5.5

The relationship between average monthly rainfall and Penman evaporation for two lakeshore stations (1956-78)



Figure 5.6

Average seasonal variations in the lakeshore rainfall and implied net rainfall for the period 1969-78


Figure 5.7

Seasonal relationship between monthly implied net rainfall and lakeshore rainfall (1969-78)




Figure 5.8

|  | Intercept | Slope |
| :--- | ---: | ---: |
|  |  |  |
| Jan | -150 | 1.72 |
| Feb | -107 | 1.13 |
| Mar | -94 | 1.05 |
| Apr | -94 | 1.13 |
| May | -53 | 0.78 |
| Jun | -143 | 1.41 |
| Jul | -136 | 1.30 |
| Aug | -133 | 1.10 |
| Sep | -63 | 0.38 |
| Oct | -146 | 1.44 |
| Nov | -98 | 1.07 |
| Dec | -93 | 1.11 |

These parameters provide a basis for estimating the net rainfall for the lake from the average rainfall measured at the 6 chosen lakeshore stations. From Figure 5.9, it can be seen that this method gives a good representation of the monthly net rainfall over the calibration period. Also, over the longer period 1925-90, the annual variations (Figure 5.10) are very similar to those implied by the IH 2 rainfall series (Rain A) and the annual lake rainfall model described in Section 4.1. It therefore seems that this alternative method for estimating the monthly net rainfall gives results of comparable accuracy to those given by the IH2 scaling and combination method. The method also relies solely on observed data for a short calibration period (1969-78) and so can provide a more objective test of the water balance model over the full simulation period.

Using this new series, it is interesting to see whether the suspected change in rainfall regime in March, October and November discussed earlier is also reflected in the net rainfall series. If anything, this change should be more apparent due to the amplifying effect of decreases in evaporation in wetter months and increases in drier months. Figure 5.11(a) shows the variations in net rainfall in the period 1925-90 for three averaging periods: March-May, JuneSeptember, October-December. For the first of these periods, the March-May wet season, there seems to have been a slight increase in the 1960s which has now subsided while, for the June-September dry season, there was no change in the early 1960s. For the OctoberDecember wet season, though, the change is very marked and appears to have persisted up to the present. The long term increase in net rainfall on the lake is about $30 \mathrm{~mm} / \mathrm{month}$, equivalent to about $100 \mathrm{~mm} /$ year, or about $5 \%$ of annual rainfall, for these three months alone. This seasonal increase would indeed seem to be one of the main reasons for the persistent increase in lake levels since the early 1960s. There is, of course, no way of knowing if this is a permanent change or if the rainfall regime might change to pre-1961 conditions or some completely new state in the future.

As a further check on this important result, independent estimates of net rainfall have been calculated using the observed net basin supply derived in Section 3.3 and a best estimate for the inflow record which is derived in the following section. Figure 5.11(b) shows the variations in these estimates of net rainfall from 1925 to 1990 for the same three seasons shown in Figure 5.11(a). The results are very similar and again indicate that, since the early 1960s, there has been a significant and sustained increase in the net rainfall during the months of October to December. Thus the same conclusion is reached using both the modelled net rainfall series (based on rainfall data for the lakeshore stations) and the implied net rainfall series (based on observed inflows, outflows and lake levels).


Figure 5.9

## Comparison of annual total lake net rainfalls for three models

 for the period 1925-90

Figure 5.10

## Variations in the net rainfall in the period 1925-90

(a) Based on lakeshore rainfall (5 year moving average)

(b) Based on observed inflows and net basin supply (5 year moving average)

——March-May $\quad . . . . . . . .$. June-September -- October-December

Figure 5.11

An additional conclusion from this comparison is that the seasonal distribution predicted by the net rainfall model appears to be broadly correct throughout the period 1925-90. Thus the use of only a short calibration period for this model (1969-78) - after the rains of 1961 to 1964 - does not appear to have introduced any major bias into the predicted net rainfall values. This also implies that the seasonal relationship between the lake rainfall and lakeshore rainfall must have remained reasonably constant in the period 1925-90, which provides additional confidence in the results given by the net rainfall model.

## Tributary inflows

The IH2 study used a simple conceptual rainfall runoff model to hindcast inflows back to 1925 for 5 major rivers in the lake catchment. The total inflow was then estimated by applying monthly scaling factors to these flows. These factors were estimated from flow data for the period 1969-78, in which most of the lake catchment was gauged and hence reasonably accurate estimates were available for the total inflow. The aim in the present study has been to update the total inflow and catchment rainfall series as far as possible and to provide an independent check on the hindcasting work by using a different rainfall runoff model. The IH2 study included a very careful assessment of the original data and of suitable methods for estimating the ungauged runoff and it was not felt necessary to repeat this work here other than to make a few spot checks on the accuracy of the calculations.

The new total inflow record is based as far as possible on observed data. During the 1969-78 period, approximately 20 of the main tributaries of Lake Victoria were gauged, together accounting for about $80 \%$ of the $194,000 \mathrm{~km}^{2}$ catchment area (WMO, 1982). Before 1969, only the river Kagera and four of the main (northeastern) Kenyan tributaries were gauged accounting for about $40 \%$ of the catchment area and about $50 \%$ of the total inflow. These tributaries were the Nzoia, Yala, Sondu and Awach Kaboun. Gauging started in 1956 on these tributaries and in 1940 on the Kagera. A search for more recent data showed that regular measurements have only been made on the Kagera in recent years. Monthly mean levels were obtained for the Nyakanyasi gauging station up to 1984 and were converted to flows using an implied rating based on observed monthly levels and flows for the period 1973-1978. Monthly flows for the period 1985-89 were obtained for the Rusomo Falls station in Rwanda and values for Nyakanyasi further downstream were estimated by a simple correlation model. Flows were also estimated for the period 1933-39 from some previously overlooked level data for the Kyaka Ferry gauging station reported in the Nile Year Books (Hurst, various years). For the remaining gauging stations in the catchment, only some limited monthly mean level data was available covering the period up to 1984. Again, implicit ratings were developed to convert these levels into approximate flows.

Two methods have been used to compile the total inflow record. For the period 1969-78, the estimates from the IH2 study have been used which are based on the total gauged inflow with some allowance for the inflow from the remaining ungauged $20 \%$ of the catchment. These estimates were also used in developing the net rainfall model discussed above. For the remainder of the period 1925-90, the total inflows have been estimated by scaling the inflows from the 4 northeastern tributaries and the Kagera, again using the method developed during the IH2 study. To obtain a full record for each of these 5 rivers, missing periods of data have been infilled using a rainfall runoff model. For the Kagera, infilling was necessary for the period 1925-33 and for 1990 while, for the northeastern tributaries, infilling was required for the periods 1925-55 and 1985-90.

Figure 5.12 shows the structure of the model used for this infilling work. The model - called HYRROM - is a simple 9 parameter conceptual rainfall runoff model developed at the Institute of Hydrology. The model has four main stores which control the translation of rainfall into runoff. An efficient optimisation routine allows the model to be calibrated to obtain a good match between the catchment rainfall and any available observed flow data. The philosophy underlying HYRROM is that the model is simple enough to apply when the data are of limited accuracy yet includes some representation of real catchment processes. Although it is by no means the best or most sophisticated model available, HYRROM has been shown to give good results on a wide range of catchment sizes and types.

To run the model, estimates are required for the catchment average rainfall and potential evaporation. Both the rainfall and evaporation values from the IH 2 report were used here. The catchment rainfall series were extended from 1978 to 1990 using the same averaging process used to derive the IH2 values. Typically, each series is based on data for 5-6 raingauges scattered around the catchment. The model was optimised against various 4 year periods from the measured flow data for each catchment and a set of typical parameter values was derived from these analyses. For future reference, these values are summarised in Table 5.5.

Figure 5.13 shows a comparison of the observed and predicted monthly total inflows from the 4 northeastern tributaries for the period 1956-78. The agreement is reasonable, particularly during low flow periods. The average predicted annual inflow over this period was $5,100 \mathrm{MCM}$ compared to the observed value of $5,270 \mathrm{MCM}$. Figure 5.14 shows that, for each of the 4 catchments, the average seasonal errors are generally lower than those obtained from the IH2 model. The best improvements are in the months of January and February. HYRROM was also applied to the Kagera but no great improvement was noted over the IH2 model.

The infilled records for the 5 catchments were used to generate a new total inflow record for the period $1925-90$. Figure 5.15 compares this record with that given by the IH 2 rainfall runoff model. It can be seen that the IH2 record tended to overestimate flows slightly during periods of recession but otherwise gave a reasonable representation of the seasonal variations in inflows. As expected, both records show a marked increase in lake inflows in the period 1961-64 as a result of the increased rainfall in this period. An inspection of the individual inflow records shows that the main increases in the Kenyan tributaries were in late 1961 and 1962 while the increased inflows from the Kagera occurred mainly in 1963 and 1964. This difference results partly from the large storage in lakes and swamps in the Kagera basin, and partly because 1963 was an unusually wet year in the Kagera catchment.

For the new record, the overall average annual inflow is about $20,000 \mathrm{MCM}$. For the period 1925-60, before the sudden rise in inflows, the average is about $17,000 \mathrm{MCM}$ and, for the period 1965-90 after the rise, the average has increased to $22,500 \mathrm{MCM}$. The standard deviations for these three periods are 910,700 and 910 MCM respectively. These results suggest that, in parallel with the apparent shift in rainfall patterns since 1961-64, there has been an associated increase in tributary inflows of about $5,500 \mathrm{MCM}$ per year, or about $80 \mathrm{~mm} /$ year when expressed in depth over the lake surface. In terms of the annual lake rainfall, this increase represents roughly an additional $5 \%$ change above that due to the direct rainfall on the lake surface.

## Structure of HYRROM model



Figure 5.12

## Comparison of the observed and modelled total inflow

 from four northeastern tributaries (1956-78)

Figure 5.13

Seasonal variations in errors for the modelled inflows, calculated using the IH2 and HYRROM models (1956-78)



Figure 5.14

Comparison of IH2 modelled total inflows with the best estimate derived in the present study (1925-90)



Figure 5.15

## Summary of HYRROM parameters

| Catchment | Nzoia | Yala | Sondu | Awach <br> Kaboun | Kagera |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Area $\left(\mathrm{km}^{2}\right)$ | 11900 | 2650 | 3230 | 610 | 56000 |
| SS | 1.63 | 1.20 | 1.70 | 2.61 | 0.53 |
| RC | 0.56 | 0.25 | 0.35 | 0.86 | 0.46 |
| RDEL | 0.0 | 0.005 | 0.0 | 0.2 | 2.86 |
| RX | 1.0 | 1.0 | 1.02 | 1.0 | 1.08 |
| RK | 0.02 | 0.04 | 0.05 | 0.05 | 0.02 |
| FC | 0.58 | 0.75 | 0.41 | 0.80 | 0.76 |
| GSU | 190 | 500 | 230 | 901 | 1204 |
| GSP | 2.96 | 1.54 | 1.56 | 1.70 | 1.0 |
| GDEL | 0.0 | 0.03 | 0.01 | 0.01 | 12.9 |


| PARAMETER | NAME |
| :---: | :--- |
| SS | Size of the vegetation interception <br> and surface detention store (in <br> millimetres) |
| RC | Surface runoff partitioning factor |
| RDEL | Routing store delay (in days) |
| RX | Routing store index |
| RK | Routing store factor <br> factor |
| GC | Groundwater store delay (in days) |
| GSP | Groundwater store index |
| GSU | Groundwater store factor |

## Revised water balance 1925-90

The revised inflow record and the net rainfall model allow a new, updated estimate to be made for the monthly net basin supply series and the lake water balance in the period 192590. Figure 5.16 shows the estimated levels using this new series for the periods 1956-90 and 1925-90. The revised model seems to give excellent predictions of levels in the periods 192542 and 1962-90, but overestimates levels in the period 1943-61 by some $0.2-0.5 \mathrm{~m}$. Levels at the end of each simulation are surprisingly close to the observed levels considering that, unlike in the IH2 study, there was no constraint on the overall water balance. Overall, the accuracy of the estimated levels is comparable with that given by the IH2 model (Figures 5.1 and 5.2), although the timing of the main errors has shifted. The revised water balance for selected periods is summarised in Table 5.6.

Figure 5.17 shows a more stringent comparison in terms of the cumulative error in the predicted net basin supply. Also shown is the error series for the IH2 Rain A series combined with the IH 2 rainfall runoff model. For the IH 2 series, some minor differences will be noted between this plot and Figure 5.4 since the observed net basin supply here is based on the best estimate of the observed flows derived in Section 3.3 (Record D) rather than on the Agreed Curve. Also, the tributary inflows in the period 1979-90 have been re-calculated using the full IH2 rainfall runoff model.

These comparisons show that the cumulative errors in the two series are similar but occur in different periods. As suggested by the lake level predictions, the main accumulation of errors for the new series occurs between 1943 and 1961, with very small errors outside this period. The rate of accumulation of error in this period is very similar to that of the IH2 model and is of the order 100 mm per year. This is roughly equivalent to a $5 \%$ error in estimates of the lake rainfall. The 1925-34 errors discussed earlier seem to have been almost eliminated in the revised model.

These encouraging results suggest that the revised model developed here gives very similar results to the original IH2 lake rainfall model. Either of these models could therefore be used in assessing the likely impacts of land use change on lake levels. Also, the revised tributary inflow series should prove useful in helping to calibrate process-based models of the impacts of land use change on runoff into the lake. The remaining uncertainties in the water balance almost certainly arise from minor errors in the estimates of the net rainfall. As discussed earlier, these errors probably arise from a combination of deficiencies in the data and actual shifts from year to year in the distribution of rainfall and evaporation around the lake. In particular, the net rainfall component was calibrated for a period (1969-78) after the apparent shift in October-November rainfalls which occurred after 1961-64, so cannot be expected to perform as well for earlier periods.

Comparison of observed and predicted levels using the revised water balance model (a) 1956-90 (b) 1925-90



Figure 5.16

Cumulative errors in the predicted net basin supply series calculated using the revised water balance model (1925-90)


Figure 5.17

Revised water balance for selected periods 1925-92
(IH2 values in brackets)

| Period | Net rainfall | Inflow | Outflow |  | Net basin supply (mm) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | (mm) | (mm) | (mm) | Predicted | Error |  |
| $1925-55$ | $120(66)$ | $250(275)$ | 350 | $370(341)$ | 55 |  |
| $1956-78$ | $260(235)$ | $340(342)$ | 550 | $600(577)$ | 10 |  |
| $1925-78$ | $180(138)$ | $290(304)$ | 440 | $470(442)$ | 35 |  |
| $1925-60$ | 120 | 250 | 360 | 370 | 60 |  |
| $1965-90$ | 210 | 340 | 570 | 550 | -5 |  |
| $1925-90$ | 180 | 300 | 450 | 480 | 25 |  |

## 6. Impacts of land use change

Previous studies on Lake Victoria have generally sought to explain the fluctuations in lake levels this century in terms of variations in climate or the influence of Owen Falls dam. Little work has been done on assessing the possible impacts of land use change on inflows and hence on lake levels.

Over this century, there has been a rapid growth in the population of the countries surrounding the lake with increasing urbanisation and many associated changes in agricultural practices. It seems likely that these changes must have had some effect on the tributary inflows to the lake and hence on lake water levels. This section presents a preliminary assessment of the magnitude of these changes and their likely impact on the water balance of the lake.

The discussion begins with a description of the existing land use in the lake catchment and the changes which have been experienced this century and over longer time scales. In the time available, it has only been possible to perform a brief review of the literature on this subject; however, this has been sufficient to obtain order of magnitude estimates of the likely impacts of land use changes on tributary inflows to the lake. The effects of these changes on the water balance of the lake are discussed in Section 6.2. Tentative estimates are also presented for possible future changes in land use and the impact these changes might have on future lake levels. These results are interpreted with the assistance of stochastic models of the likely variations in lake levels due to natural variations in climate alone.

### 6.1 ESTIMATES OF LAND USE CHANGE

Lake Victoria has a catchment area of about $194,000 \mathrm{~km}^{2}$ representing about three times the surface area of the lake ( $67,000 \mathrm{~km}^{2}$ ). The catchment includes northern Tanzania and western Kenya and most of Rwanda and Burundi. Only a small proportion of the inflow to the lake originates in Uganda. Land surface types range from highland forests in the headwaters of the Kagera and the Kenyan tributaries to open savannah and grasslands in the low lying regions immediately to the south and east of the lake. The highland forests in Rwanda and Burundi have been all but cleared and lowland forest remains only in the form of a few isolated patches along the Ugandan and Tanzanian shores. Extensive areas of swamp and marshlands are found in the lower parts of the Kagera catchment to the west of the lake.

There have been very few detailed surveys of land use in the lake catchment but one of the best seems to be that due to Morgan (1969) which is based on data collected up to the late 1960s. Figure 6.1 shows a map based on the results of this study. The most intensively cultivated regions lie around the lake margins, on the slopes of the Kenyan highlands and in Rwanda and Burundi. Much of the remaining low lying land is used for animal grazing, especially to the south east of the lake. The main cities lie in Rwanda and Burundi and around the lake shore.

The main increases in population and associated changes in land use have occurred this century. However, there is some evidence that extensive clearance of natural vegetation for cultivation began as long ago as 3,000 BP (Kendall, 1969). Over such long time scales, variations in climate have also had a dramatic effect on the vegetation cover in the catchment.

Map of estimated land use in the lake catchment in the 1960's
(Morgan, 1969)

## Key



Figure 6.1

Pollen analyses and other techniques suggest that, within the Holocene period (the last 11,000 years), the region has been subject to several wetter 'pluvial' periods, an important one beginning around $9,500 \mathrm{BP}$ (Butzer et al., 1972). There have also been relatively dry periods such as the Younger Dryas from 11,000 to 10,000 years BP (Roberts, 1990). From 12,000 BP, the lake basin appears to have been extensively forested then, between 7,000 and 6,000 years BP, there was a shift from evergreen to semi-deciduous forest. In the past 3,000 years, the forests have became progressively less abundant (Kendall, 1969) with the result that, at present, only a small proportion of the lake catchment has extensive forest cover.

The main interest in this study is to estimate the rate at which land use is changing in the catchment. This then allows an assessment to be made of the impact of these changes up to the present and allows projections to be made of likely future changes. One of the main factors influencing the rate of change is the rate of population growth. Some approximate estimates of population growth in the period 1957-90 are shown in Table 6.1. The population figures for the lake catchment have been estimated from the total population of each country and the fractional area of the country within the lake catchment. In these calculations, the proportion of the population living in the lake catchment has been assumed to remain constant in time. These estimates indicate that the population of both the lake catchment and the region as a whole has doubled over the last 20 years and trebled since 1957. The most densely populated part of the catchment is the Kenyan portion; between 1970 and 1989, the population density increased from about 19 to 41 people $\mathrm{km}^{-2}$ in the country as a whole and is currently approximately 300 people $\mathrm{km}^{-2}$ in the lake catchment region (EIU, 1991). These results indicate that major changes in land use are likely to occur if the population continues growing at the present rate.

The types of changes which may occur include changes in agricultural practices, such as the clearance of natural vegetation for cultivation, land and swamp drainage, increased irrigation and land degradation due to overgrazing, and changes resulting from increasing urbanisation, such as an increase in the area of impermeable surfaces, vegetation clearance, improved land drainage and road construction. Changes in hydrological regime may also arise as a result of abstractions by industry and the construction of hydropower schemes and sewerage systems. The following sections present some preliminary assessments of the magnitude of each these types of change in the catchment of Lake Victoria.

## Vegetation cover

Table 6.2 gives some recent estimates of the main changes in vegetation cover which are believed to have occurred in the lake catchment in the past 25 years (UNEP, 1991). The dominant trend in East Africa over this period has been an increase in the area of land under crops. The increased cultivated area amounts to some $40,000 \mathrm{~km}^{2}$ in the region as a whole, and it is reasonable to assume that much of this change has been concentrated in the Lake Victoria catchment as this is the major high rainfall zone in East Africa.

In Kenya and Tanzania the increase in cultivated land has occurred mainly as a result of deforestation and most of the $2-6 \%$ change indicated in productive land has probably been concentrated in the forested upland regions in the lake catchment. In Kenya, montane forests are increasingly being cleared to make way for plantation crops, especially coffee and tea. This activity has been mainly concentrated on the Nandi Hills around Kericho while, further north in the Kitale area, maize is the dominant crop. This pattern of increasing cultivation of both subsistence and cash crops has been repeated throughout the Lake Victoria catchment (Morgan, 1969). In Tanzania coffee production is dominant in the Bukoba region whilst irrigated cotton production occurs in conjunction with cattle rearing to the south of the lake

Population change in the Lake Victoria catchment and East Africa 1957-1990

|  | \% Total Popn in Lake Victoria Catchment | Population of Lake Victoria Catchment (and Total Population) in millions |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1957 |  | 1970 |  | 1990 |  |
| Burundi | 75 | - | - | 2.6 | (3.5) | 4.1 | (5.5) |
| Kenya | 39 | 2.8 | (7.3) | 4.4 | (11.2) | 9.8 | (25.1) |
| Rwanda | 75 | - | - | 2.8 | (3.7) | 5.4 | (7.2) |
| Tanzania | 25 | 2.3 | (8.9) | 3.4 | (13.5) | 6.9 | (27.3) |
| Uganda | 40 | 2.5 | (6.2) | 4.0 | (9.8) | 7.4 | (18.4) |
| TOTAL | 41 | - | - | 17.2 | (41.7) | 33.6 | (83.5) |

Sources: Morgan (1969) and UNEP (1991)

## Land use change in East Africa 1966/68-1986/88

| Country | Total productive area ( $10^{3} \mathrm{~km}^{2}$ ) |  | Land use as \% total productive area |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Crops |  | Pasture |  | Forest |  |
|  | 1966/68 | 1986/88 | 66/8 | 86/8 | 66/8 | 86/8 | 66/8 | 86/8 |
| Burundi | 17.2 | 23.4 | 61 | 57 | 36 | 40 | 3 | 3 |
| Kenya | 102.6 | 96.9 | 18 | 24 | 41 | 41 | 41 | 35 |
| Rwanda | 21.8 | 22.3 | 31 | 52 | 40 | 22 | 29 | 26 |
| Tanzania | 841.7 | 824.0 | 5 | 6 | 42 | 43 | 53 | 51 |
| Uganda | 162.0 | 174.0 | 30 | 39 | 30 | 28 | 40 | 33 |

Source: UNEP (1991)
around Mwanza. The Ugandan shore of the lake is also an important area for the production of cash crops. Some recent deforestation has also occurred in Rwanda but the main change has been an increase in cropland at the expense of land for pasture. In Burundi there has been not only an increase in cultivated land but also pastures and even forests with a resulting large fall in the area of unproductive 'other' land. This may represent a deliberate reversal of the process of deforestation which has left the country almost entirely devoid of forests.

Catchment studies suggest that the clearance of natural vegetation and, in particular, forests will lead to increased flood flows and reduced baseflows. However, very few studies have been made in conditions appropriate to the Lake Victoria catchment. Research into the hydrological impacts of deforestation has been concentrated mainly on the highlands of Kenya and to a lesser extent Tanzania. For example, during the 1960s, a number of experimental catchments were established to investigate the effects of land use change including the change from tall rain forest to tea estates and forest clearance for subsistence cultivation (Pereira, 1962, Blackie et al., 1979).

The impact of clearing tall rain forest and the subsequent cultivation of tea plantations was investigated by comparing two subcatchments in the Kericho region of Kenya. This area falls within the Lake Victoria catchment. The two main differences between the cleared and the forested plots (Dagg and Blackie, 1965) were that transpiration losses were lower in the cleared plot and stormflow runoff from the cleared slopes was greater than from the forested plot. The stormflow runoff was found to increase by between $120 \%$ and $300 \%$ according to rainfall intensity. However, after a period of about 5 years, the tea plants on the cleared land had become sufficiently established to minimise any difference in the hydrological behaviour of the two plots (Blackie, 1972). The main conclusion was that the change in land use from forest to tea is of hydrological significance only during the period that the land is relatively bare (Blackie, 1979).

The impact of forest clearance for subsistence cultivation was investigated at a similar experimental site in the Mbeya region of southern Tanzania (Edwards, 1979a). The research team again expected to find a reduction in dry season flows and an increase in peak flows but the data suggested that the change from forest to cultivated land had little impact on the hydrology of the site. This result was attributed to the high permeability of the local soil which allowed most of the incident rainwater to enter the ground as storage, even on the cleared slope. This surprising conclusion illustrates the importance of taking local factors into account (in this case soil type) when considering the impact of land use change on a region wide scale.

Other experimental work in the region has investigated the influence of land use on sediment production (Ongweny, 1979; Dunne, 1979). Studies on Mount Kenya and the Abedares found that sediment yield increases progressively from undisturbed forest to cultivated and grazing land due to increased overland flows. The clearing of forests to make way for roads is a particularly important factor in the increase in overland flow and sediment yield (Ongweny, 1979). Some limited experimental studies have also been made of the impact of grazing on an area of semi-arid rangeland in Atumatak, Uganda (Edwards, 1979b). Two enclosures were used, one of which was allowed to recover from overgrazing by the removal of all livestock while the other remained grazed. The ungrazed enclosure underwent rapid re-vegetation, which led to a reduction in peak flows during flash floods. Total runoff was also reduced as rainwater was able to percolate deeper into the soil. This suggests that, as livestock rearing is intensified or moves onto marginal land, the hydrology of these areas is likely to change to a regime of greater runoff and more rapid and larger flood peaks. However, the most
heavily grazed areas of the lake catchment are not classed as being in serious danger of desertification (UNEP, 1992) although there is some evidence of overgrazing to the southeast of the lake. These areas also have the lowest annual rainfall within the catchment, so the hydrological importance of overgrazing is likely to be limited mainly to increased peak flows during flash flooding.

Of all the changes in vegetation cover which have occurred in the lake catchment, it seems likely that the greatest hydrological impact arises from the clearing of the forests in Kenya to make way for cultivation. As both a gradual process over the last 3,000 years and a more rapid one as tea and coffee plantations have been established this change is likely to have increased the runoff to Lake Victoria. Of particular importance is the fact that the regions which have been subjected to the most recent deforestation lie in the zones of greatest rainfall which is where most of the runoff to the lake originates. For example, Vowinckel and Orvig (1979) estimate that as much as $60 \%$ of total annual runoff in East African catchments comes from deforested slopes. In conjunction with the increase in runoff, deforestation may also have led to the establishment of a more responsive hydrological regime in the catchment. Storm flow peaks are likely to be greater and lag times reduced.

From the results presented by Dagg and Blackie (1965), it seems reasonable to assume that, in the Kenyan highlands, complete clearance of forests can result in roughly a 2-3 fold increase in runoff in the period when the soil remains bare. After a few years, the effect becomes negligible due to the rapid growth of crops or natural vegetation in the cleared areas. Figure 6.1 suggests that, in the 1960 s, about $25 \%$ of the Kenyan catchments were forested and Table 6.2 indicates that there has been a reduction of about $1 / 7(=15 \%)$ since then in this cover, or about $0.5 \%$ per year. An approximate estimate of the resulting change in runoff can be obtained by assuming that this change has occurred gradually over this period, that the effect of each clearance becomes negligible after 5 years and that runoff is increased by the worst case of 3 times while the land is bare.

Approximate calculations then suggest that this would have caused a sustained increase of about $5 \%$ in runoff at worst. Since the Kenyan catchments contribute about $35 \%$ of the total inflow to Lake Victoria, the total effect on inflows to the lake would be a sustained increase of about $2 \%$. The effect on lake levels is therefore likely to be small although, locally, other impacts, such as loss of habitat and increased erosion, are likely to be severe. A similar change might be expected from the forested areas in Rwanda and Burundi since the forests occupy a similar proportion of the catchment and the river Kagera contributes a similar proportion of the total lake inflow.

## Irrigation and land drainage

Table 6.3 gives some recent estimates of the increase in irrigated land in East Africa over the last 25 years. With the information available, it is not possible to estimate the percentage of this irrigated land lying within the lake catchment. However, even if all the irrigated land were in the lake basin, the total area currently under irrigation would represent only about $1 \%$ of the total catchment area. The main changes have occurred on the lower uplands in Kenya where intensive cultivation of sugar and cotton has increased and often requires irrigation, usually on a small scale. Irrigation is also used in zones of rice production such as in the swamps around the mouth of the Yala River (Morgan, 1969).

A preliminary assessment of future irrigation requirements in the lake catchment was performed as part of the Hydromet Survey project (WMO, 1982). In this study, water requirements were estimated on the basis of information supplied independently by each of

## Area of irrigated land, East Africa 1966/68-1986/88

| Country | Area Irrigated (10 $\left.{ }^{\mathbf{3}} \mathrm{ha}\right)$ |  | \% of Arable land |  |
| :--- | :---: | :---: | :---: | :---: |
|  | $1966 / 68$ | $1986 / 88$ | $1966 / 68$ | $1986 / 88$ |
| Burundi | 18 | 69 | 2 | 5 |
| Kenya | 20 | 46 | 1 | 2 |
| Rwanda | 4 | 4 | 1 | - |
| Tanzania | 32 | 147 | 1 | 3 |
| Uganda | 4 | 9 | - | - |
| TOTAL | 78 | 275 | - | - |

Source: UNEP (1991)
the countries in the lake catchment. Estimated annual requirements were expected to reach about 10, 210, 80 and 250 MCM for Burundi, Kenya, Rwanda and Tanzania respectively by the year 1980 and $100,900,330,2280$ MCM by the year 2000. Assuming that these very tentative estimates are correct, and that these changes all apply to the lake catchment, the proportion of lake inflows used for irrigation will have risen to about $15 \%$ by the year 2000 from a proportion of about $2 \%$ in 1980 and, by interpolation, about $10 \%$ at present, assuming the worst case of irrigation losses of $100 \%$. In practice, a proportion of these flows is likely to return to the lake via increased baseflows and improved drainage.

Land drainage often occurs in conjunction with irrigation schemes but the hydrological impacts are difficult to determine. One immediate effect is to cause a fall in the local water table and hence baseflows. Also, annual runoff may be expected to increase as a result of drainage and clearance, with less evaporation from standing bodies of water and a reduction in transpiration from swamp vegetation. Flows in the dry season may, however, decrease as the water stored in the system from the wet season is reduced. The construction of drainage ditches aids in the more rapid and efficient transport of rainfall to the stream channel network. The clearance of swamps may be expected to have a similar effect, reducing the storage time in a river system. These land use practices are therefore likely to lead to a changed hydrological regime in which peak flows are both more instantaneous and higher. No figures are available for the area of land which has been drained in the lake catchment; however, this area is likely to be much less than the area under irrigation and the net hydrological impact is likely to be small when compared to the impact of the irrigated areas.

## Major engineering works

There is normally a close link between the need for water engineering schemes and the urban population. Table 6.4 summarises the increase in the urban population in East Africa between 1970 and 1988. The proportion of the population living in urban areas has more than doubled in each country other than Uganda. In Tanzania the increase has been in the order of $450 \%$ in only 18 years. The rate of urban growth has apparently stabilised at around $7 \%$ across the region. Similar rates of increase presumably also apply to the towns in the Lake Victoria catchment. The population of Kisumu, for example, increased from 153,000 to 185,000 between 1979 and 1989 (EIU, 1991). Other cities which may have benefited from rural to urban migration include Kitale and Mwanza. In addition to the rapid growth of urban areas there has been an associated expansion of the national and regional road networks, leading to an acceleration of the destruction of natural habitats. This expansion has taken the form of both an increase in the number of roads and the proportion which are paved. Between 1978 and 1984-85 the total length of road in Tanzania increased from $45,000 \mathrm{~km}$ to $81,800 \mathrm{~km}$ whilst in Uganda the proportion of roads that were paved rose from $9 \%$ to $22 \%$ (UNEP, 1991).

Increased urbanisation often causes major hydrological changes related to the removal of vegetation and the construction of buildings, roads and other surfaces from impermeable materials. Typical effects are a reduction in evapotranspiration, an increase in runoff and a fall in groundwater levels. Runoff is dominated by overland flow off the impermeable surfaces to gutters and drainage channels so that annual total and storm flows may be expected to increase. The fall in the water table can result in lower dry season flows, however, as the baseflow component is reduced. These changes may also be accompanied by changes arising from the construction of sewerage systems for the removal of waste water and storm water runoff. The impact of a sewerage network is to increase the speed at which water and waste may be discharged from an urban area to an outlet into, say, a nearby river channel. This factor, along with the fact that there is an increased amount of storm water

## Urbanisation in East Africa 1970-1988

| Country | Urban Population (\%) |  | Annual Growth Rate (\%) |  |
| :--- | :---: | :---: | :---: | :---: |
|  | 1970 | 1988 | 1970 | 1988 |
| Burundi | 2 | 7 | 1.6 | 7.7 |
| Kenya | 10 | 23 | 8.4 | 7.3 |
| Rwanda | 3 | 7 | 7.7 | 7.4 |
| Tanzania | 7 | 32 | 12.0 | 7.9 |
| Uganda | 8 | 10 | 4.1 | 6.1 |

Source: UNEP (1991)
population increases continue at the present rate, this rate of change may increase in the next century. These changes may also be accompanied by a marked change in the seasonal distribution of inflows to the lake.

These estimates, although very crude, do at least give an idea of the likely magnitude of the changes in inflows and provide a starting point for a more detailed assessment. It is now of interest to estimate the impact of these changes on lake levels by means of the water balance models developed earlier in this study.

### 6.2 EFFECTS ON THE LAKE WATER BALANCE

## Impact to date

Considering first the impact in recent years, the sensitivity studies described in Section 4 suggest that a $10 \%$ decrease in inflows would have only a small effect on lake levels. Figure 4.13 suggests that, since about 1979 , the net basin supply has been oscillating about a mean of about 0.48 m , corresponding to a lake equilibrium level of about 11.9 m , and Table 5.6 suggests that the mean lake inflow has been about 0.34 m in terms of equivalent depth over the lake. If land use changes have caused a $10 \%$ drop in inflows, then the natural net basin supply in the absence of the land use changes should have been about 0.51 m , corresponding to a lake equilibrium level of about 12.0 m . The difference in levels is only about 0.1 m , which is within the normal month to month variations in lake levels due to seasonal variations in rainfall (see Figure 3.2). This result suggests that the impact of land use change to date has been negligible. In particular, the high levels of recent years cannot be attributed to changes in land use.

Viewed in terms of flows, this change in inflows would imply that outflows, and hence flows in the Victoria Nile, are some $60 \mathrm{~m}^{3} \mathrm{~s}^{-1}$, or about $6 \%$, lower than they would have been if the land use changes had not occurred. For comparison, the sensitivity studies in Section 4.2 indicated that a similar result could arise from a shift in mean rainfall of only $10-15 \%$ the fractional change in runoff, corresponding to a sustained change of only about $1 \%$ in the lake rainfall. These results suggest that any changes in lake levels due to land use change may be very difficult to disentangle from the natural variations in levels due to long term shifts in rainfall. Nevertheless, the changes are real and should be considered in planning any developments in the lake basin which are likely to have a significant effect on the total inflow into the lake. The likely cumulative effect of smaller developments should also be considered. Seasonal changes in the inflow pattern may also have some additional effect but a detailed assessment must await the development of process-based rainfall runoff models for the lake catchment which explicitly account for the impacts of land use change on runoff.

## Methods for assessing future impacts

To assess the impact of future land use changes on lake levels, it is necessary to consider first the changes which may arise from natural variations in climate alone. Several possible approaches could be used. The most obvious is to attempt to extrapolate the observed lake level or outflow records into the future or to develop stochastic models based on these records. However, an inspection of these two records suggests that any attempt to predict future levels based on these records alone will be doomed to failure due to the sudden jump in levels and outflows between 1961 and 1964. This jump either has to be removed before performing the simulations and then replaced afterwards, or a large and unrealistic linear trend component has to be accepted in the results. If the jump is removed, the question then arises of the appropriate timing, duration and magnitude of the jump which should be re-
inserted after the simulations have been completed. Some discussion of these problems and the results obtained using this approach is given by Kite (1982) and Salas (1982).

A more productive approach is to attempt to model the future behaviour of the net basin supply series. The simplest approach is to identify some portion of this record as being 'typical' of the sequence and to assign a subjective probability to the chance that this portion will re-occur in the future. The portion selected can then be replayed through a lake water balance model to estimate future lake levels. This is essentially the approach taken by Acres (1990) in their recent assessment of the future hydropower potential of Owen Falls dam (see Appendix A).

A more sophisticated approach is to develop stochastic models for the net basin supply. Each of the simulated sequences can then be run through a water balance model and, if sufficient realisations are performed, objective estimates can be made of the probability of future lake levels. This is the approach used in the two previous Institute of Hydrology studies (1984, 1985). Several options are available for modelling the net basin supply series. The simplest is to take the observed sequence and either to build up new sequences by random sampling, or to develop a model for generating new sequences based on the statistics of the observed sequence. The second of these approaches was used in the IH2 study. A more convincing approach, also used in the IH2 study, is to attempt to model the observed net basin supply series using rainfall data alone and a rainfall-runoff model. This then yields results which are independent of the accuracy of the observed lake level and outflow records, and is also perhaps the only plausible method for assessing the impact of land use or climate changes on the lake water balance. In the IH2 study, a multivariate model was developed which modelled the joint statistics of the 8 lakeshore rainfall stations used to estimate lake rainfall and the 5 catchment rainfall series used as input to the conceptual rainfall runoff model. Some allowance was also made for the observed serial correlation between monthly rainfall values.

The IH2 model was used mainly to estimate the likelihood of the Owen Falls dam being overtopped and the effects of possible alternative regulation plans for Lake Victoria. Some thought was given to using this model in the present studies but this was not thought to be necessary since such a complex model is normally only required for engineering design studies. It is also worth noting that the model was based on the statistics of the observed rainfall series, so provided these have not changed significantly since the 1925-78 period used in the IH2 study, the results of the simulations should still apply. Some comparisons with these results are made in the following section and several suggestions for future studies of this type are made in Section 7. Encouragingly, a visual comparison of the most recent rainfall data with the pre-1978 data (e.g. Figure 4.8, 5.11) suggests that there has not recently been any significant change in either the long term mean or the natural variability of this series.

After reviewing all available methods for estimating future lake levels, the method chosen in this study was an intermediate approach which should be sufficient to estimate the sensitivity of lake levels to land use change without necessarily providing statistically rigorous probabilities for the estimated levels. In fact, as will be shown, the results of the simulations agree surprising well with the results of the more sophisticated IH2 studies. The method uses the annual water balance model developed in Section 4.1 together with a large number of realisations of the net basin supply series based on the predicted net basin supply calculated using the linear inflow model. Each series is generated by sampling random years from the full 92 year lakeshore rainfall series and merging these years to form a new sequence.

This approach is valid provided that there is no significant serial correlation between consecutive years. This seems to be confirmed by Figure 6.2, which shows the dependence of the autocorrelation coefficient for this series on lag time ( N ) where the coefficient is defined as the lag $\mathbf{N}$ covariance divided by the variance of the series. Also shown are lines representing departures of two standard deviations from the mean. These lines give a scale against which to judge the degree of autocorrelation. It can be seen that there is no statistically significant relationship between adjacent years and that the autocorrelation varies randomly about zero as the lag time increases. For this preliminary assessment of the impact of land use change, it therefore seems reasonable to use unconstrained random sampling to build each new realisation of the net basin supply. It should be noted, however, that the autocorrelation coefficient is only one measure of the time dependence in a series and that a more detailed analysis might reveal additional features in the series. This point is discussed further in Section 7.

## Future levels in the absence of further land use change

Figure 6.3 shows an example of the results obtained from the annual water balance model for 1000 realisations assuming the runoff coefficient k remains at 0.16 throughout the simulation. The simulations were performed for a period of 50 years for a start level of 12.0 m , which is typical of the lake level in recent years. It was assumed throughout that releases from Owen Falls dam continue to be set according to the current Agreed Curve. The plot shows the levels which will be exceeded $10 \%, 50 \%$ and $90 \%$ of the time for each year in the simulation. Also shown are the maximum and minimum levels reached in any one year.

The results suggest that, after an adjustment period of 5-10 years, levels will tend to an equilibrium level consistent with the assumed exceedance probability. The equilibrium $10 \%$, $50 \%$ and $90 \%$ levels reached are about $12.14 \mathrm{~m}, 11.62 \mathrm{~m}$ and 11.13 m and the maximum and minimum equilibrium levels are 13.10 m and 10.38 m . For 1000 realisations, the maximum and minimum levels equate to non exceedance probabilities of $99.9 \%$ and $0.1 \%$ respectively. In any one year, the maximum level reached was 13.45 m and the minimum level reached was 10.08 m . It should be remembered that these are annual levels, so the instantaneous maximum and minimum levels in any one year would be more extreme than this. For comparison, in the period 1896 to 1991, the maximum and minimum observed annual mean lake levels were 10.45 m and 13.04 m . This suggests that the stochastic model must have successfully generated rainfall sequences in which several of the wettest and driest years from the observed sequence occurred in rapid succession.

Table 6.5 compares the results of this simulation with the equilibrium results from the IH2 multivariate model. Very similar response curves were also generated by this model. The IH2 estimates were presented in the form of the highest and lowest expected levels within a year and these values have been averaged for comparison with the present model. The agreement is reasonably good, especially considering that the models used different datasets and different stochastic generation techniques. The predicted median (50\%) levels agree to within about 0.1 m and even the extreme levels are at worst 0.3 m apart. The main difference is that the absolute maximum and minimum levels reached are about 0.6-0.7 m more extreme. Part of this difference is of course due to including a representation of the seasonal variations in rainfall, and hence levels, in the IH2 model. A repeat of the present simulations using the polynomial inflow model (Section 4.1) indicated similar median levels but differences of up to $0.4-0.8 \mathrm{~m}$ in the maximum/minimum levels in any one year.

Before describing the effect of changes in inflow on these results, it is helpful to consider the predicted median equilibrium levels a little further. If sufficient realisations are performed,


Figure 6.2

Predicted lake level non - exceedance probabilities for a start level of 12.0 m assuming no further land use change


| -- Minimum | --- 10\% | .......... $50 \%$ | --..-- 90\% | -.---- Maximum |
| :---: | :---: | :---: | :---: | :---: |

Figure 6.3

## Comparison of stochastic modelling results from the IH 2 and present studies

| Non exceedance <br> probability \% | Equil | Max/min | Highest | Lowest | Mean | Difference |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 10.48 | 10.28 | 10.30 | 9.86 | 10.08 | -0.30 |
| 1 | 10.72 | 10.58 | 10.61 | 10.22 | 10.42 | -0.30 |
| 10 | 11.13 | 11.08 | 11.09 | 10.68 | 10.89 | -0.24 |
| 50 | 11.62 | 11.58 | 11.71 | 11.28 | 11.50 | -0.12 |
| 90 | 12.14 | 12.28 | 12.35 | 11.90 | 12.13 | -0.01 |
| 99 | 12.65 | 12.78 | 12.88 | 12.40 | 12.64 | -0.01 |
| 99.9 | 13.10 | 13.48 | 13.28 | 12.80 | 13.04 | -0.06 |
| Max | 13.10 | 13.45 | 13.79 |  |  |  |
| Min | 10.38 | 10.08 |  | 9.41 |  |  |

the median equilibrium level should approach the level corresponding to a net basin supply equal to the mean of the net basin supply series used as input. For the period 1900-90, the mean net basin supply is about 0.40 m which would correspond to an equilibrium level of about 11.5 m using the current Agreed Curve, which is similar to the median equilibrium level generated by the stochastic model. Also, the time taken to reach this level was about 510 years as indicated by the response time estimates given in Section 4.2. Estimates of the median of future levels can therefore be made simply from inspection of the net basin supply series. For a completely random series, of course, the median level would also correspond to the most likely expected level.

From Figure 4.13, there have been several periods this century in which the net basin supply was more or less constant. Several possible future scenarios can be envisaged based on this record. Statistically, the most likely value for the net basin supply is the mean of the whole series which, in this case, is about 0.40 m , corresponding to an equilibrium level of about 11.5 m . If the 1961-64 rains are viewed as a freak event, with a return period much greater than 100 years, the net basin supply reduces to about 0.37 m corresponding to an equilibrium level of 11.4 m . If, as discussed in Sections 3.2 and 5.2, the 1961-64 period marked the start of a regional shift in rainfall patterns, then the net basin supply from that time is about 0.52 m , corresponding to an equilibrium level of about 12.0 m . Alternatively, the current net basin supply of about 0.48 m might continue for some years in which case the equilibrium level would tend towards 11.9 m .

For all these scenarios, the likely range only covers the levels 11.4 to 12.0 m , with corresponding outflows of about 800 to $1100 \mathrm{~m}^{3} \mathrm{~s}^{-1}$. The results presented in Section 4.2 indicate that the time taken to reach these levels from the current level of about 12.0 m would be about 10 years. The results presented in Table 6.5 should also apply to within an accuracy of about 0.3 m . However, it must again be emphasised that these results are only indicative and are not intended for input to engineering design studies. Also, the stochastic model takes no account of possible long term shifts in climate, such as that which appears to have occurred since 1961-64. For example, there is always the possibility of a shift to a completely new rainfall regime, or a return to the low rainfall conditions of the 1940s and 1950s. With current knowledge, it is not clear how such changes could be accounted for in estimating future lake levels.

## Impacts of land use change on future lake levels

These baseline results can now be used to estimate the likely impacts of future land use changes on future lake levels. For the first simulation, the best estimates given earlier were assumed, namely a reduction in inflows of about $1 \%$ per year up to the year 2000 and beyond. The simulations have only been performed for a planning horizon of 30 years because of the tentative nature of these estimates. The resulting non exceedance curves (Figure 6.4) are similar to those obtained in the absence of any change (Figure 6.3) but are shifted downwards by about 0.3 m after 30 years. The implied effect of these changes is therefore to cause a 0.1 m reduction in levels after 10 years over the levels which would have been obtained if no further changes in land use had taken place. In interpreting these results, it is of course worth remembering the results from Section 4 which suggest that similar changes would result from a sustained reduction of about $3-5 \%$ in mean rainfall over the full 30 years.

Overall, these results suggest that changes in land use are unlikely to have a severe impact over the type of planning horizon used in engineering design studies. Nevertheless, the impact is sufficient to cause a noticeable change in outflows and further studies would be worthwhile

Predicted lake level exceedance probabilities for a start level of 12.0 m assuming a $1 \% /$ year reduction in inflows



Figure 6.4
to obtain better estimates of past and planned land use changes in the lake catchment. Processbased models and more accurate land use assessments should also assist with obtaining better estimates of the impact of these changes on inflows to the lake. Some suggestions for future studies of this type are given in Section 7.

As a final comment, it is worth noting that all these results assume that releases from Owen Falls dam continue to be set according to the Agreed Curve. A discussion of alternative regulation plans falls outside the scope of this study. However, for general interest, some illustrative examples have been prepared showing the effects of operating the lake as a regulating reservoir, again assuming a start level of 12.0 m . Three simplistic plans have been assumed, in which outflows are set at $700 \mathrm{~m}^{3} \mathrm{~s}^{-1}, 900 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ and $1100 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ throughout the simulation. To avoid the lake draining down, releases have been constrained to follow the Agreed Curve if levels drop below an arbitrary level, taken here to be 10.5 m , which is roughly the minimum annual mean value observed this century. Again, only 200 realisations have been used in each simulation. The median equilibrium levels for these simulations are shown in Figure 6.5. The long term sustainable flow at current levels would appear to be slightly less than $900 \mathrm{~m}^{3} \mathrm{~s}^{-1}$. The $700 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ case causes a rapid and sustained rise in levels while a flow of $1100 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ causes levels to drop quickly down to the floor level specified. The $10 \%$ non exceedance lines are all close to the minimum level specified of 10.5 m , indicating that shortfalls in releases would have a return period of less than 1 in 10 years. In reality, of course, the operating policy would also allow outflows to be adjusted according to the levels reached. Some further discussion of more practical regulation plans is given by Kite (1984) and Okidi (1990).


| -700 cumecs $(50 \%)$ | $\cdots \cdots . . .900$ cumecs $(50 \%)$ | --1100 cumecs $(50 \%)$ |
| :--- | :--- | :--- |
| -700 cumecs $(10 \%)$ | $-\quad 900$ cumecs $(10 \%)$ | $--\cdots 1100$ cumecs $(10 \%)$ |

Figure 6.5

## 7. Discussion and conclusions

Several aspects of the hydrology of Lake Victoria have been investigated in this study using water balance models and regional comparisons of data. The main findings relate to the availability and quality of data for the lake and its catchment, the historical response of the lake this century and the sensitivity of the lake to changes in the main components in the water balance. A preliminary assessment has also been made of the likely impacts of past and future land use changes on lake levels.

The main results of this study are summarised below together with some suggestions for future work:

## Regional analysis of data

A regional analysis of rainfall and lake level data for East Africa shows a consistent picture over the whole of this century. Until 1960, rainfall over the region varied by only a few percent about the long term mean. Rainfall was below average for several years in the 1940s and 1950s. Unusually heavy rains in late 1961 were recorded as far afield as northern Kenya and Malawi and caused abrupt rises in several lakes in the region including Lake Victoria. Inflows to Lake Victoria also increased in 1961 and continued to rise until 1964, partly as a result of unusually high inflows from the Kagera in 1963 and 1964. Since 1964, there appears to have been a persistent region-wide increase of about $5-10 \%$ in rainfall due mainly due to increased rainfall in the October-December rainy season. The level of Lake Victoria has continued to be high (but slowly declining) due to this increased rainfall. There is also some evidence that lake levels were as high in 1878, but dropped rapidly in the next few years as a result of a prolonged period of low rainfall.

## The lake outflow record

The outflow from Lake Victoria approximately doubled as a result of the 1961-64 increase in lake levels and the Agreed Curve operating policy in use at the time. This change, and other smaller variations this century, is mirrored in all the Nile flow records down to the Sudd swamps and even downstream at Malakal. There is no evidence, as implied by the conclusions of the Acres (1990) study, that flows in the upper Nile dropped suddenly in 1954 with the onset of operations at Owen Falls dam.

As a further check on the outflow record and the Agreed Curve, detailed comparisons have been made of the recorded turbine/sluice releases at Owen Falls dam, contemporary discharge measurements at Namasagali and the outflows estimated from the Agreed Curve. This work suggests that, before 1961, discharges at Namasagali were possibly over-estimated by up to 5\%, suggesting a small error in the Agreed Curve in the opposite direction to that suggested by Acres. There is some evidence of an increase in these errors at the higher river levels since 1961 but this issue is irrelevant to discussions on the accuracy of the current Agreed Curve since this was established on the basis of measurements in the 1940s and early 1950s, and hydraulic model tests during the 1960s.

These comparisons also show that, on a monthly basis, there have often been large differences between the actual releases at Owen Falls dam and the releases required by the Agreed Curve operating policy. However, on an annual basis, these differences have usually been eliminated by making compensatory releases at a later date. These comparisons also confirm that, during the main construction phase of the dam (1952-53), flows were substantially reduced to allow
construction work to proceed. On the basis of these studies, revised outflow and observed net basin supply series have been estimated for use in evaluating the water balance models described below.

## Water balance models

Two types of water balance models have been used in this study; a simple annual model and a more sophisticated monthly model. The annual model has proved useful in sensitivity studies of the lake response and the monthly model has helped to improve understanding of the historic lake water balance.

The monthly model is based on the model developed in the IH2 study, but uses an alternative method for estimating the contribution from the lake rainfall. Rather than attempting to model the lake rainfall and evaporation separately, the net contribution - the net rainfall - has been modelled directly. Also, rather than estimating the individual contributions from each rainfall station, the model has been calibrated in terms of the average of all stations. The calibration period chosen was 1969-78. As part of this work, a new inflow record was produced based as far as possible on observed data for 5 key tributary flows. Missing data were infilled using a conceptual rainfall runoff model. The total inflow was estimated from these 5 flow records using a seasonally varying scaling method developed during the IH2 study. The annual model uses a simpler approach in which the lake rainfall is estimated by applying a single multiplying factor to the average lake shore rainfall. Annual runoff is assumed to scale on the lake rainfall either linearly or using a runoff coefficient which varies with rainfall.

## The water balance $1925-90$

Simulations for the period 1925-90 using both the IH2 and revised monthly models and the annual model show that satisfactory predictions of levels are obtained throughout, including the recent period 1979-90 not covered by the IH2 study. The revised monthly model has similar performance to the IH2 model in the period 1925-55 and slightly improved performance in the period 1956-90. A more stringent comparison in terms of net basin supply showed that both models accumulate errors of up to $1-2 \mathrm{~m}$ in equivalent depth over the lake in some periods, equivalent to a sustained error of about $5 \%$ in the lake rainfall in these periods. The most likely cause of these errors is that the model calibrations do not -and, with current knowledge, cannot - allow for occasional shifts in the seasonal pattern or spatial distribution of rainfall and evaporation over the lake, such as that which occurred following the rains of 1961-64.

The overall water balance, in terms of equivalent depth over the lake, gives an annual net rainfall of about 0.18 m , an inflow of 0.30 m , an outflow of 0.45 m and a net basin supply of 0.48 m for the period 1925-90. Values for the periods 1925-60 and 1965-90 are 0.12 m , $0.25 \mathrm{~m}, 0.36 \mathrm{~m}, 0.37 \mathrm{~m}$ and $0.21 \mathrm{~m}, 0.34 \mathrm{~m}, 0.57 \mathrm{~m}$ and 0.55 m respectively, showing that the net basin supply has increased by about 0.18 m , or 180 mm between the two periods. An examination of the monthly net rainfall series suggests that this change has occurred mainly in the October-December rainy season. This is clearly the reason for the relatively high lake levels since the rains of 1961-64.

## The water balance over the whole century

Using the annual water balance model it has been possible to relate the variations in the level of Lake Victoria over the whole century solely to variations in rainfall. These studies suggest that, up to 1925, the lake was in transition between a succession of equilibrium levels, causing levels to rise and fall in rapid succession. A stable period of low rainfall and net basin supply then persisted up to 1960 . Following the rise of $1961-64$, levels have been in
a slow decline towards a new equilibrium level corresponding to the current net basin supply, which is about $0.1-0.2 \mathrm{~m}$ above pre-1961 values. The more sophisticated monthly water balance studies from 1925 also achieve a good match between predicted and observed lake levels using only lakeshore and catchment rainfall data as input. There is therefore no need to hypothesise any significant influence on levels from Owen Falls dam as some previous studies have done.

## Sensitivity studies

Modelling studies confirm previous results which have shown the extreme sensitivity of the lake to fluctuations in the lake rainfall. An approximate model suggests that, for a given change in lake rainfall $-z$ metres say - sustained over a period of several years, levels will change by about 5 z metres, outflows by about 2500 z (in $\mathrm{m}^{3} \mathrm{~s}^{-1}$ ) and potential power output by about 375 z (in MW). So, for example, a sustained 100 mm increase in lake rainfall, equivalent to an increase of about $5 \%$, would cause a rise of about 0.5 m in levels, $250 \mathrm{~m}^{3} \mathrm{~s}^{-1}$ in outflows and (potentially) a 37.5 MW increase in hydropower output.

These approximate results are largely confirmed by the monthly and annual water balance models. Changes in level seem to occur over a timescale of about 10 years at current levels, this being the time taken for levels to rise or fall to values which cannot be easily distinguished from the natural seasonal variations in lake level. This response time increases as lake levels drop, and is about 20 years at levels of 10 m .

These studies also suggest that levels are much less sensitive to changes in tributary inflows. A given small percentage change in inflow causes approximately $10-15 \%$ of the change in level for an equal percentage change in lake rainfall. So, for example, a sustained $5 \%$ increase in rainfall would have roughly the same effect as a sustained $35-50 \%$ increase in inflow. The implication of this result is that it may be difficult to disentangle the effects of land use changes on levels from the effect of long term changes in rainfall.

## Future lake levels

To provide a baseline against which to judge the impacts of land use change, future lake levels were estimated using the annual water balance model and a simple stochastic rainfall generation model. These simulations suggest that the most likely outcome is that lake levels will decline over the next few years to an equilibrium level of about 11.5 m , corresponding to an outflow of about $850 \mathrm{~m}^{3} \mathrm{~s}^{-1}$. Slightly different results are obtained if the rains of 1961-64 are viewed as a freak event or as marking a change in climate; however, the overall range is still only $11.4-12.0 \mathrm{~m}$. Absolute maximum and minimum levels in any one year are about 10.1 m and 13.5 m . These results agree reasonably with the much more sophisticated stochastic simulations performed in the IH2 study.

## Impacts of land use change

A preliminary review has assessed the likely impacts of several types of land use change on inflows into the lake. From the limited data available, the most likely causes of changed inflows are deforestation in the headwaters of the Kagera and Kenyan tributaries, increased irrigation, and new hydropower developments, particularly in the Kagera basin. The overall effect of these changes to date was assessed as a reduction in inflows of about $10 \%$. The water balance simulations suggest that this has caused lake levels to be about $0.1-0.2 \mathrm{~m}$ lower than they would have been in the absence of any change. The high lake levels of recent years therefore cannot be attributed to the effects of land use change.

The rate of change appears to be accelerating and was estimated to be currently equivalent
to a reduction in inflows of about $1 \%$ per year. The stochastic simulations of future levels were repeated assuming that reductions continue at this rate for the next 30 years. The overall drop in levels, compared to the case of no further change, was about 0.3 m . It must be emphasised, however, that these estimates are very approximate, and are only intended to provide a starting point for a more detailed assessment.

## Suggestions for future work

This study has suggested several topics which could lead to a better understanding of the hydrology of Lake Victoria:
(a) Impacts of land use change

The work described in this report provides a useful starting point for more detailed studies of the impacts of land use change on Lake Victoria. The two priorities now are to improve the estimates of current and projected land use in the catchment and to develop process-based models which can give better estimates of the impact on inflows. Archived maps, aerial photos and satellite images could all be used to quantify changes in land use over this century. Future changes could be estimated by contacting planning authorities in the lake catchment and reviewing reports on proposed developments. The development of process based models is one of the main objectives of the wider "Water Balance of African Lakes" study referred to in the introduction to this report.
(b) Net rainfall studies

The main uncertainty in the lake water balance is the net contribution of rainfall and evaporation. The priority is to re-establish the lakeshore and lake island meteorological stations established during the Hydromet Survey project, together with the tributary gauging stations. This might be possible within the framework of the existing Hydromet Survey project.

Satellite images of cloud cover, possibly combined with atmospheric circulation models, could also improve understanding of the distribution of rainfall over the lake. Direct estimates of rainfall could also be obtained from remotely sensed estimates of cloud temperature (cold cloud duration), although some research work would be needed to apply this technique over a large water surface with its own climate like Lake Victoria. Similarly, it might be possible to estimate the distribution of evaporation from remotely sensed estimates of water temperature. Modern turbulence measurement techniques, such as those recently used by the Institute of Hydrology over a tropical lake in Indonesia, could also be used to directly measure lake evaporation in order to calibrate longer term Penman estimates of evaporation. The emphasis in all these studies should be on assessing the variations from year to year, rather than obtaining only instantaneous estimates. The emphasis should therefore be on long term monitoring of the behaviour of the lake.
(c) Water balance of the upper Nile

A better understanding of the hydrology of the upper Nile basin could improve confidence in the outflow record for Lake Victoria, as well as being an interesting and useful research topic. There is also the possibility of using downstream stations such as Mongalla as a basis for estimating the true lake outflow record before 1940.

A mixture of water balance models, channel routing models and rainfall runoff models could
be used to model each of the main lakes and rivers in the system. A review of similar previous studies could guide the construction of an appropriate model for the whole basin. The model could be used to improve understanding of the historic variations in flows and levels in the Upper Nile and for feasibility studies of alternative hydropower developments and regulation plans for the Upper Nile. Particular issues might be the likely impacts on the levels of Lakes Victoria, Kyoga and Albert and the implications on the flow regime in the Sudd.

## (d) Implications for hydropower development

To provide a basis for future planning, it would be interesting to use the projections of water levels and flows from this study and the IH2 study to assess the long term hydropower potential for the Owen Falls dam site. Comparisons with previous studies could also be made although - in the case of the Acres study - this would require considerable additional information on the methods and data series used. Alternative dam sites and regulation plans might also be investigated and, with input from an economist, least cost development options could be identified. The long standing question of the need for a spillway at Owen Falls dam could also be considered further. The projections of future levels could also be refined further using the methods outlined in the next section.

## (e) Improved estimates of future lake levels

The approximate stochastic simulations described in this report were intended to provide a basis for estimating the changes in levels due to land use change, and not to provide rigorous statistical estimates of lake levels for use in engineering design studies. The IH2 studies remain the most comprehensive assessment of likely future lake levels. Future studies could include a more detailed review of the stochastic models used in the IH2 study and an update of the simulations to include the data from 1979 to the present. The review should include an assessment of the degree of serial correlation (persistence) in the net basin supply and lake rainfall series and the evidence for periodicity in these series. Also, the possible impacts of land use change and current climate change scenarios should be considered. Some thought would also need to be given as to the likelihood of future regional shifts in climate such as that which occurred following the rains of 1961-64. Possible future variations in lake water temperature should also be considered since this affects both the lake rainfall and evaporation.

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# Appendix $A$ Review of the hydrological aspects of the Acres report 


#### Abstract

The Acres International study (see Section 2.4) is to our knowledge the only major study of the hydrology of Lake Victoria in recent years. However, the conclusions reached about some aspects of the lake water balance were radically different to those reached in any previous work. As part of the present study, a careful assessment has been made of the findings of the Acres work as they relate to the water balance of Lake Victoria. The main conclusions from this assessment are summarised in this Appendix. The operational and planning aspects of the Acres study are not reviewed here except where they affect understanding of the water balance of the lake.


## A. 1 SUMMARY OF THE ACRES FINDINGS

The Acres studies aimed to identify a 'reference hydrology' for Lake Victoria and the upper Nile defined as a monthly series of the 'inflow available for outflow' (i.e. net basin supply) from which the firm energy potential of Owen Falls and sites downstream could be evaluated. Net basin supply or 'inflow available for outflow' was defined as the sum of observed or estimated lake outflows and observed changes in lake storage. Until 1954, outflows were estimated from the Agreed Curve and thereafter were based on recorded releases (where available) from the Owen Falls dam. Generally the hydrological studies were limited to a review of previous studies, primarily those by WMO (1974) and IH (1984 and 1985), but new work was presented on the derivation of the Agreed Curve.

Acres reached the conclusion that lake outflows before the construction of Owen Falls dam were unreliable and have in the past been seriously underestimated. They concluded that decisions on hydropower development should therefore be based on the higher net basin supply series from 1961 onwards, being representative of the true long term flow regime. Acres estimated that the likelihood of a return to outflows as low as those 'observed' in the period 1900-60 is very small, especially as they also contend that the current Agreed Curve is not a reasonable representation of the natural rating curve for Ripon Falls prior to construction of Owen Falls dam.

These conclusions were supported by the following evidence and interpretation of the results of previous studies:

1. Synthetic rating curves for the pre-construction geometry of Ripon Falls were presented to show that outflows derived by the Agreed Curve could be underestimated by between $19 \%$ and $65 \%$. These synthetic ratings were based on hydraulic theory applied to the historical cross-section of the falls insofar as the original profile could be determined.
2. Acres interpretation of the Namasagali and Mbulamuti ratings and the history of Owen Falls construction (cofferdam) and Ripon Falls excavation suggest that the original rating curve for Ripon Falls seriously underestimated the actual lake outflows.
3. The apparent inhomogeneity of the basin supply series since 1900 (i.e. a step change in 1961) and the inability of the WMO (1982) study to draw up a satisfactory water balance description of the lake behaviour over the period 1950-79 were cited as evidence of serious errors in one of the components of the lake balance.
4. Comparative data from other lakes, other rivers and other gauging stations on the Nile were presented to show that the Lake Victoria net basin supply series should be homogeneous i.e. with no sudden changes in the long term mean. This issue was amplified by a discussion of several possible explanations for changes in the observed net basin supply and especially the substantial rise in lake level in the early 1960s, involving both natural and man-made events.

In their field work, Acres were able to confirm that the gauge at Jinja has not been altered over time and that the record of lake levels is consistent over the whole of this century.

## A. 2 IMPLICATIONS OF THE ACRES ANALYSIS

Although Acres do not present all the implications of their hypothesis that outflows in the early years have been substantially underestimated, it is possible to imply the following:

1. A description of the lake balance (such as that reported in the IH 1 and IH 2 studies) should not be possible using the historical data.
2. If the IH 2 water balance is accepted as broadly correct, then rainfall in the early period must be increased significantly relative to the later period.
3. The lake has not drained down to pre-1961 levels because the Agreed Curve (based on the original Ripon Falls curve) does not allow sufficient outflow to give the expected lake level decline.
4. Observed flows at lower stations on the White Nile, particularly at Mongalla, must have been adjusted historically to give a series consistent with the lake outflow.
5. Substantial increases in outflow would be permissible if the Agreed Curve were to be revised to the 'true' natural curve.
6. Predictions of future lake response based on the use of stochastic rainfall models fitted to the full historical record cannot be valid because of the inhomogeneity of the rainfall or net basin supply series.

Gibb (1990) noted in addition that, if the Acres hypothesis were correct, there should have been a noticeable drop in lake levels and flows in the Upper Nile basin in 1954 when the incorrect Agreed Curve started to be used at the start of operations at Owen Falls dam.

## A. 3 COMMENTARY ON THE ACRES FINDINGS

## A.3.1 General Issues

The Acres findings are very far reaching in their effects and merit careful consideration given
the possible investments in hydropower development on the Nile in Uganda over the coming decades and the important contribution the lake outflows make to flows in the main Nile down to Cairo. Acceptance of the high flow regime since 1961 as the basis for planning could result in substantial overestimation of the potential generating capacity at Owen Falls and elsewhere if the Acres findings are not valid. The average flow recommended by Acres is nearly double the average that persisted according to the records over the first five decades of this century.

An accurate description of the hydrology of Lake Victoria and the upper Nile is not straightforward. Otherwise many expensive studies would not have been necessary to try to explain the changes in regime that have taken place, especially the sudden rise in lake levels in the early 1960s. There are uncertainties in all the data, particularly for the components on the inflow side of the lake water balance. For example, estimates of lake rainfall must be derived from the few gauges that have continuous records dating back to the mid-1920s. Flows in the main tributaries excepting the Kagera were not measured until the mid 1950s. Outflows are hindcast from the rating of Ripon Falls based on measurements of flow at the Namasagali station some 75 km downstream in the 1940s and 1950s. The measurements at Namasagali and later at Mbulamuti were made under difficult conditions at sites that are less than ideal. Evaporation is implied from meteorological data from only a few shore stations. Only the lake levels recorded at a number of stations since the late 1800s are generally accepted as accurate.

The excess of rainfall over evaporation is small. Consequently the net basin supply is very sensitive to small changes in rainfall. This effect is further magnified as tributary inflows increase in wet periods and decrease in dry periods, and the lake evaporation is likely to decrease in wet periods and increase in dry periods. It is therefore not surprising that the net basin supply series should have exhibited some unusual variations over the decades as rainfall varied both annually and in its seasonal and spatial distribution. All these considerations make it vital that projections into the future are based on as good a description as possible of the water balance of the lake over the full period for which records are available.

One further general point is that the immense storage in the lake means that lake levels and hence outflows respond slowly to changes in the rainfall regime. Thus the expected flows over short time scales, say up to $10-15$ years, must be conditional on the starting level and outflow.

## A.3.2 The Agreed Curve

In the Acres report, one of the main reasons cited for doubting the Agreed Curve is the instability of the river section at Namasagali due to shifting control, unsuitable cross-section and possible backwater effects from Lake Kyoga. Except to the extent that these considerations make direct flow measurements difficult, they are largely irrelevant. It is not the stage-discharge curve for Namasagali that is at issue. The flow measurements at Namasagali are taken as a surrogate for flows at Ripon Falls, and are related to lake levels measured on the Jinja gauge. Thus it is the stability of the historical rating of Ripon Falls (i.e. the Agreed Curve) that should be considered and nobody has suggested that this is an issue. There might be some difference between the rating before construction of Owen Falls dam and afterwards as implied by the results of the hydraulic modelling carried out by Acres and discussed below. But this does not invalidate the original Agreed Curve developed from flow measurements at Namasagali mainly in the 1940s and early 1950s. The checks made in Section 3.3 of this report show that there can be little doubt about the accuracy of the discharge measurements themselves, and that the errors in these measurements were most
likely of the order $5 \%$ in the 1940s and early 1950s, and in the opposite direction to that which Acres suggest.

In their discussion of the Agreed Curve, Acres reproduce the graph of observed discharges against lake level originally compiled by the WMO (1982) study. This shows that the Agreed Curve passes through the mass of points relating to discharge measurements at Namasagali up to 1948 , corresponding to lake levels up to about 12.0 m . The Agreed Curve also passes through the points relating to discharge measurements at the nearby station of Mbulamuti in the period 1970 to 1972 when lake levels were in the range 12.0 to 12.5 m . However many of the discharges relating to lake levels above 12.5 m in the period 1968-70 were higher than the curve would suggest.

Acres rightly point out that all observations after the construction of Owen Falls dam should lie on the Agreed Curve as, with minor exceptions, outflows have been controlled to follow the Agreed Curve (see Section 3.3). The 1969 and 1970 gaugings at Namasagali therefore indicate either that flows exceeded (and were allowed to exceed) the Agreed Curve in this period or that the measured discharges were all systematically too high. The first possibility could also suggest that the sluice and turbine ratings at Owen Falls are in error if the excess flows were believed to lie on the Agreed Curve; however, Acres own analysis suggests these ratings overestimate the actual flow by only about $2.4 \%$ and the analyses described in Section 3.3 suggest a difference of only about $5 \%$ between these ratings and flows measured at Namasagali in the 1950s. The second possibility could suggest (as Acres imply) that for some reason observations at both Namasagali and Mbulumati were in error in the same direction at this time. This could possibly be due to some modification of velocity profiles at the higher water levels after 1961. The analyses described in Section 3.3 suggest that the Namasagali measurements at this time might have been about $10 \%$ too high in this period, but that errors in the Mbulamuti measurements were much smaller.

A firm conclusion about this issue could only be reached by locating the daily release records at Owen Falls dam for the periods in which the gauging measurements were made (if these are still available) and then making direct comparisons on a day by day basis. In this context it is interesting that synthetic Ripon Falls rating curves developed by Acres using the HEC-2 hydraulic modelling software suggest that the rating should flatten off to a greater extent than the actual Agreed Curve at high lake levels, indicating that the Agreed Curve does not correctly represent the outflows which would have occurred from the undisturbed Ripon Falls. However, Acres also concluded from these studies that the Agreed Curve underestimated flows by between $19 \%$ and $65 \%$ at pre-1960 lake levels.

Gibb (1990) also attempted to derive synthetic rating curves for Ripon Falls using hydraulic theory applied to the natural (pre-1951) and present geometry of the falls. Gibb treated Ripon Falls as a compound broad-crested weir and obtained a rating curve that indicated flows about $9 \%$ lower than the Agreed Curve when the geometry of the falls was based on 1956 survey data. This rating curve showed reasonable agreement with the Agreed Curve if some deepening of the west channel was made in the model to compensate for the known obstructions by cofferdam material following construction of Owen Falls dam.

Cassidy (1991) reviewed these calculations along with simultaneous flow measurements at the dam and at Namasagali for 1955 and 1956 and reached the following interim conclusions:

1. The HEC-2 model used by Acres to determine the critical depth of flow over Ripon Falls together with the water surface profile upstream of the falls is inappropriate for
a complicated river section such as that at Ripon Falls. HEC-2, developed by the US Army Corps of Engineers, assumes one-dimensional gradually varied flow in a uniform cross-section.
2. The weir discharge coefficient used by Gibb is at the theoretical upper limit for a broad-crested weir. Thus the flows estimated for a given lake level are also an upper limit. It is therefore unlikely that underestimates of the order of $30 \%$ can be envisaged as Acres suggest.
3. Estimates of flows at the dam agree to within a few percent with the observed flows at Namasagali based on observations in 1955 and 1956.

In reaching his overall conclusion that the Agreed Curve was confirmed to within a few percent, Cassidy placed most weight on Item 3 above which indicates that gaugings at Namasagali at that time were broadly correct.

Without further survey and gauging, it is impossible to reach a final conclusion on this issue. However, if there are errors in the Agreed Curve - in the sense that it does not fully represent the natural rating of Ripon Falls before construction of Owen Falls dam - it seems much more likely that these errors are at the upper end rather than at the lower end as suggested by Acres (see also Section 3.3).

## A.3.3 Comparison With Nile Flows At Downstream Stations

Acres compare the trends in outflows from Lake Victoria with flows at several long term gauging stations in West, Central and East Africa, all of which lie within the typical range of movement of the Intertropical Convergence Zone. The comparisons are made on a cumulative flow basis and the conclusion is reached that only the Lake Victoria outflow record and, possibly, the Kagera record, show significant unexplained changes in slope. This is taken as evidence of a possible error in the Lake Victoria outflow record. However, given the complexity of climatic influences over the vast region of the Nile basin and the equatorial belt, it would be irrationally optimistic to expect all hydrological series to show similar fluctuations over time. Anomalous behaviour is commonly observed in flows of the larger rivers in Africa. The anomalies usually cover different periods and by implication they have different, though possibly inter-related, causes.

Of more importance is a comparison with flows observed at stations lying between Lake Victoria and the Blue Nile, of which Mongalla is the principal station. Acres comment that the cumulative mass curve of flow at Mongalla closely resembles that of the Lake Victoria outflows (e.g. Figure 2.7). This is not surprising since by far the major proportion of the flow at Mongalla originates from the lake basin; however, Acres suggest that the Mongalla record has possibly been adjusted in the past to accord with that of the lake outflows. It is extremely unlikely, though, that adjustments on the scale required by the Acres hypothesis would have been made without considerable reported analysis of the cause of error in some of the many technical papers, books or conference proceedings concerning the hydrology of the Nile.

Both IH (1985) and Gibb (1990), quoting many past and contemporary sources, review the historical evidence for changes in the flow regime of the White Nile, the Bahr el Jebel and the variation of the area covered by the Sudd. Additional results are also presented in Section 3.3 of this report. While some of the Nile flow data may be in error by a small margin
representing the difficulties of gauging, there is no doubt that the general increase in flows in recent decades is real and not the result of substantial errors introduced by the acceptance of an erroneous Agreed Curve. Also, as Gibb point out, there is no evidence of a sudden drop in flows in 1954 when the lake releases started to be determined by the supposedly incorrect Agreed Curve.

## A.3.4 Variations in lake rainfall

Acres repeatedly comment that the slope of the cumulative mass curves for lake rainfall and net basin supply are constant apart from discontinuities in the early 1960s. From these studies they conclude that the net basin supply as a function of rainfall is greater in the later period, a result that must be due to errors in the measurement of outflow in the early period in the absence of any more plausible explanation.

That Acres should reach this view is difficult to understand. Figure A. 1 shows the IH2 lake rainfall series which is similar to the series from the individual gauges and to the series derived by de Baulny and Baker (1970) and used by Acres. The de Baulny and Baker series gives mean annual rainfalls for the periods 1925-60 and 1965-78 of 1634 mm and 1734 mm respectively while the IH 2 series based on equal weights applied to the standardised eightgauge series gave 1671 mm and 1793 mm for the same periods, and 1758 mm for the period 1965 to 1989 . Similar results are also obtained using the annual lake rainfall model and the monthly net rainfall model developed in this study. The additional 122 mm of rainfall in the period $1965-78$ is sufficient to increase net basin supply by $8,100 \mathrm{MCM} / \mathrm{year}$, or about $250 \mathrm{~m}^{3} \mathrm{~s}^{-1}$, which alone explains about half of the observed increase in outflow. Much of the remaining difference is explained by the higher tributary inflows that followed from a regional increase in rainfall. The water balance studies described in Sections 4 and 5 of this report clearly show that a satisfactory water balance can be achieved for the whole century if proper account is taken of this increase in tributary inflows and the differences between rainfall over the lake and that measured at the lakeshore.

That rainfall has increased in the lake basin since 1961-64 is not in doubt. Farmer (1981) attributes this increase primarily to an increase in rainfall during the October and November wet season and this observation seems to be confirmed by the results reported in Section 5.2 of this report. Climatologists talk in terms of anomalies, hydrologists and statisticians in terms of homogeneity of records. Acres appear to take a narrow definition of homogeneity implying that the time series of rainfall should have a stationary mean (and presumably variance) over time-scales of a few decades or less. By increasing the lake outflows substantially in the period before 1960 it is possible to equalise the mean outflow with that observed after 1965. While this might give the impression of homogeneity, it cannot be used as an argument to justify the adjustment of the data, especially when there has been a real increase in rainfall. We believe that the descriptions and hypotheses should fit the data, not that the data should be adjusted to fit the hypotheses.

## A.3.5 Implications on the water balance

It is unfortunate that Acres did not extend their analysis of a possible error in the pre-Owen Falls outflows to a review of the lake water balance. They relied heavily on the findings of the WMO study (Kite, 1981) that the full rise in lake levels in the early 1960s could not be reproduced by a water balance model and that this was possibly because portions of the available rainfall data were not consistent. They placed little emphasis on the two IH studies and other studies (cited in this report) which have shown that, by using more plausible


Figure A. 1
Comparative curves of cumulative net basin supply


Figure A. 2
modelling assumptions, the lake response can be explained by water balance principles alone. In fact Acres cite as evidence a supposed irregular pattern of rainfall adjustments that was needed by IH to achieve a satisfactory lake water balance. This belief derives from the IH1 study, but the later IH2 study placed great emphasis on developing a consistent way in which a revised lake rainfall series could be derived and there were no irregular adjustments of rainfall.

Instead of a lake water balance, Acres presentation relies on the visual comparison of cumulative flow and rainfall curves with arguably misleading results. Comparisons of the same data on a time series basis leads to very different conclusions (see Sections 2.4 and 3.3). It is self-evident that the net basin supply, derived from observed outflows and changes in lake level, has been higher since 1964 than it was before 1961. It is also evident that the net basin supply estimated from rainfall, evaporation and tributary inflows has also been higher (see Sections 4 and 5). Acres' statement that the slope of the lake rainfall mass curve remains constant over the period of record except for a step in the early 1960s is incorrect, even when it is based on the de Baulny and Baker rainfall series. Irrespective of the rainfall series used, lake rainfall in the period from 1965 is at least 100 mm higher per year than it was in the period 1925-60. This is equivalent to at least $7,000 \mathrm{MCM}$ per year in the net basin supply, not allowing for the simultaneous increases in tributary inflows or any amplifying effect due to changes in evaporation.

Figure A. 2 shows the impact of the Acres conclusions on the cumulative net basin supply series, and compares the inflow series derived from rainfall, evaporation and tributary inflows with the outflow series derived from outflows and changes in lake storage. Curves are shown for the observed data and for cases where the outflows before the construction of Owen Falls have been increased arbitrarily by $30 \%$ and $65 \%$ respectively. The curves based on the range of increase in outflows presented by Acres lie substantially above the curves for net basin supply based on rainfall and inflows. The implication of the Acres work is that if the lake outflows were substantially underestimated in the years up to the mid 1950s, the rainfall must have been significantly underestimated in the same period. There is little evidence of this, irrespective of the method used to derive lake rainfall from the eight rainfall series and from other rainfall series for the whole lake catchment and East Africa (see Section 3.2).

It is also interesting to repeat the monthly water balance simulations of Section 5.2 assuming that the Acres hypothesis is correct. Figure A. 3 shows the predicted levels using the revised water balance model and assuming that outflows were underestimated by $30 \%$ or $65 \%$ in the period up to 1954. The effect on levels is roughly as expected, with a reduction in predicted levels of about 0.3 m for the $30 \%$ change and about 0.6 m for the $65 \%$ change in the period up to 1954. For the $30 \%$ change, the agreement between predicted and observed levels is slightly worse than for the case of no adjustment to the outflows (Figure 5.16) while, for the $65 \%$ change, the agreement is significantly worse.

A more stringent comparison is shown in Figure A. 4 and Table A. 1 for the case of a $30 \%$ increase in outflows. To facilitate comparison with Tables 5.1-5.3 and Figures 5.4 and 5.17 of this report, these results have been computed using the IH2 models for the lake rainfall and tributary inflows. To distinguish the resulting lake rainfall series from series Rain A and B developed in Section 5.1, the series based on the assumed increase in outflows has been called Rain C here. The main effect of the change in outflows is to increase the errors accumulated in the period 1925-34, while producing a slight improvement in the period 1935-54. The accuracy of the water balance would only be improved if the period 1925-34 were to be omitted from the analysis. These results are therefore somewhat inconclusive due to the

Water levels predicted using the revised monthly water balance model and assuming ouflows are increased by $30 \%$ and $65 \%$ up to 1954



Figure A. 3

Cumulative errors in net basin supply assuming outflows are increased by $30 \%$ and $65 \%$


Figure A. 4

## Lake balance errors expressed in terms of the component variables

| Period |  | ${ }_{\text {mean }}^{1900-24}$ | $\begin{gathered} \text { 1925-34 } \\ \text { mean } \end{gathered}$ | $\begin{gathered} 1935-55 \\ \text { mean } \end{gathered}$ | $\begin{gathered} 1956-60 \\ \text { mean } \end{gathered}$ | 1961-64 mean | $1965-78$ <br> mean | 1979-89 mean | 1965-89 mean | $\begin{gathered} 1956-78 \\ \text { mean } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Basin supply (inflow) | mcm |  | 19941 | 30608 | 23020 | 68260 | 40170 | 35616 | 38166 | 41327 |
| Basin supply (outflow) | mcm | 25688 | 30087 | 27286 | 20011 | 72517 | 38114 | 30859 | 34922 | 40162 |
| Error in water balance BS(Infiow) - BS(outflow) | mcm |  | -10146 | 3322 | 3009 | -4257 | 2056 | 4757 | 3244 | 1165 |
| Error as percentage of: |  |  |  |  |  |  |  |  |  |  |
| Outfiow | \% |  | 38 | -12 | -11 | 22 | -5 | -12 | -9 | -3 |
| Lake rainfall (Rain C) | \% |  | -9 | 3 | 3 | 3 | 2 | 4 | 3 | 1 |
| Tributary Inflow | \% |  | -67 | 17 | 18 | -14 | 9 | 20 | 14 | 5 |
| Lake evaporation | \% |  | 10 | -3 | -3 | 4 | -2 | -4 | -3 | -1 |
| Notes: <br> A positive percentage error indicates a possible overestimate of the relevant variable. In this case outfiows have been increased by $30 \%$ up to 1955. Other variables are assumed to be accurate. |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |

Table A. 1
uncertainties inherent in the water balance; for example, in the period up to 1954, the change in levels caused by a $30 \%$ error in outflows is similar to that resulting from only a $5 \%$ error in the rainfall or evaporation components of the water balance. As noted in Section 5.1, a small error in rainfall is much more likely than a large (and previously unreported) error in outflows.

## A.3.6 Operational analysis

A detailed assessment of the operational aspects of the Acres study falls outside the scope of the present study. However, some comments can be made based on the results of the IH2 stochastic modelling studies and the additional studies described in Section 6.2 of this report.

For their long term yield assessments, Acres selected the 1961 to 1989 outflows (or net basin supply) as representing the long term average flows available for power generation. While they state that this length of record is normally sufficient for planning purposes, it appears that the reference hydrology was extended to a 40 year period by adjusting the outflows in the 1949 to 1961 period to conform with the mean of the original 29 year reference period. This is a curious starting position for what were rather elaborate studies of alternative hydropower schemes that included the active operation of the lake as a reservoir. Given only the indisputable evidence of the variations in the rainfall regime, it is unjustifiable to select only the wetter period of the record as a basis for operational and planning studies. In any event the selection of a record as short as this is unrealistic when there are apparent long term changes in the inflows, and when the effect of the large lake storage can persist over a decade or more. If Acres had confidence in their adjustment procedure for the period from 1949 to 1961, it is unclear why they did not adjust the whole of the early record so as to use the whole of the available information in their reservoir trials.

Some account is taken of the earlier period of lower outflows and the operational analysis initially accorded a probability of $1 \%$ to the return to this low flow regime. This appears to be no more than the relative frequency accorded to the lowest flow in a 90 year record which would be valid if the flows could be considered to be independent. But there are dependencies in the data, particularly in the short term, that combine to produce some longer term dependencies in the net basin supply. In addition, the IH2 and present analyses show that predicted lake levels (and therefore outflows under the Agreed Curve operation), over a time horizon of the order of $10-15$ years are dependent on the assumed starting lake level. This is the kind of timescale over which economic analyses are most sensitive.

Little of the statistical analysis is presented in the Acres report and it is therefore difficult to make direct comparisons with the projections developed in the IH2 and present study. However it is possible to assess the probability of exceedance of the outflow on which the firm energy would be based assuming that outflows continue to be determined by the Agreed Curve. Acres seem to define the Firm Energy flow as the monthly flow with a 1 in 10 year return period, which is effectively the 1 in 10 year lowest monthly flow in the sequence used. Acres do not present the sequence actually used in their reservoir trials and the detail of the flow adjustments made for the 1949 to 1960 data is not known. Because of the marked change in lake levels and outflows that took place at the end of 1961 the value of the critical monthly low flow depends strongly on the start month of the sequence and the adjustments made. Nevertheless from the results of the Acres study we can estimate that the Firm Energy flow as defined is in the range 550 to $650 \mathrm{~m}^{3} \mathrm{~s}^{-1}$. The results of the IH2 study suggest a probability of non-exceedance on an annual basis of roughly $20 \%$ and $37 \%$ respectively for these monthly flows. These figures compare directly with the $10 \%$ risk assumed by Acres.

Taking a broader view, Acres are correct in assuming that high flow conditions will persist for some years simply because the lake is currently at a relatively high level. The exploratory stochastic studies described in Section 6.2 suggest that the most likely future outcome based on data for this century is a slow drop in levels over the next 10 years or so to reach a mean level of about 11.5 m , corresponding to an outflow of about $850 \mathrm{~m}^{3} \mathrm{~s}^{-1}$. Since the Owen Falls dam was designed during a period when outflows were typically varying in the range 500-900 $\mathrm{m}^{3} \mathrm{~s}^{-1}$, there is therefore some benefit in considering the economics of increasing the installed capacity at Owen Falls to take advantage of this increased mean outflow.

The Acres' assumption of a very low probability of flows returning sooner or later to a much lower level depends critically on their hypothesis that the Agreed Curve is wrong and that the pre-1954 flows were seriously underestimated. Indeed, the sensitivity studies and stochastic simulations described in Sections 4 and 6.2 of this report show that, viewed over timescales of a few years or more, lake levels can drop as rapidly as they rise and changes of the order $1-2 \mathrm{~m}$ within less than a decade have occurred several times this century and also following the peak of 1878 . These studies also show that Acres view of a constant net basin supply in recent years is overly simplified and can give misleading views of the future lake response.

The results of the present study suggest that a more detailed stochastic assessment of future lake levels would have to consider several factors, including the serial correlation present in the lake rainfall series, the statistical relationships between lake rainfall and catchment rainfall, the possibilities of future distinct shifts in climate (as occurred in the early 1960s), the impact of changes in lake temperature and the possible future impacts of land use change and, in particular, irrigation and hydropower schemes in the lake catchment.

## A. 4 CONCLUSIONS

We believe that it is unreasonable to propose any hypothesis regarding errors in data or rejection of data or selection of a subset of the data as representative of the long term condition, unless the proposed change adds to our ability to describe the hydrology of the lake over the period for which data are available. The simpler the solution to the water balance, the more likely it is to be accepted, and estimates for the lake rainfall, tributary inflows and, indeed, outflows should be derived in a consistent way throughout the historical period.

We have demonstrated by annual and monthly water balances using carefully derived data sets that a realistic balance exists and that adjustment of the outflows in the early period would disrupt that balance, if the adjustments are on the scale suggested by Acres. There is no doubt that the lake rainfall and tributary inflows increased substantially in the period 1961-64, and that this increase was responsible for the relatively large rise in lake levels in those years. But it is also beyond doubt that the average annual lake rainfall has been higher in the subsequent period than it was in the period 1925-60. This largely explains the higher net basin supply in recent decades and hence the ability to sustain higher outflows without rapid drops in lake level.

There is little, if any, evidence that positively supports the hypothesis that outflows derived from lake levels using the Agreed Curve are seriously underestimated. We showed in Sections 5.1 and A.3.5 that the lake water balance model could not be used conclusively to refute this hypothesis, but that the balance of probability is against it. More direct evidence from other stations on the Nile, from reviews of climatological variations, and from the simple fact that rainfall in the lake region has increased sufficiently to cause the observed changes in outflows
makes it difficult to sustain the Acres hypothesis.
Without better evidence, it is dangerous to assume that the net basin supply of recent decades is a reliable measure of the long term value; indeed at the simplest level, there are more years of low outflows than of the higher outflows in the record since it began in 1896. Also, anecdotal evidence (see Section 3.1) suggests that, although lake levels may have been as high in 1878 as 1964, the lake level dropped rapidly in the next few years. It is evident that the assumption that the later high outflow period of record represents the long term average outflow for energy generation seriously overestimates the firm energy flow unless the Acres hypothesis is correct. For example, the rough comparisons made in Section A.3.6 suggest that outflows could be less than the firm energy flow approximately four times as often as expected by Acres if their hypothesis is not correct.

## Appendix B Data Summary

## List of tables

End of month lake levels above Jinja datum (1896-1992)
Net rainfall calculated using the revised model of Section 5.2 (1925-1990)
Revised outflow record from Section 3.3 (1896-1992)
Revised tributary inflow record from Section 5.2 (1925-1990)
Lakeshore rainfall records (various periods 1900-1992)

Jinja<br>Entebbe<br>Kalangala<br>Bukoba<br>Kagondo<br>Mwanza<br>Musoma<br>Kisumu

Catchment rainfall records (1925-1990)
Nzoia
Yala
Sondu
Awach Kaboun
Kagera


## LAKE VICTORIA: Lake rainfall (mm)



LAKE VICTORIA：Revised outflows（MCM）

| rov |  <br>  －Onvavfuna |   |  <br> プoo |  |  リトか＋ oocvanfun」 | Oono oovanawna NWWWHWNWM |  |  | পপপoপ옹ㅇㅇㅇ |
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| No | NNNNNNNWNN NONOOAWONOGH | AWWNWWNWWW OWONOVNONACN | NWWWNFAWWG OOOONJVNTN | ज゙ज゙ひज WVWO VロFのaーのaWW |  | NNW్NM， NWWWNTRONA |  NơOWNONNVO |  | a゙がいNONNべべいい <br> aీNNWVMF <br> OVNOW $\rightarrow$ WVIOMO |
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| $\begin{aligned} & \overrightarrow{0} 0 \\ & \text { Nit } \\ & \end{aligned}$ | NWNNNNNWNN スNNuNNONがNOC WVNodionole |  |  |  | WNFW్NNTV जOOUn＝U |  －cooanw |  WNOVOMOMO | NNOMNWーW $\rightarrow+$－NNかへ |  |
| $\begin{aligned} & \text { NN } \\ & \mathbf{N}^{\circ} \\ & \text { W్囚 } \end{aligned}$ | WWNNNNNNNN人f | AWWNNNNWWW ㄱuNNMOFMNON | WFWWWA WOONGOWNOG |  givouwnitioiva |  |  FNFTOMONVNON |  <br>  | Conondwnuిw |  |
| N | WWNNNNNNWN NANNNGNWNN： OVㄱㅇㅇONONO | WFWWNNOWNW <br>  |  |  <br>  |  |  | Noan focm | oavNougजv | ódNOMNMNO <br>  |
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| W్N入 |  | AWWWNNWWWW ODNF NoN NANONW | 今fWWWWNAWが OWOOFOजNUN Of VovーWuiow | $\underset{\infty}{\mathbf{N}}$ | $\rightarrow N N \overrightarrow{N い か ゙ ~}$ OWNAVMO VUNNOO | $\rightarrow$ ヘNNN～ーシNNN <br>  |  FOOVFOOWN जUNーACNWC |  |  <br>  <br>  |
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| - N | $\begin{aligned} & \text { OONWUNANO } \\ & \rightarrow \AA 0_{0} \end{aligned}$ | WWWWNWWNNW <br>  Gめぁ Vivunw |  |  VIN＋NコンNW | $\begin{aligned} & \text { WWNNNA } \\ & \text { ONONNNN } \end{aligned}$ | NONWUजMOOO WOFWWMOVN | NaV゚がNWON <br>  |  NNNONNNNWN | べNOがンONNA へancumonauc |
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| $\begin{aligned} & \mathbf{w} \\ & \mathbf{O} \\ & \stackrel{N}{N} \end{aligned}$ | NUNNNWNNNN OONNNNNONT <br>  | WWWNNNNNWW <br>  | WWWNWWWWWN ONNOWOANOON． |  <br>  |  |  |  | ज్MaOOUNW <br>  |  |
| $\begin{aligned} & \text { w } \\ & \text { o } \\ & \text { o } \end{aligned}$ | NーTNNNNNWー <br>  －ONOOOOOOO | WWWWNNNWWW ago ANNFOONW －めのNUすONON | WFWWWWAWWN <br>  |  <br>  | जW゙がが Nacoungiaf |  <br>  |  <br>  |  |  |
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## LAKE VICTORIA: Estimated total tributary inflows (MCM)

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | Sum |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1925 | 955 | 744 | 1116 | 1487 | 1627 | 1530 | 1526 | 1601 | 1320 | 960 | 1345 | 1733 | 15943 |
| 1926 | 1162 | 787 | 850 | 2241 | 2490 | 1646 | 1511 | 1926 | 2449 | 2285 | 1911 | 1867 | 21123 |
| 1927 | 1394 | 924 | 986 | 1829 | 1781 | 1108 | 858 | 789 | 674 | 557 | 500 | 546 | 11945 |
| 1928 | 548 | 451 | 491 | 1570 | 2282 | 1713 | 1109 | 996 | 937 | 765 | 652 | 597 | 12112 |
| 1929 | 493 | 366 | 399 | 937 | 1542 | 1142 | 1328 | 1269 | 909 | 666 | 547 | 591 | 10190 |
| 1930 | 640 | 461 | 1608 | 4565 | 3604 | 2451 | 1795 | 1405 | 1447 | 1434 | 1256 | 1163 | 21830 |
| 1931 | 934 | 658 | 1063 | 2377 | 2324 | 1579 | 1428 | 1907 | 1966 | 1517 | 1169 | 1130 | 18050 |
| 1932 | 966 | 711 | 1248 | 2206 | 2245 | 1651 | 1561 | 1915 | 2097 | 1784 | 1377 | 1320 | 19082 |
| 1933 | 1085 | 810 | 859 | 883 | 907 | 711 | 757 | 1193 | 1313 | 1012 | 725 | 648 | 10903 |
| 1934 | 511 | 354 | 381 | 1074 | 1505 | 1271 | 1424 | 1632 | 1364 | 931 | 782 | 766 | 11995 |
| 1935 | 628 | 786 | 833 | 964 | 2099 | 2358 | 1815 | 1219 | 1041 | 953 | 813 | 735 | 14244 |
| 1936 | 655 | 854 | 1425 | 3075 | 2493 | 2229 | 2002 | 1772 | 1685 | 1335 | 1113 | 1093 | 19730 |
| 1937 | 931 | 737 | 1020 | 3240 | 3536 | 2710 | 2588 | 2609 | 2192 | 1658 | 1736 | 1958 | 24916 |
| 1938 | 1454 | 1054 | 1190 | 1484 | 1788 | 1726 | 2048 | 2408 | 2172 | 1623 | 1309 | 1237 | 19492 |
| 1939 | 1040 | 811 | 815 | 2181 | 1744 | 1145 | 1418 | 1341 | 984 | 768 | 684 | 674 | $13605$ |
| 1940 | 646 | 613 | 1571 | 3040 | 2477 | 1576 | 1475 | 1520 | 1242 | 984 | 1071 | 1215 | 17429 |
| 1941 | 903 | 636 | 711 | 2282 | 2654 | 1974 | 1488 | 1403 | 1244 | 974 | 1517 | 2224 | 18012 |
| 1942 | 1622 | 1001 | 2057 | 4189 | 3955 | 3042 | 2448 | 2200 | 1931 | 1502 | 1230 | 1195 | 26371 |
| 1943 | 1020 | 773 | 827 | 1599 | 1998 | 1634 | 1335 | 1216 | 1084 | 827 | 666 | 654 | 13632 |
| 1944 | 585 | 487 | 607 | 1219 | 1777 | 1359 | 1369 | 1357 | 1309 | 1142 | 962 | 899 | 13073 |
| 1945 | 649 | 472 | 491 | 470 | 1662 | 2106 | 1812 | 1760 | 1863 | 1526 | 1116 | 1047 | 14973 |
| 1946 | 848 | 603 | 624 | 1560 | 1970 | 1894 | 1683 | 2101 | 2174 | 1575 | 1180 | 1086 | 17297 |
| 1947 | 932 | 669 | 683 | 3318 | 3881 | 2821 | 2712 | 2304 | 2071 | 1781 | 1389 | 1348 | 23908 |
| 1948 | 1133 | 847 | 879 | 2029 | 2224 | 1885 | 1845 | 1726 | 1579 | 1426 | 1256 | 1169 | 17997 |
| 1949 | 944 | 671 | 701 | 1696 | 2028 | 1533 | 1394 | 1368 | 1487 | 1302 | 970 | 918 | 15012 |
| 1950 | 750 | 521 | 633 | 1364 | 1293 | 1130 | 1556 | 1651 | 1403 | 1073 | 841 | 797 | 13012 |
| 1951 |  | 492 | 651 | 4038 | 3726 | 2113 | 1638 | 1515 | 1286 | 1034 | 1378 | 2809 | 21341 |
| 1952 | 2568 | 1464 | 1414 | 3164 | 4010 | 2651 | 2097 | 2012 | 1698 | 1399 | 1199 | 1169 | 24845 |
| 1953 | 1001 | 774 | 855 | 1890 | 1971 | 1727 | 1421 | 1152 | 1062 | 892 | 795 | 817 | 14355 |
| 1954 | 668 | 519 | 588 | 1689 | 2693 | 2115 | 1638 | 1443 | 1341 | 1128 | 866 | 802 | 15489 |
| 1955 | 671 | 538 | 604 | 1524 | 1555 | 885 | 1181 | 1737 | 2023 | 1710 | 1290 | 1286 | 15006 |
| 1956 | 1513 | 962 | 805 | 1848 | 2561 | 1684 | 1674 | 2201 | 2170 | 1688 | 1156 | 1065 | 19326 |
| 1957 | 840 | 677 | 816 | 1842 | 2890 | 2898 | 1881 | 1944 | 1468 | 951 | 910 | 1003 | 18121 |
| 1958 | 768 | 802 | 862 | 998 | 1867 | 1295 | 1593 | 1646 | 1663 | 1335 | 838 | 962 | 14629 |
| 1959 | 787 | 618 | 1016 | 1558 | 1841 | 967 | 941 | 1091 | 1259 | 1146 | 1062 | 1025 | 13310 |
| 1960 | 823 | 671 | 1210 | 2641 | 2149 | 1644 | 1488 | 1573 | 1953 | 1336 | 1084 | 954 | 17526 |
| 1961 | 737 | 611 | 690 | 1070 | 1276 | 922 | 1031 | 1938 | 1772 | 1903 | 4517 | 5390 | 21856 |
| 1962 | 3886 | 1712 | 2155 | 3700 | 6064 | 3541 | 3172 | 3192 | 3030 | 2203 | 1756 | 1727 | 36136 |
| 1963 | 1865 | 1482 | 1827 | 3928 | 7552 | 3473 | 2686 | 2813 | 2048 | 1563 | 1605 | 3822 | 34664 |
| 1964 | 2231 | 1546 | 2161 | 5598 | 3677 | 2608 | 2746 | 3134 | 2655 | 2617 | 1618 | 1741 | 32332 |
| 1965 | 1557 | 1099 | 1175 | 2099 | 2502 | 1423 | 1343 | 1180 | 987 | 1047 | 1503 | 1513 | 17428 |
| 1966 | 1130 | 1137 | 1748 | 4231 | 2881 | 1763 | 1674 | 1445 | 1852 | 1267 | 1288 | 1021 | 21435 |
| 1967 | 923 | 771 | 890 | 1783 | 3400 | 2066 | 2439 | 2068 | 1662 | 1500 | 1893 | 2054 | 21448 |
| 1968 | 1372 | 1455 | 2687 | 4880 | 5490 | 3407 | 2721 | 2969 | 1930 | 1458 | 1571 | 2661 | 32600 |
| 1969 | 1553 | 1612 | 2127 | 2439 | 2990 | 1888 | 1570 | 1705 | 1523 | 1293 | 1171 | 1212 | 21083 |
| 1970 | 1493 | 1402 | 2690 | 4127 | 3542 | 2666 | 1997 | 2731 | 2219 | 1915 | 1406 | 1384 | 27572 |
| 1971 | 947 | 704 | 706 | 2024 | 2850 | 2044 | 1902 | 2472 | 2433 | 1743 | 1186 | 1128 | 20139 |
| 1972 | 953 | 992 | 983 | 927 | 1988 | 2036 | 1920 | 1618 | 1270 | 1445 | 3207 | 2611 | 19950 |
| 1973 | 2168 | 1497 | 1106 | 1279 | 1956 | 2048 | 1312 | 1721 |  | 1578 | 1968 | 1244 | 19982 |
| 1974 | 896 | 685 | 872 | 4166 | 2470 | 1708 | 3006 | 1719 | 1942 | 1503 | 1069 | 910 | 20946 |
| 1975 | 781 | 591 | 1076 | 1588 | 1652 | 1621 | 1540 | 2473 | 2964 | 2334 | 1234 | 1114 | 18968 |
| 1976 | 813 | 726 | 735 | 1089 | 1721 | 1681 | 1668 | 1405 | 1680 | 1029 | 975 | 887 | 14409 |
| 1977 | 1102 | 1053 | 1016 | 3816 | 6081 | 2452 | 2527 | 2291 | 1717 | 1291 | 3716 | 2085 | 29147 |
| 1978 | 1455 | 1551 | 6112 | 4789 | 4591 | 2634 | 2721 | 2916 | 2507 | 1977 | 1870 | 2452 | 35575 |
| 1979 | 1400 | 2356 | 2184 | 3206 | 2701 | 2409 | 2028 | 2228 | 1749 | 1510 | 1295 | 1188 | 24254 |
| 1980 | 1006 | 910 | 1044 | 1225 | 2167 | 1631 | 1624 | 1221 | 1216 | 878 | 836 | 877 | 14634 |
| 1981 | 1007 | 978 | 1400 | 3077 | 2875 | 2146 | 1787 |  |  | 1463 |  |  |  |
| 1982 | 1032 | 978 1769 | 979 1479 | 3691 | 2748 | 1754 | 1736 1590 | 2260 | $2091$ | 1339 | 2256 | 3284 1607 | 24110 |
| 1983 | 1277 | 1769 | 1479 | 1503 | 1465 | 1282 | 1150 | 1141 | 962 | 807 | 974 | 990 | 13654 |
| 1985 | 970 | 908 | 1379 | 3666 | 3327 | 2363 | 2309 | 1983 | 1500 | 1138 | 1040 | 1084 | 21668 |
| 1986 | 1009 | 983 | 1050 | 2985 | 3007 | 1992 | 2060 | 1714 | 1433 | 1177 | 909 | 951 | 19270 |
| 1987 | 1011 | 926 | 1114 | 1583 | 2394 | 2228 | 1734 | 1428 | 1396 | 1093 | 1350 | 1782 | 18038 |
| 1988 | 1671 | 1295 | 1301 | 4372 | 4030 | 2573 | 2957 | 3102 | 3180 | 2736 | 2162 | 2118 | 31497 |
| 1989 | 1807 | 1450 | 1947 | 3108 | 2946 | 2451 | 2379 | 2449 | 2291 | 2076 | 1861 | 1842 | 26605 |
| 1990 | 1622 | 1382 | 2627 | 4777 | 3578 | 2173 | 1702 | 1617 | 1543 | 1230 | 1116 | 1178 | 24544 |

## JINJA RAINFALL (mm)



## ENTEBBE RAINFALL (mm)



## KALANGALA RAINFALL（mm）

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| NNNAUNF <br> NNWMNGN్త | AWUMNTNAN <br>  | WWNAWW WNW <br>  | WWWM $\stackrel{N}{0}$ WUN <br>  | WNGFUTWNNWN <br>  | WWAN＋ ज゙ञいてN゙ロ： |
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## BUKOBA RAINFALL（mm）

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## KISUMU RAINFALL（mm）

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## SONDU CATCHMENT RAINFALL（mm）

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## AWACH KABOUN CATCHMENTRAINFALL（mm）

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## KAGERA CATCHMENT RAINFALL（mm）

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