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The Hydrogeology of the Nyamazura Area.

by

Jeffrey Davies



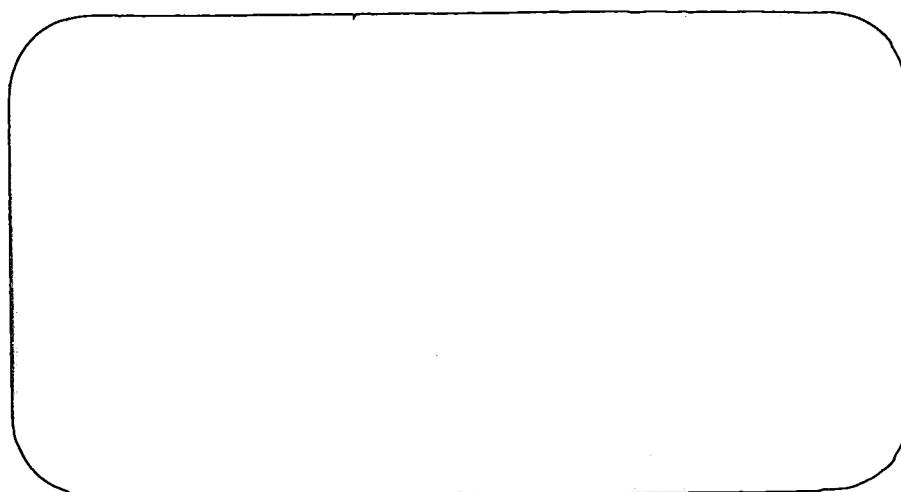
Natural Environment Research Council

British Geological Survey

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# HYDROGEOLOGY

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## THE HYDROGEOLOGY OF THE NYAMAZURA AREA

### 1. INTRODUCTION

#### 1.1 Background.

The development of the Nyamazura Intensive Resettlement area is being jointly funded by the Government of Zimbabwe and the British Government. The British funded Primary Water Supply Unit of the Ministry of Water Resource and Development were required to undertake an exercise of borehole drilling, cleaning and equipping to supply good quality domestic water supplies to a series of twelve villages located within the scheme area. This work was undertaken during July-September 1984, during which time a series of 13 holes were drilled (9 successfully) and 3 holes were cleaned out. All holes were equipped with hand operated Bush Pumps of local design. While undertaking the above work the opportunity was taken to make a fairly detailed assessment of the hydrogeological potential of the Basement Aquifer in the area (see Fig. 1).

#### 1.2 Location.

The Nyamazura area is located some 40 kilometres west of Mutare in the Manicaland Province of Zimbabwe. The area encompasses some 10,830 hectares of farmland located north of Odzi primarily between the Odzi and Chingwandum rivers.

#### 1.3 Topography.

The topography of the area is of gently undulating aspect with prominent hills to the west (Chenyamatsini 1320 metres), the north (south of Game Valley 1213 metres) and the north east (in Osbourne up to 1250 metres). The drainage pattern is rectalinear in form being developed upon primary structural lineations that trend NW-SE, NE-SW and N-S. The main rivers are the Chingwandum, Inyamazura and Odzi. Only the latter was flowing after 3 years of drought in September 1984.

#### 1.4 Climate.

During the last 50 years annual precipitation has varied between a maximum of 1300 mm and a minimum of 370 mm (see Fig. 2) (average of 700 mm p.a.). During the period of 1982-84 the area has experienced drought conditions with annual rainfall of 550 mm or less. The rainy season usually lasts from 3-5 November until 28th March. Evapotranspiration rates are not known.

#### 1.5 Vegetation.

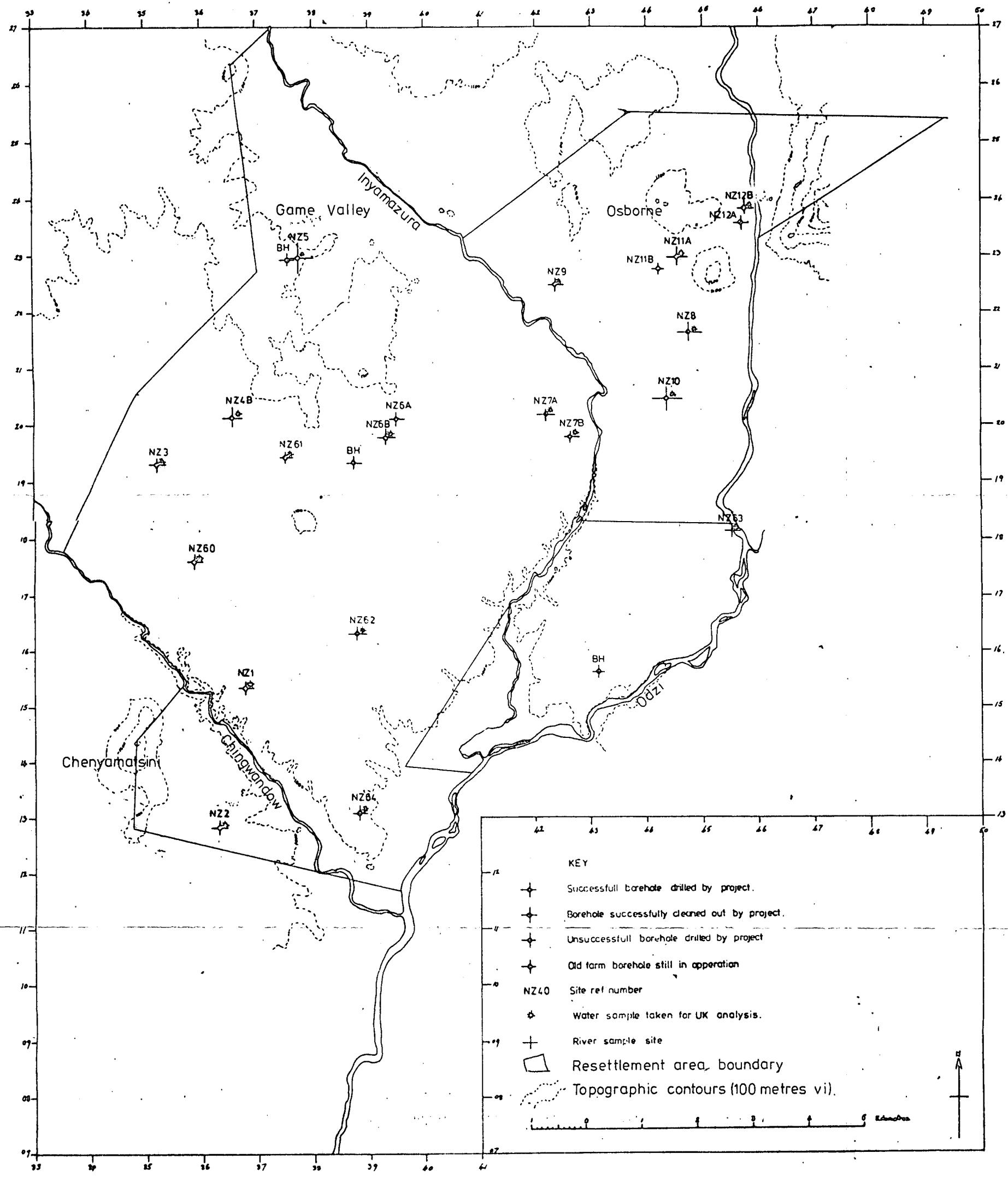
The natural vegetation of bush scrub has been replaced over much of the area by open grassland as a result of farming activities, primarily cattle ranching and the cultivation of tobacco and maize.

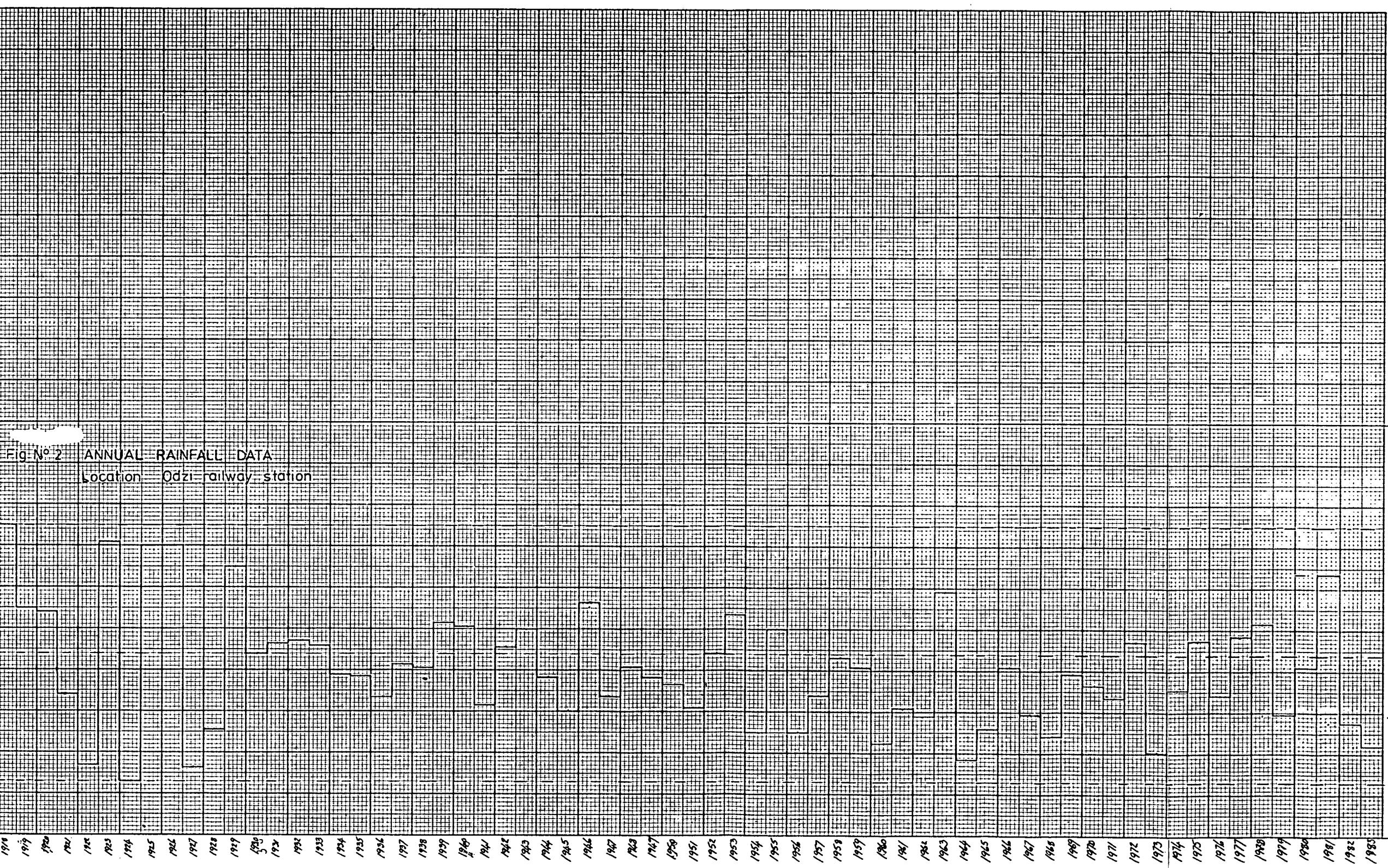
#### 1.6 Population and Settlement.

The resettlement area encompasses a series of former commercial farms, now vacated. The proposed pattern of settlement within the Nyamazura area is described in detail within a report prepared by Agritex (June 1981). The PWSU were primarily concerned with the supply of domestic water to a series of 12 peripheral villages in which 400 plus families have been settled.

# INYAMAZURA RESETTLEMENT SCHEME

Fig N° 1 BOREHOLE LOCATION MAP





### 1.7 Aims of the Project.

The primary aim of the project is the provision of a source of good quality domestic water within 0.5 kilometres of each village at as low a cost as possible. During the project, opportunity was taken to conduct a fairly detailed hydrogeological investigation of the area that included the collection of sludge samples from each newly drilled hole, the test pumping of new and cleaned out holes and obtaining water samples for chemical analysis from new, cleaned out and old boreholes and as and when access permitted. It is hoped that the report produced will provide an additional insight into the occurrence of groundwater in Weathered Basement strata.

## 2. GEOLOGY

### 2.1 Introduction.

No detailed geological map of the Nyamazura Resettlement scheme exists. To obtain an understanding of the geology of the area two sources were consulted, i.e. the National Geological Map of Zimbabwe that provides an incomplete description of the geology of the area, and Swift (1972) which describes in detail the geology of the Odzi Gold Belt that lies adjacent to the southern margin of the area.

Additional data especially of a structural nature were derived from a study of topographic maps and aerial photography to locate linear fault/joint zones. During the project period geophysical surveys using EM and Wenner type resistivity arrays were used during the location of drill sites. The boreholes drilled yielded detailed geological data. Unfortunately no data whatsoever were forthcoming from the pre-project boreholes.

Details of the data sources used are presented in Appendix I.

Of prime interest to the project were a knowledge of rock types, depths of weathering and occurrence and nature of linear fault/joint fractures.

### 2.2 Pre-cambrian Basement Strata.

For the purposes of this report the Basement Precambrian strata have been divided into two (see Fig. 3):-

- (1) Granitic gneisses of the Older gneiss series and the younger granodiorite-adamalite series.
- (2) Dolerite dykes and sills.

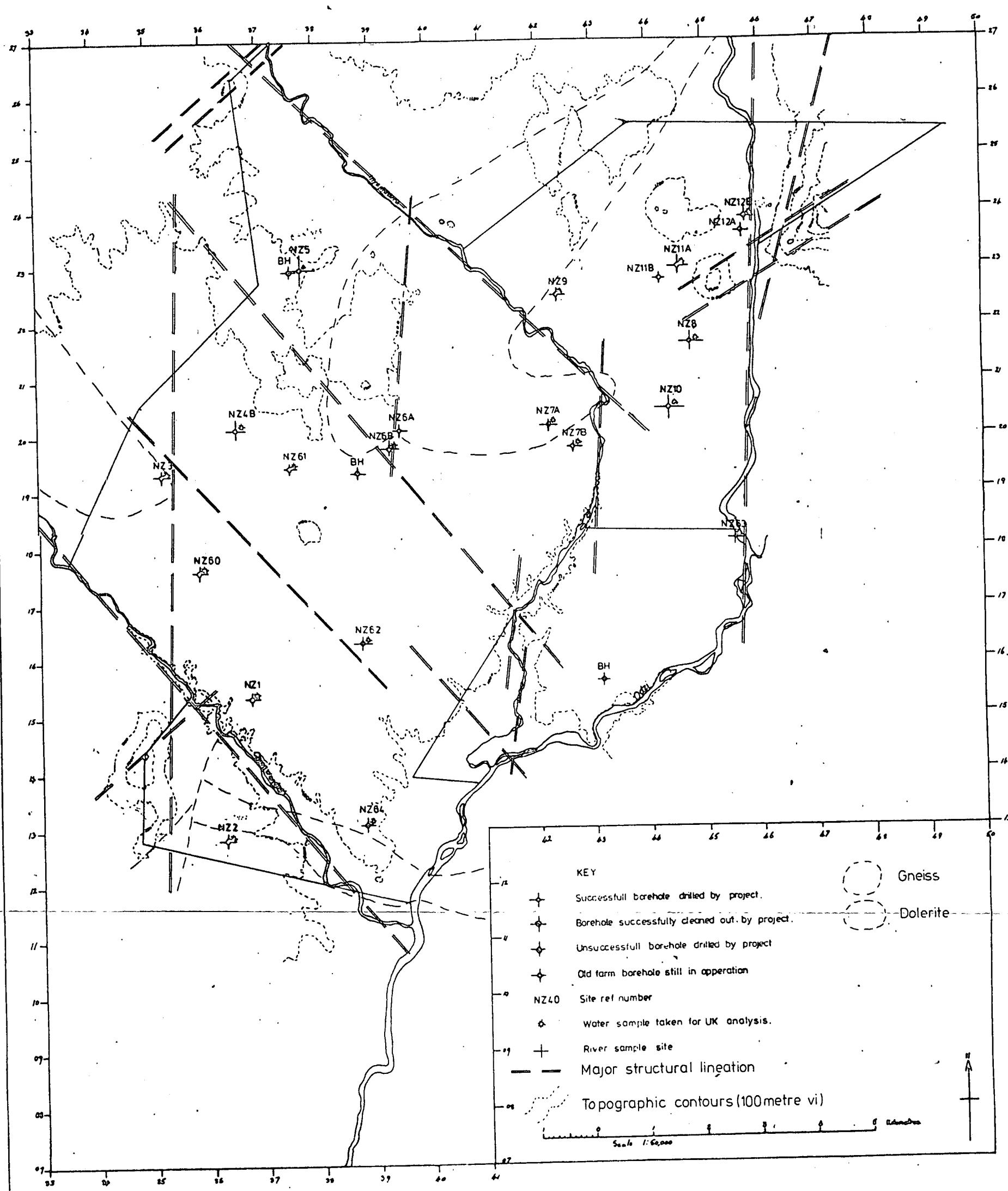
#### 2.2.1 Precambrian granites and gneisses

Most of the area is underlain by granites and gneisses of Precambrian age. Except in the north east of the area the strata appear to be quite uniform in type being composed of white quartz and feldspar with varying proportions of mafic minerals. These rocks are usually quite coarse grained. Sometimes rock with predominantly pink feldspars occur commonly as vein material within the white granitic gneiss.

Within the north east corner of the area more schistic strata are found alternating with white to grey coarse grained gneiss. The schistose bands are

# INYAMAZURA RESETTLEMENT SCHEME

Fig No 3 GEOLOGICAL MAP



commonly formed of dark green micaeous amphibolitic schists. Within the north east of the area these latter rocks form high hills whereas the rest of the area is underlain by the older granites and gneisses is topographically subdued.

### 2.2.2 Precambrian dolerites

Dolerites of Precambrian age occur along two east-west trending bands within the central and southern parts of the area. They commonly crop out as low ridges and occasionally high hills with a litter of rounded dark green dolerite blocks scattered at their margins. The soil cover developed upon these dolerite ridges has a characteristic bright brick red hue. These dolerites are younger than the surrounding granites and gneisses into which they have been intruded.

### 2.3 Structural Geology.

An analysis of topographic maps and aerial photography of the area has indicated the presence of a rectalinear series of fault/joint lineations that traverse the area. These lineations trend NW-SE, NE-SW and N-S, forming zones of weakness that are now followed by the main rivers of the area (see Fig. 3).

### 2.4 Borehole Siting.

Where new water supply boreholes were required to be drilled, geophysical methods were employed in association with aerial photograph interpretation, to locate the sites. Two geophysical methods were utilised:-

- (1) Electrical resistivity utilising a Wenner spread arrangement.
- (2) Electromagnetics (EM) using a 20 or 40 metre spread length.

The resistivity method is a well tried and proven method which suffers from several disadvantages:

- (a) relatively slow survey speed compared with the EM method,
- (b) lack of definition in the latter part of the curve made the calculation of depths of weathering difficult to undertake.

Therefore use was mainly made of the EM method with which rapid traversing of fairly large areas could be undertaken in relatively short periods. The contour maps produced can indicate lineations, zones of weathering, the presence of ferro-mag mineral rich rocks etc. Several examples of survey results obtained from the Nyamazura area are included in Appendix II.

Generally the higher the EM effect the greater the depth of weathering. Using this criteria three or more boreholes were located upon dolerite dykes and sills where the EM effect reflected the large amount of ferro-mag minerals present in the strata. Similarly high EM measurements (>25 millivolts) have also been obtained from amphibolitic gneisses and schists. Therefore the counterpart hydrogeologists undertaking the EM surveys were encouraged to undertake a grid survey of at least 6 lines at each site, to allow adequate contouring of the site, to note the soil colour at the site and the solid geology of and outcrops at site before selecting a drill site.

Reinterpretation of the results obtained from the surveys using the drilling results have indicated that:-

- (1) Readings of <10 millivolts generally indicate the presence of hard to solid rock near surface.
- (2) Readings of >20 millivolts indicate the presence of ferro-mag conductive minerals within the surface weathered layers derived from dolerites, amphibolitic gneisses or schists etc. and that additional data must be obtained about the solid geology before a drill site can be located.
- (3) Readings of 10-20 millivolts can generally be regarded as good indications of adequate weathering although due regard has to be made to any other geological data derived from the area. If possible Electrical Resistivity VES probes should be undertaken at the favourable site.

Of the surveys detailed in Appendix II that at village 12 (NZ2) is typical of condition 3 where a successful borehole was drilled upon an EM effect of 14.5 millivolts. That at village 7 shows the contrast between high EM effects (>25 millivolts) measured over dolerite (where an unsuccessful borehole, NZ6A, had already been drilled) and the optimum levels of <20 millivolts obtained from a lineation that turned out to be a waterlogged zone of faulted granitic gneiss. The results of the survey at village 6 again shows the contrast between a dolerite (upon which an unsuccessful borehole, NZ7A, had already been drilled) and an area further downstream where a successful hole was drilled upon an EM effect of 11 millivolts. Various NW-SE trending lineations are also located. The last example was at village 1A where a borehole was drilled into an area where the EM effect was greater than 25 millivolts. This borehole was drilled into water bearing amphibolitic gneisses and schists whose EM effect is much the same as dolerite.

### 3. HYDROGEOLOGY

#### 3.1 Introduction.

Prior to the start of the project ten boreholes had been located within the project area. No data existed from these or any unsuccessful borehole that may have been drilled in the area. No hydrogeological study had been made of the Nyamazura area or adjacent areas prior to the current study being undertaken. For the purposes of the current study hydrogeological data was obtained from thirteen holes drilled and three holes cleaned out during the study. Additional hydrochemical data were obtained from four still operational old farm boreholes.

#### 3.2 Current Studies.

During the current study the following classes of data were collected.

- (a) Geological data during borehole drilling.
- (b) Aquifer parameter data during pumping tests.
- (c) Hydrochemical data during pumping tests and spot sampling exercises.

These data were used to produce an idea of the nature of the weathered Pre-cambrian Basement aquifers within the Nyamazura area.

### 3.3 Geological Data.

A series of thirteen boreholes were drilled using Dando cable tool percussion rigs and a Hands England HE20 down-the-hole hammer rig within the Nyamazura area. Where possible these holes were drilled to a minimum diameter of 152 mm (6 inch) and completed at 110 mm being lined from top to bottom with 110 mm diameter Protorite PVC casing and screen. This screen was gravel packed with a sanitary seal of cement being poured from ground surface to a depth of 5 metres above the pack material. During drilling, sludge samples were collected and logged geologically at one metre intervals. The driller recorded the relative hardness of the strata penetrated, changes in colour, location of fractures, changes in rest water levels and depths of zones of water production. The sludge samples collected were stored in glass sample bottles for future analysis. Geological logs, driller's logs and borehole construction logs are recorded and presented in Appendix III.

### 3.4 Pumping Test Data.

Of the sixteen boreholes drilled or cleaned out during this study twelve were subjected to one form or another of pumping test. At the initial hole to be tested, NZ4B, a 6-hour constant yield/drawdown test was undertaken followed by a 1 hour recovery test. Due to time constraints the constant yield/drawdown test was reduced to 3 hours duration with a one hour recovery test. Such a test procedure was used at NZ5, NZ8, NZ9, NZ10 and NZ6B. Reduced time period tests were conducted at NZ7A, NZ11A and NZ12A where poor yields were encountered during testing. Latterly in an attempt to reduce testing time even further a standardised bail test was introduced. This involved bailing the hole using cable tool percussion equipment for a period of 30 minutes and recording the maximum drawdown level and recovery over a 60 minute period. This form of test was undertaken at NZ6B, NZ11B and NZ12B. When plotted upon semi log paper the recovery data from the bail tests and from the longer term constant yield tests appear to be in fairly good agreement especially the long term data where the bail test results for  $t/t' < 2$  are noted (see Fig. 4).

The constant yield/drawdown data were analysed using the Jacob Straight Line Method while the recovery data obtained from these and the bail tests were analysed using the Theis Recovery Method. In general there was good agreement between the results obtained from both methods once the effects of well storage had been overcome. The transmissivity values obtained from the bail tests were usually on the low side, there being insufficient "late" data available to allow adequate analysis of the results in most cases. The results are presented in Table 1, and in Appendix IV.

The results obtained indicated that the dolerite sills and dykes have very low transmissivities below their initial very weathered upper layers. T values of less than  $0.1 \text{ m}^2/\text{day}$  were recorded at three sites indicating that for throughflow analysis purposes the dolerites can be regarded as being impermeable blocks. Within the weathered basement complex aquifers T values were lowest within water shed areas, e.g. at NZ5 where T values of  $< 10 \text{ m}^2/\text{day}$  were recorded. T values increased down the hydraulic gradient to  $> 20 \text{ m}^2/\text{day}$  in the vicinity of the main rivers.

### 3.5 Hydrochemical Data.

Hydrochemical data were obtained from two primary sources:

- (a) pH, temperature and conductance of groundwater pumped were measured at regular intervals during pumping tests.

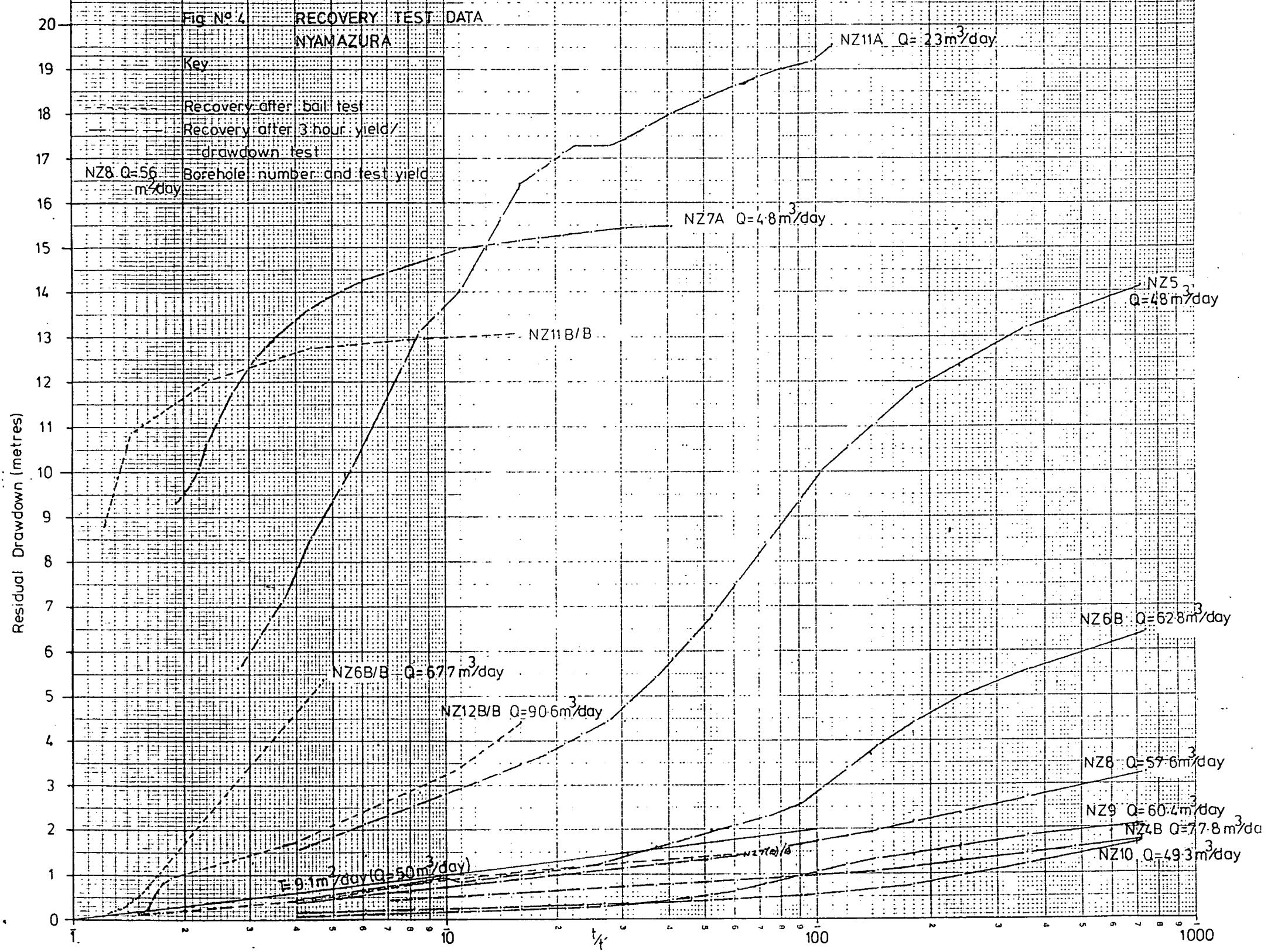


TABLE 1. PUMPING TEST RESULTS, JUNE-AUGUST 1984 - NYAMAZURA.

Borehole No.	Date	Type of Test	Test Duration (minutes)	Method of Analysis	Transmissivity (m <sup>2</sup> /day)	Specific Capacity (m <sup>3</sup> /day/m)
NZ4B	27-4-84	Yield/Drawdown	360	Jacob Straight Line " " " Theis Recovery	(1) 16.38 (2) 24.73 30.9	39.98
		Recovery	60			
NZ5	2-7-84	Yield/Drawdown	180	Jacob Straight Line	2.0	2.70
		Recovery	60	Theis Recovery " " "	(1) 1.2 (2) 3.6	
NZ7A	20-7-84	Yield/Drawdown	24	Jacob Straight Line	1.16	
		Recovery	60	Theis Recovery	0.048	
NZ8	10-7-84	Yield/Drawdown	180	Jacob Straight Line	8.43	14.08
		Recovery	60	Theis Recovery " " "	(1) 5.64 (2) 12.40	
NZ9	18-7-84	Yield/Drawdown	180	Jacob Straight Line " " " Theis Recovery	(1) 16.50 (2) 31.59 (1) 5.14 (2) 44.23	24.35
		Recovery	60	" " "		
NZ10	11-7-84	Yield/Drawdown	180	Jacob Straight Line " " " Theis Recovery	(1) 13.9 (2) 21.2 (1) 5.82 (2) 40.07	19.74
		Recovery	60	" " "		
NZ11A	16-7-84	Yield/Drawdown	115	Jacob Straight Line " " " Theis Recovery	(1) 1.2 (2) 0.27 (1) 1.3 (2) 0.28	0.77
		Recovery	60	" " "		
NZ12A	17-7-84	Yield/Drawdown	35		Very small	
NZ6B	2-8-84	Yield/Drawdown	180	Jacob Straight Line " " " Theis Recovery	(1) 5.11 (2) 8.85 (1) 2.05 (2) 5.23 (3) 8.22	8.99
		Recovery	60	" " "	1.25	
		Bail Test	30/60	" " "		
		Recovery				
NZ7B	10-8-84	Bail Test	30/60	" " "	10.26	33.92
		Recovery				
NZ11B	31-7-84	Bail Test	30/60	" " "	0.017	<6.28
		Recovery				
NZ12B	9-8-84	Bail Test	30/60	" " "	5.72	20.26
		Recovery				

- (b) A total of eighteen samples were taken from boreholes and the Odzi river for analysis in the UK. Groundwater pH, temperature, conductivity and bicarbonate content were determined at each sample site. Each sample was taken in two parts, one remained non-acidified while the other was acidified using concentrated hydrochloric acid. The results of these analyses are still awaited at the time of writing.

#### 4. RESULTS

##### 4.1 Nature of the Weathered Basement Aquifer System.

From the geological logs obtained from boreholes drilled in the area the typical sequence through the weathered zone from ground surface to solid rock would be a profile such as that illustrated in Fig. 5. Within this profile the upper weathered zone would develop within the zone of maximum water movement, i.e. rainfall recharging the aquifer passes through the zone and the water table oscillates within that zone during normal to wet climatic periods. Therefore minerals within this zone are subjected to wet-dry conditions and attack by recharge water when it is in its most acid condition. The resulting weathering is indicated by a breakdown of feldspars to clays with their removal from the near surface zone with runoff water and the oxidation of iron rich minerals with their redeposition in the near surface zone as ferricrete nodules or laterite bands.

Within the lower weathered zone the breakdown of minerals due to attack by low pH waters and wetting and drying cycles probably only takes place during near drought conditions when the water table is reduced in elevation. Within this zone ferromag minerals are initially oxidised and then removed upwards in solution by capillary action to the upper zones. The feldspar minerals are sufficiently attacked by acid water to cause them to crack but not to form clays.

The lowest zone is the relatively unweathered zone where water is contained within joints within the strata. The pH of these waters appears to have increased during their passage down through the upper weathered zones to render them non corrosive.

Within the Nyamazura area the weathered zone appears to be thickest on subdued ridges, e.g. at NZ5 where the zone of water table oscillation is greatest. There the weathered zone is in excess of 30 metres thick. However at this site the transmissivity of the saturated zone of weathering is low due to the presence of much clay in the sequence.

The thinnest zones of weathering are developed adjacent to the main rivers where the zone of water table oscillation is thinnest. In the area between the ridges and the rivers the zone of weathering is commonly 20 metres or more thick (see cross sections NZ5-NZ10 and NZ2-NZ5 (Fig. 6 and 7).

Due to the small number of pumping tests undertaken within the Nyamazura area a comprehensive distribution map of transmissivity cannot be constructed. However there is some evidence to show that over much of the area the weathered Pre-cambrian Basement aquifer has a transmissivity of between 10 and 20 m<sup>2</sup>/day.

What is evident is that the zone of weathering is enhanced along fault and prominent joint lineation zones, e.g. at NZ6B. Within the dolerites the upper weathered zone is evident to a great degree as seen in several boreholes where

Fig No. 5

Typical Weathered Rock Profile for Coarse Grained Igneous Granitic Gneisses

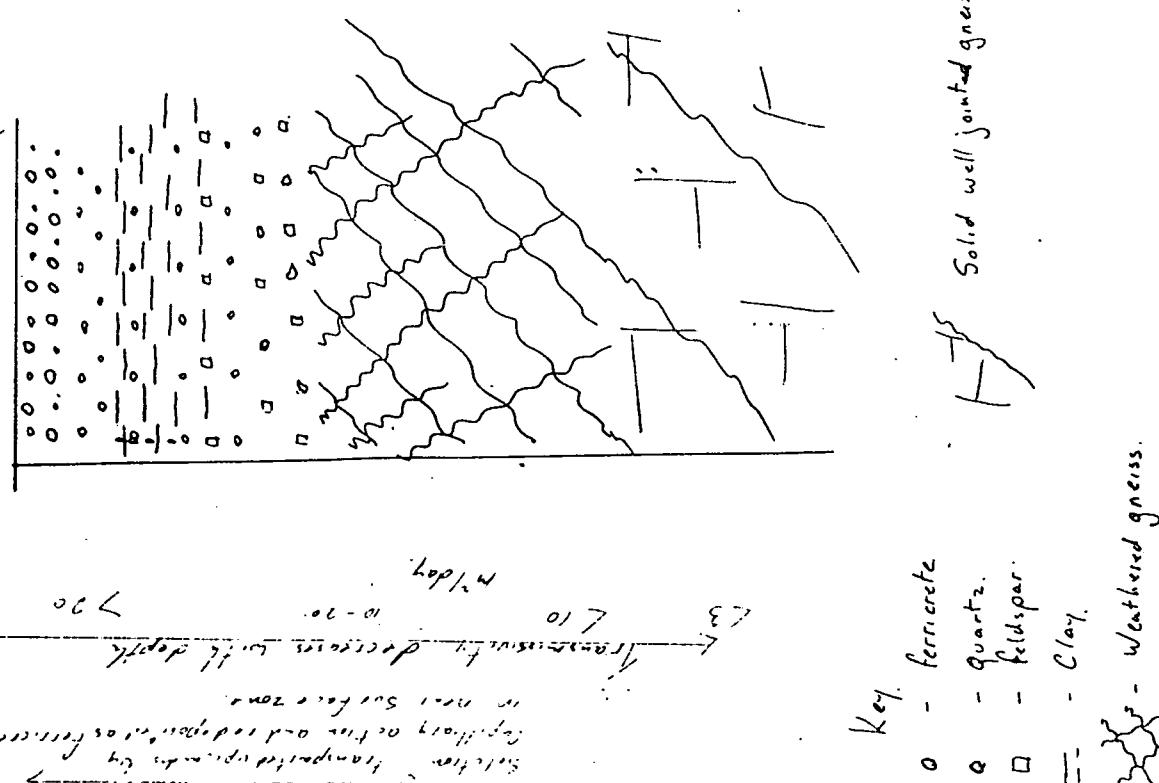
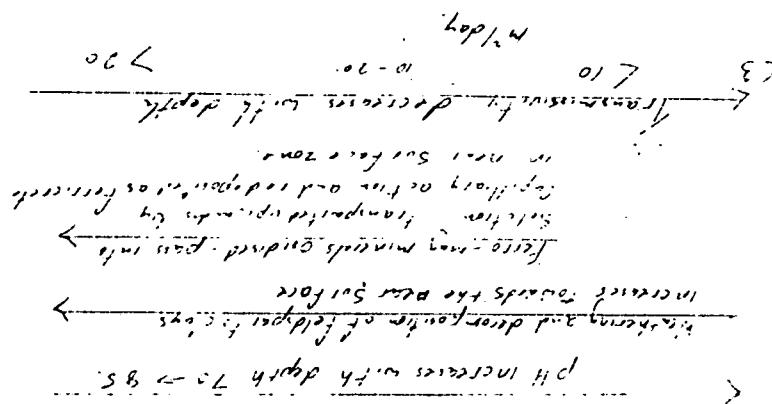
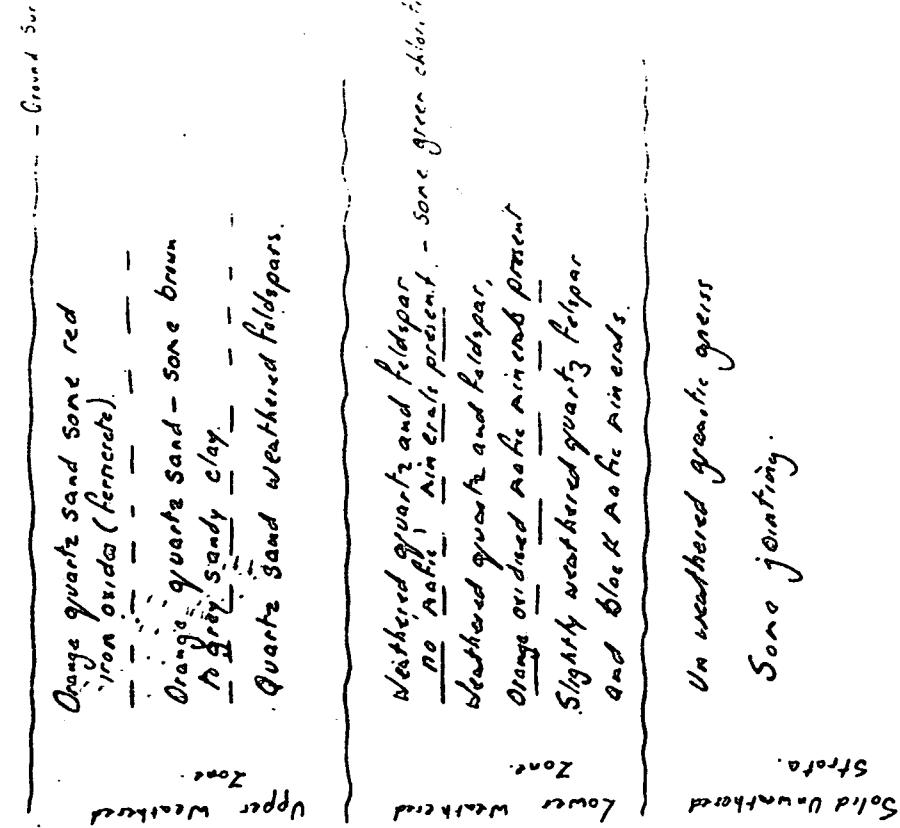


Fig. No 6 HYDROGEOLOGICAL CROSS SECTION  
NYAMAZURA

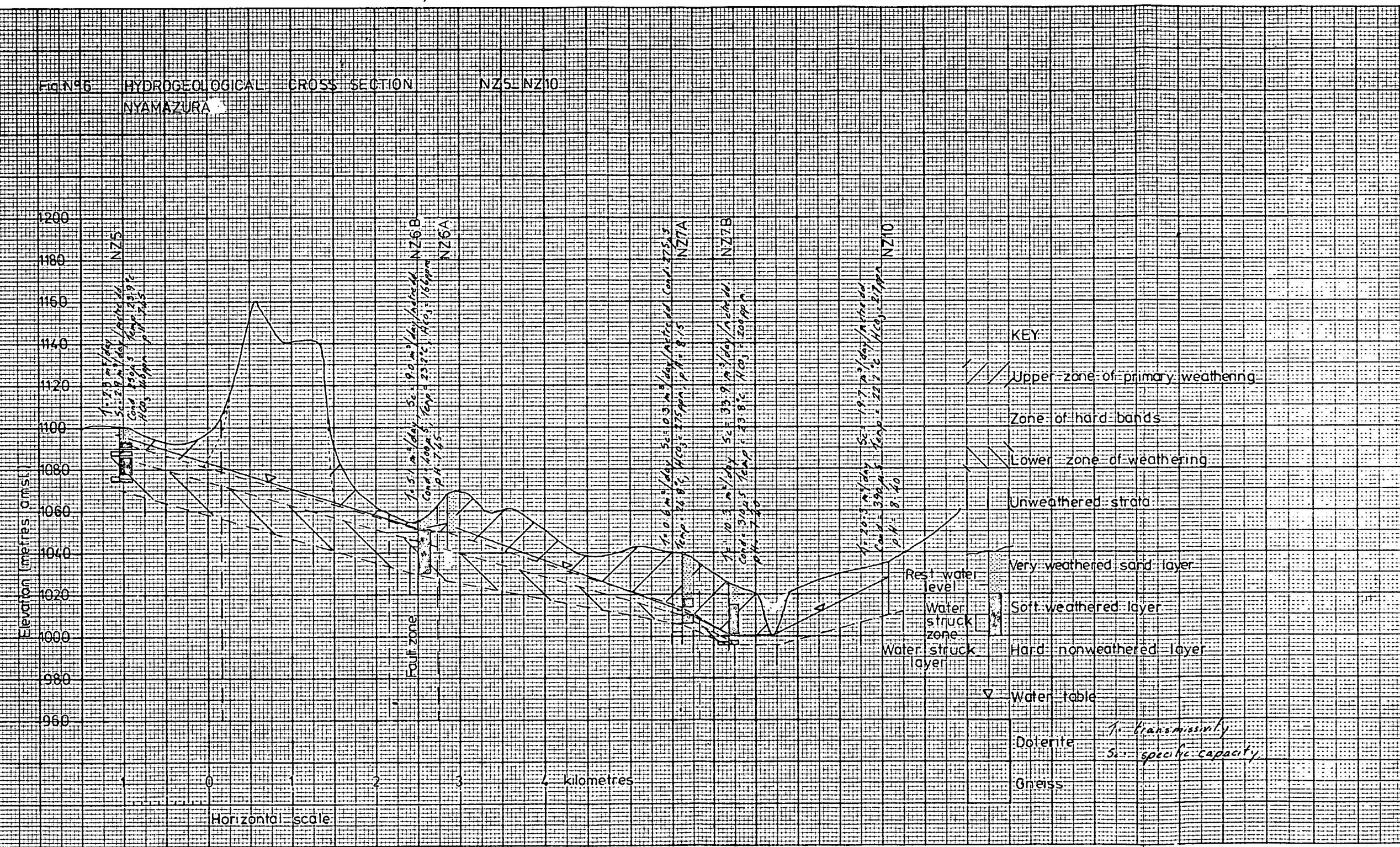


Fig No. 7 HYDROGEOLOGICAL CROSS SECTION NZ2 - NZ5  
NYAMAZURA

180

160

140

120

100

80

60

40

20

0

1000

980

960

940

920

900

Kilometres  
Horizontal Scale

Coord: 260°S 160°E Temp: 23.7°C

Coord: 163.9°E 10.5°N Alt: 7.65

Coord: 160.5°E 10.5°N Alt: 7.65

KEY

Upper zone of primary  
weathering

Lower zone of secondary  
weathering

Unweathered strata

Rest water-  
level

Water struck  
layer

Water struck  
zone

1: transmissivity

2: specific capacity

Bh. N°

Very weathered  
sand layer

Soft weathered  
layer

Soft to hard  
weathered layer

Hard nonweathered  
layer

Water table

Dolerite

Gneiss

the dolerite has decomposed to a green-brown sand. Below this upper layer however the dolerite remains quite solid with zero permeability except for very thin weathered zones which appear to coincide with the lower zone in the granitic gneisses. These thin weathered zones contain very small quantities of water and have very low permeabilities (e.g. NZ11B).

#### 4.2 Groundwater Flow and Recharge.

Within the Nyamazura area groundwater flow is controlled by topography, structural lineations and impermeable dolerite masses, taking place mainly through the weathered Basement Complex aquifer zone (see Fig. 8). Groundwater appears to move from the ridge areas of recharge down dip towards the rivers into which it flows. This general pattern of flow is altered by major structural lineations and by runoff from dolerite hills as is indicated by the groundwater conductance distribution map (see Fig. 8). Rain water runs off the dolerite hills southeast of Game Valley to enter the weathered basement aquifer and flow due south to the Chingwandum valley to the south. Meanwhile the groundwater within the aquifer to the south east of the hills is much more static in nature as indicated by the higher groundwater conductivities recorded ( $>400 \mu\text{S}$ ).

The hydrochemical results so far obtained would tend to support this view of groundwater flow in the Nyamazura area. pH values of  $<7.5$  are found around the southern margin of the dolerite block to the south east of Game Valley indicating that runoff of rainfall water probably occurs from the dolerite block onto the granitic gneiss. pH is seen to increase down dip towards the Chingwandum valley in the direction of flow (see Fig. 9).

In the same area bicarbonate concentrations are lowest along the western margin of the same dolerite block supporting the view that groundwater flows from that area to the south.

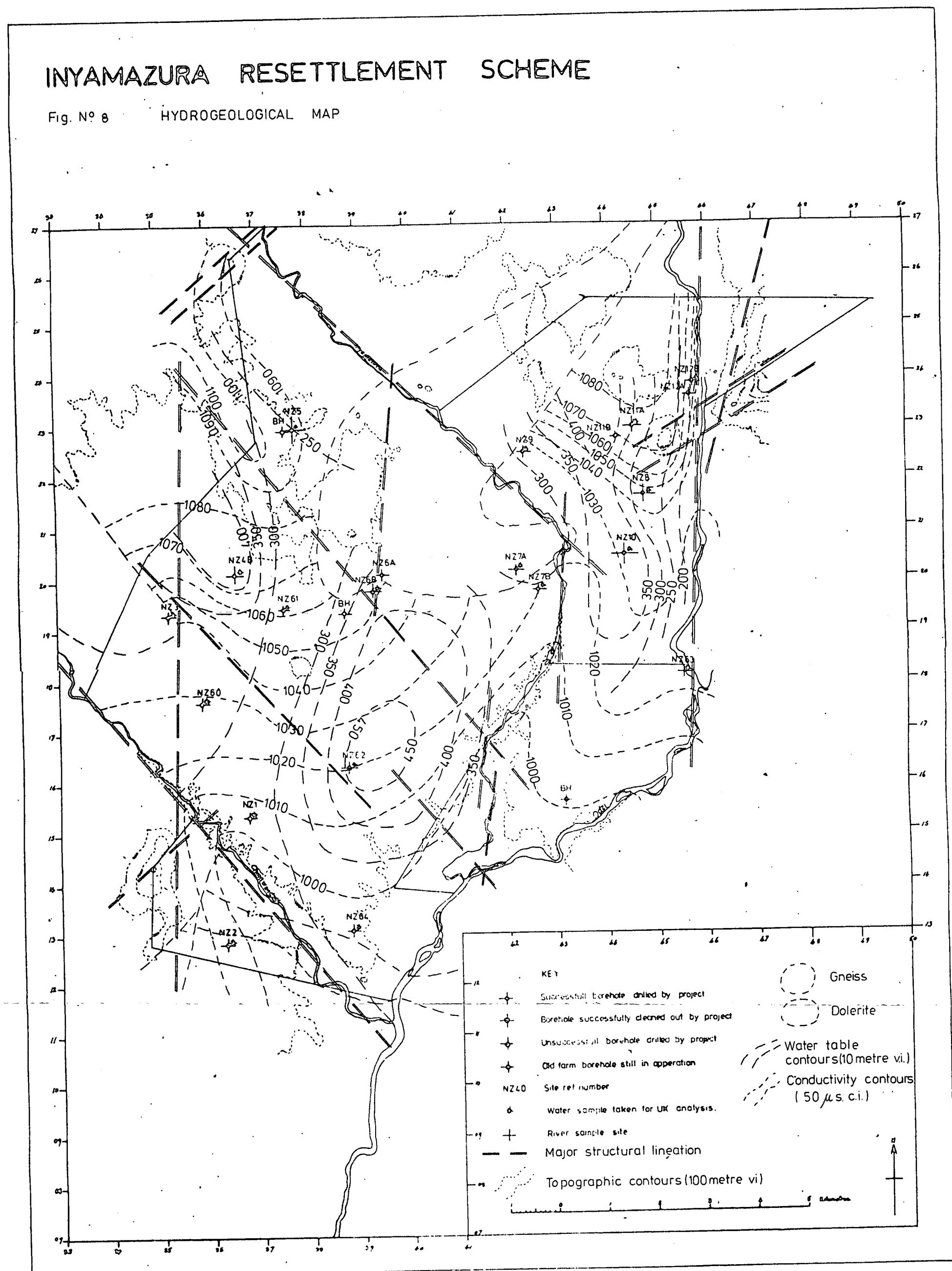
In the north east of the area transmissivity values are much less along the flanks of a steeply-sided ridge. Both pH and bicarbonate values increase rapidly down dip towards the Inyamazura river. Very low values are recorded to the east along the banks of the Odzi river probably due to mixing of groundwater and river water.

#### 5. SUMMARY

The occurrence of groundwater within the Nyamazura area has been summarised in Fig. 10. The structural fault/joint lineations, major rivers and prominent dolerite masses are seen to have marked controlling influences upon the nature of groundwater flow in the area. Factors that control the degree of weathering of the Pre-cambrian Basement Complex aquifer include the climatic regime of the area and the susceptibility of the granitic gneiss formation to weathering especially the feldspar component.

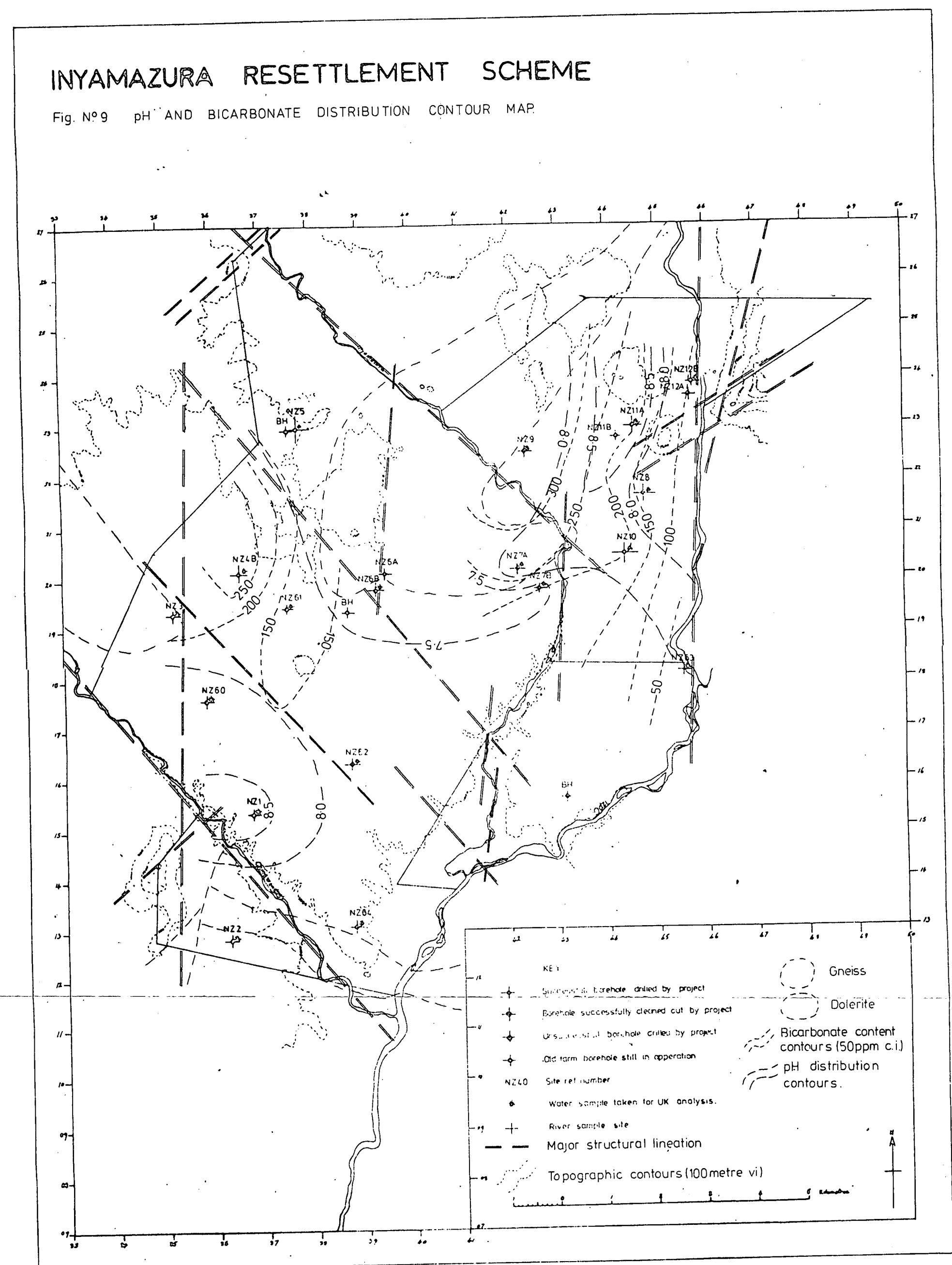
# INYAMAZURA RESETTLEMENT SCHEME

Fig. N° 8 HYDROGEOLOGICAL MAP



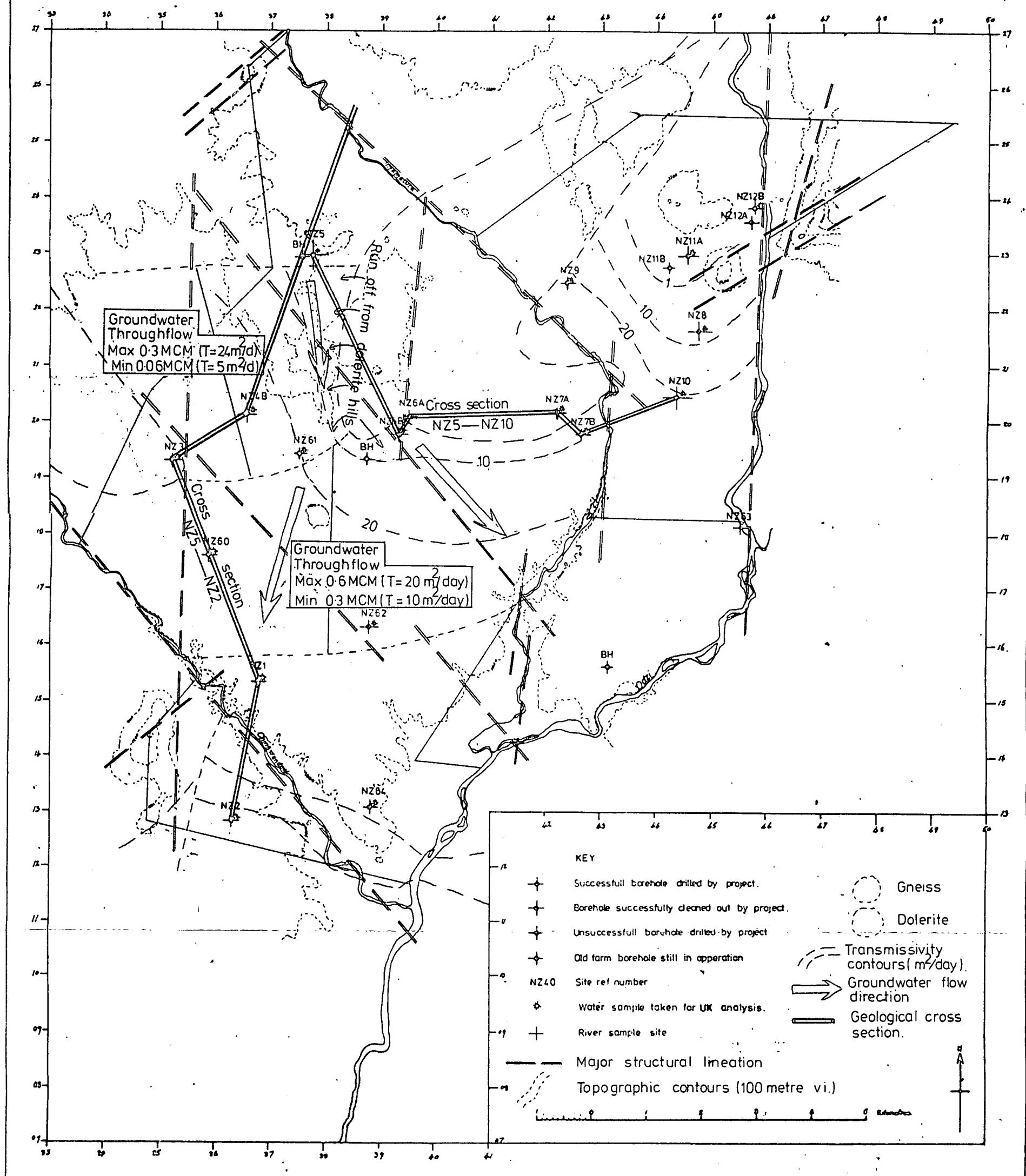
# INYAMAZURA RESETTLEMENT SCHEME

Fig. N°9 pH AND BICARBONATE DISTRIBUTION CONTOUR MAP.



# INYAMAZURA RESETTLEMENT SCHEME

Fig N°10 POSSIBLE OCCURRENCE OF GROUNDWATER WITHIN THE WEATHERED BASEMENT STRATA



A general transmissivity of 10 to 20 m<sup>2</sup>/day has been postulated for most of the Basement Complex aquifer system. Transmissivity is seen to increase radially to the south away from the southern margin of the main dolerite masses.

Groundwater gradients are fairly high. Throughflows of groundwater in the Nyamazura area have been calculated. In the north west of the area groundwater throughflow is of the order of maximum 0.3 MCM/annum to minimum of 0.06 MCM/annum. Within the central part of the area groundwater throughflow is estimated to be maximum 0.6 MCM/annum and minimum 0.3 MCM/annum (dependent upon the T value selected).

The hydrochemical data so far analysed indicate that the groundwaters of the Nyamazura are of good enough quality to be used for domestic and agricultural purposes. This situation could change when the detailed chemical analyses of the water samples obtained become available. The distribution of fluoride within the aquifer system is of particular interest.

#### Acknowledgements.

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- (i) the kind help and assistance that he received from all his colleagues at the Ministry of Water Resources and Development particularly Mr D A Whitaker (PWSU Team Leader), Mr M Sharp (Drilling Supervisor) and Miss I A Richardson (PWSU Field Hydrogeologist),
- (ii) the advice of and helpful discussions with Dr E P Wright (BGS) and Dr R Herbert (BGS),
- (iii) the analytical expertise and advice of Miss J Cook (BGS),
- (iv) the typing expertise and patience of Mrs C Cordery (BGS).

APPENDIX I  
DATA SOURCES

1. Topographic maps.

Scale 1 : 50 000      ODZI 1832 C4,      UMTALI 1832 D3

Scale 1 : 250 000      Mutare sheet SE-36-10;

2. Aerial Photography.

Umtali South 1975

579-586  
660-670  
744-752  
824-828

3. Reports.

- (a) Agritex, March 1983. INYAMAZURA Intensive Resettlement. Planning Branch, Dept. of Agriculture and Extension Services.
- (b) Swift, W H (1972). The Geology of the Odzi Golden Belt. Bull. No. B45 Southern Rhodesia Geological Survey.

4. Geological Maps.

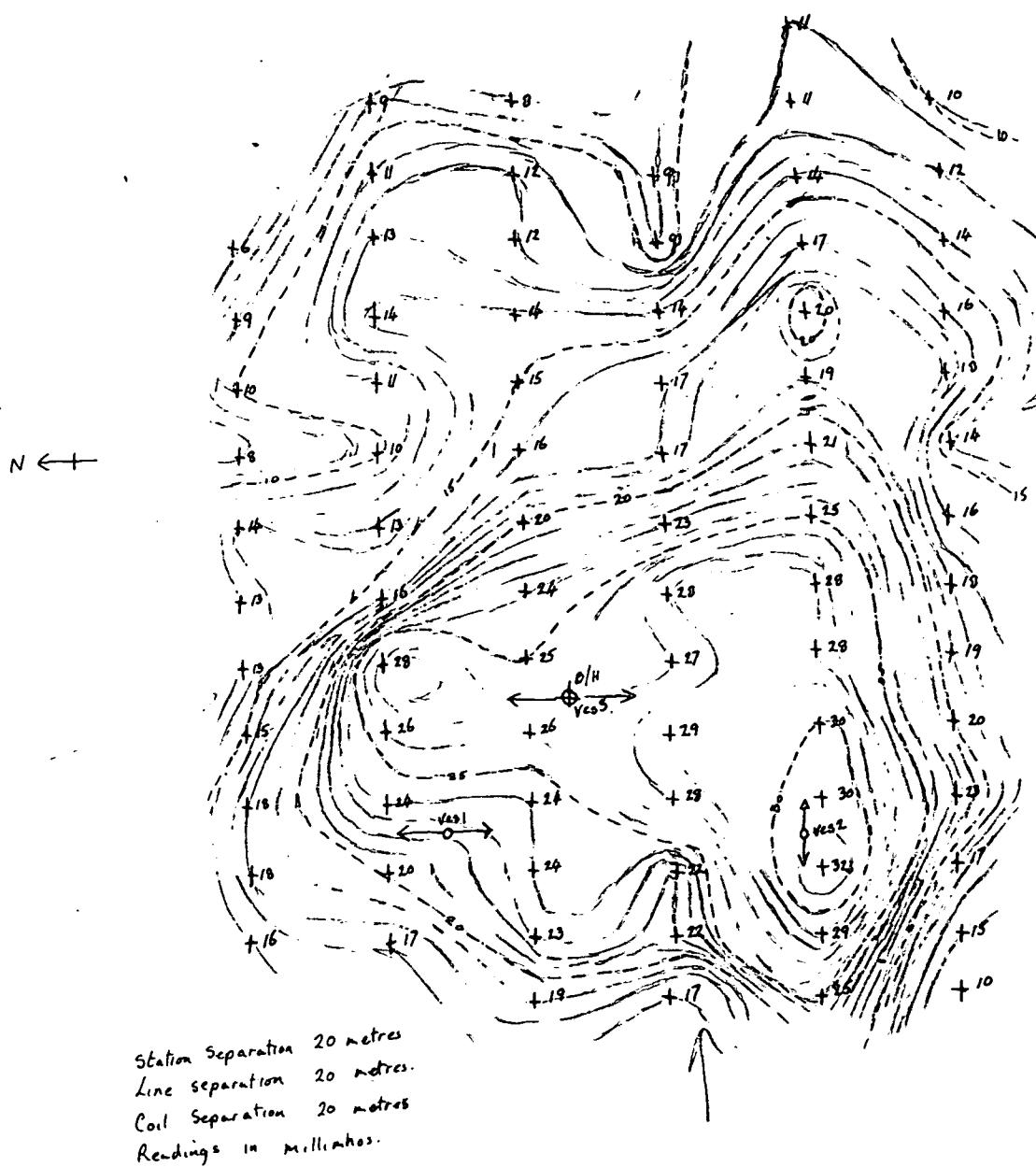
1 : 1 000 000 scale      National Geological Map of Zimbabwe, 1977.

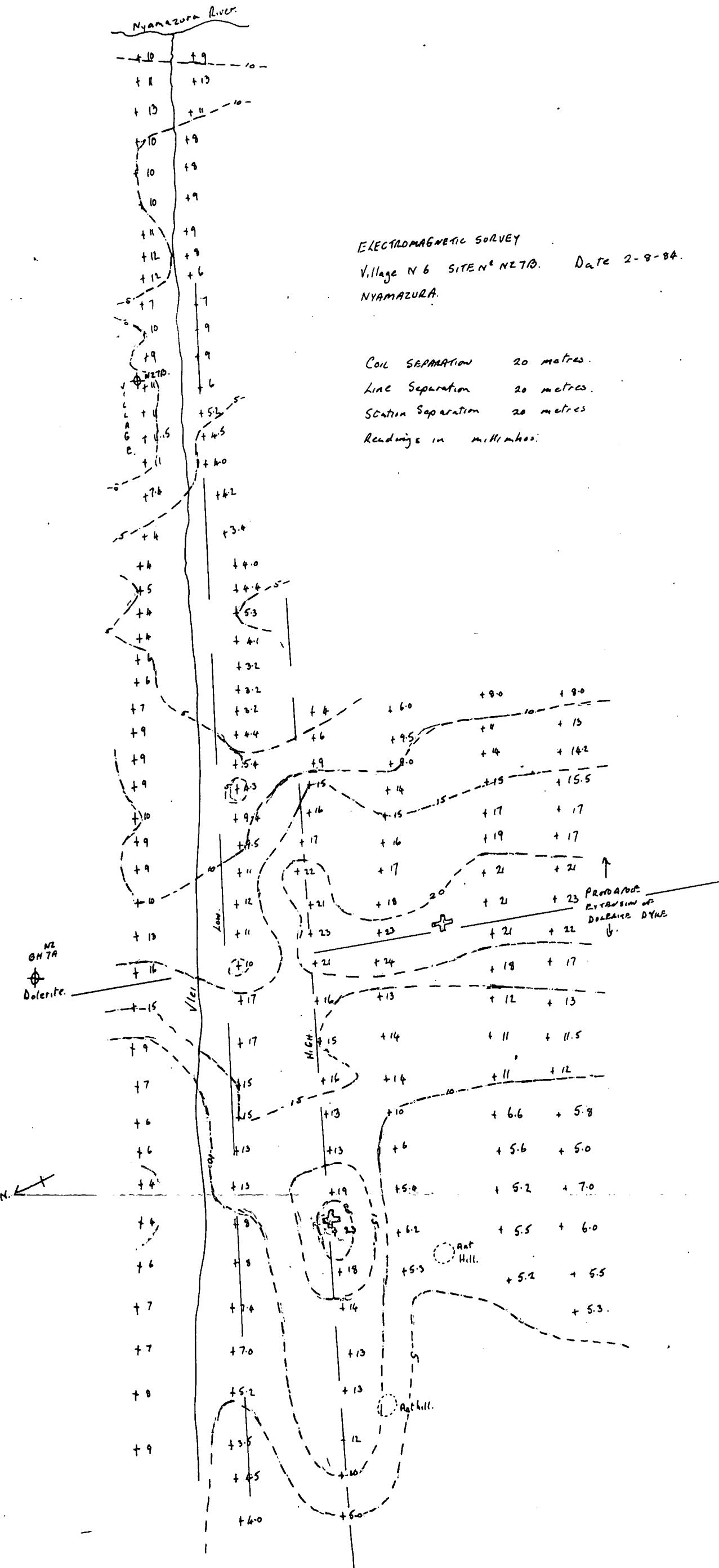
APPENDIX II

EM RESULTS

1. Village No. 1A      Borehole No. NZ9
2. Village No. 6      Borehole No. NZ7B
3. Village No. 7      Borehole No. NZ6B
4. Village No. 12      Borehole No. NZ2

ELECTROMAGNETIC SURVEY.  
Village 1A Borehole N° NZ 9 Nyamazura Date 3/7/94.



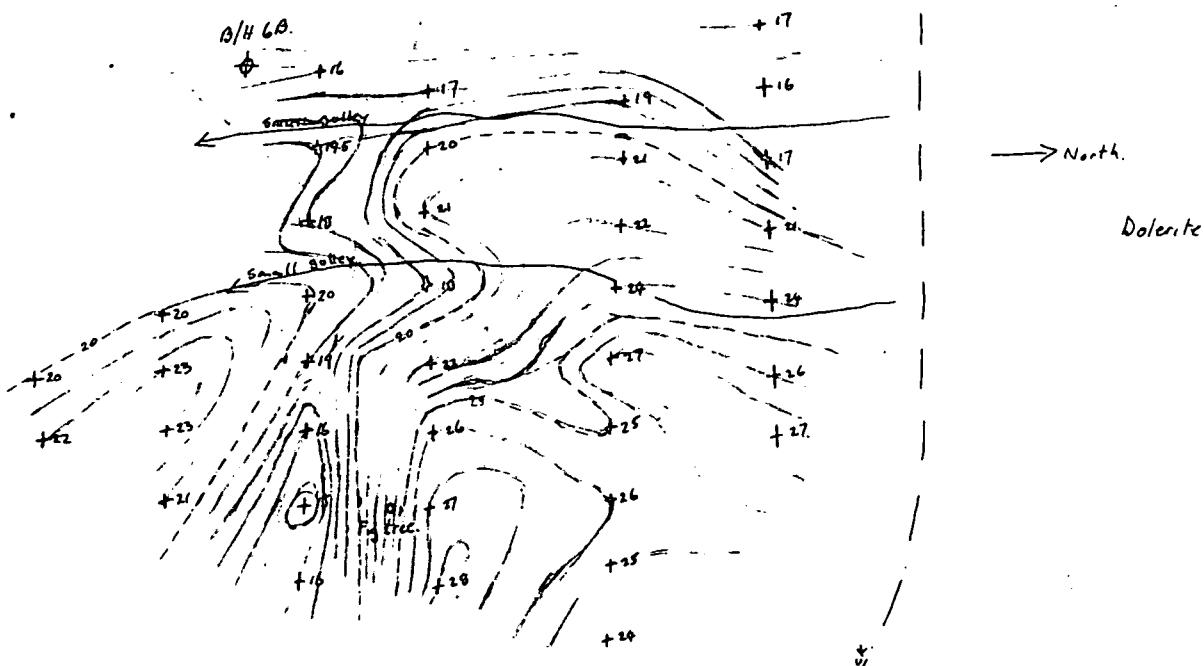


ELECTROMAGNETIC SURVEY.

Village 7 Site NZ GB Nyanzura.

Date 4/7/84.

Gneiss.



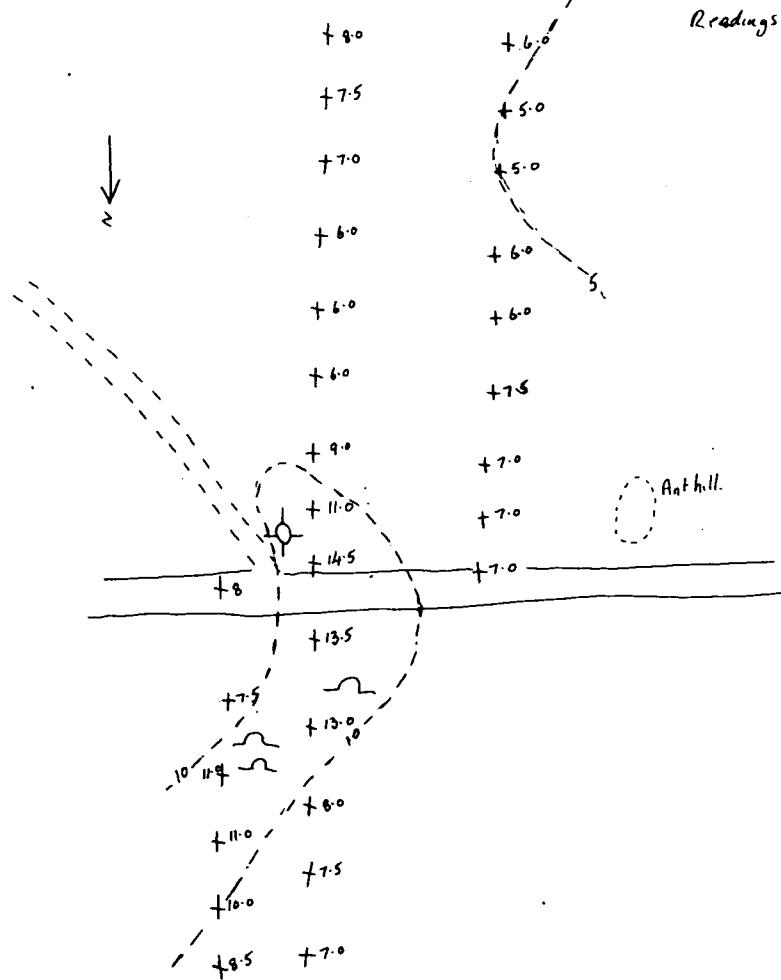
Station separation - 20 metres

Line separation ~ 20 metres.

Coil separation 20 metres.

Readings in millimhos.

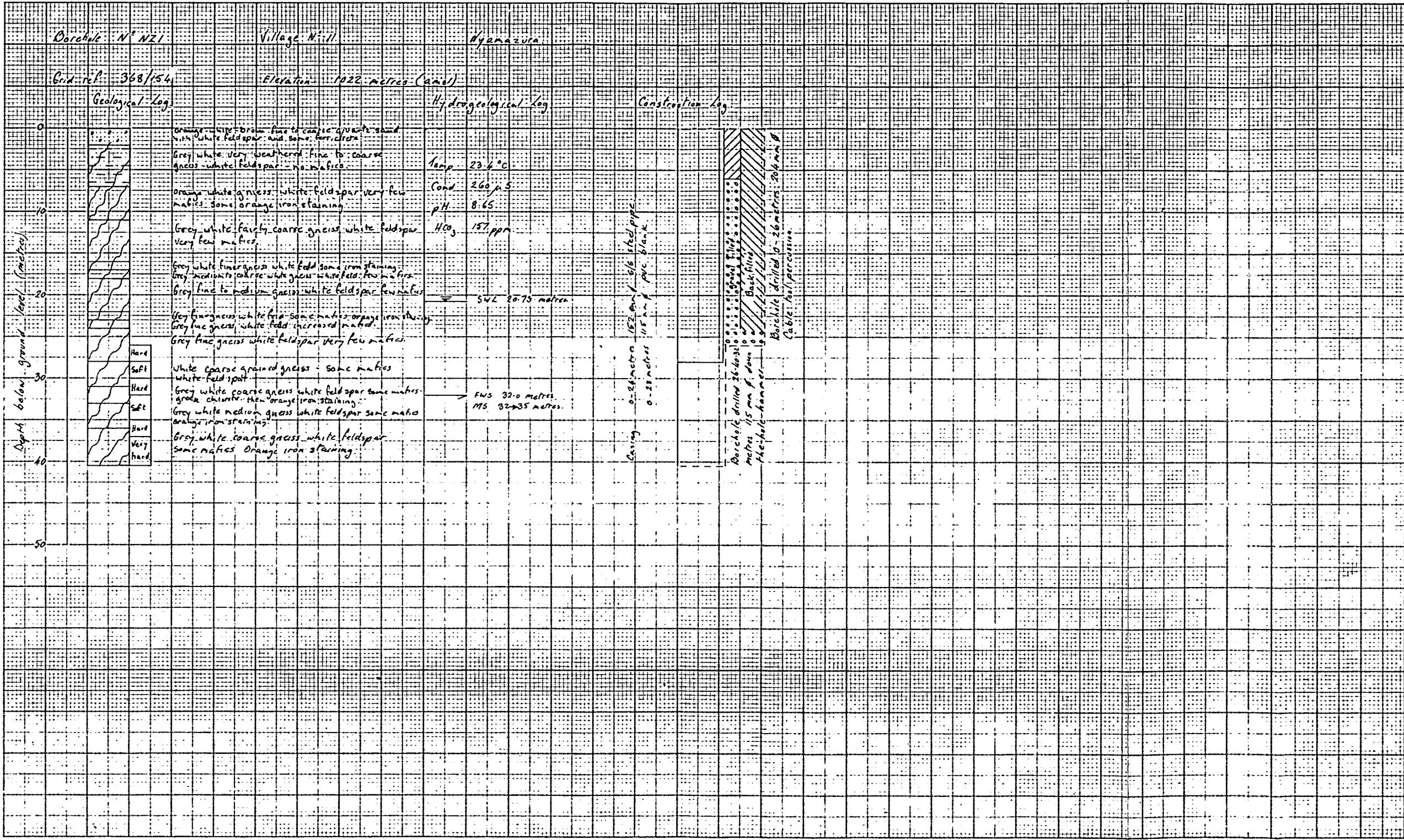
ELECTROMAGNETIC SURVEY.  
Village 12 N.Z.Z. NYAMAZURA 22-5-84  
Coil separation 40 metres  
Station Separation 20 metres.  
Readings in millimhos.

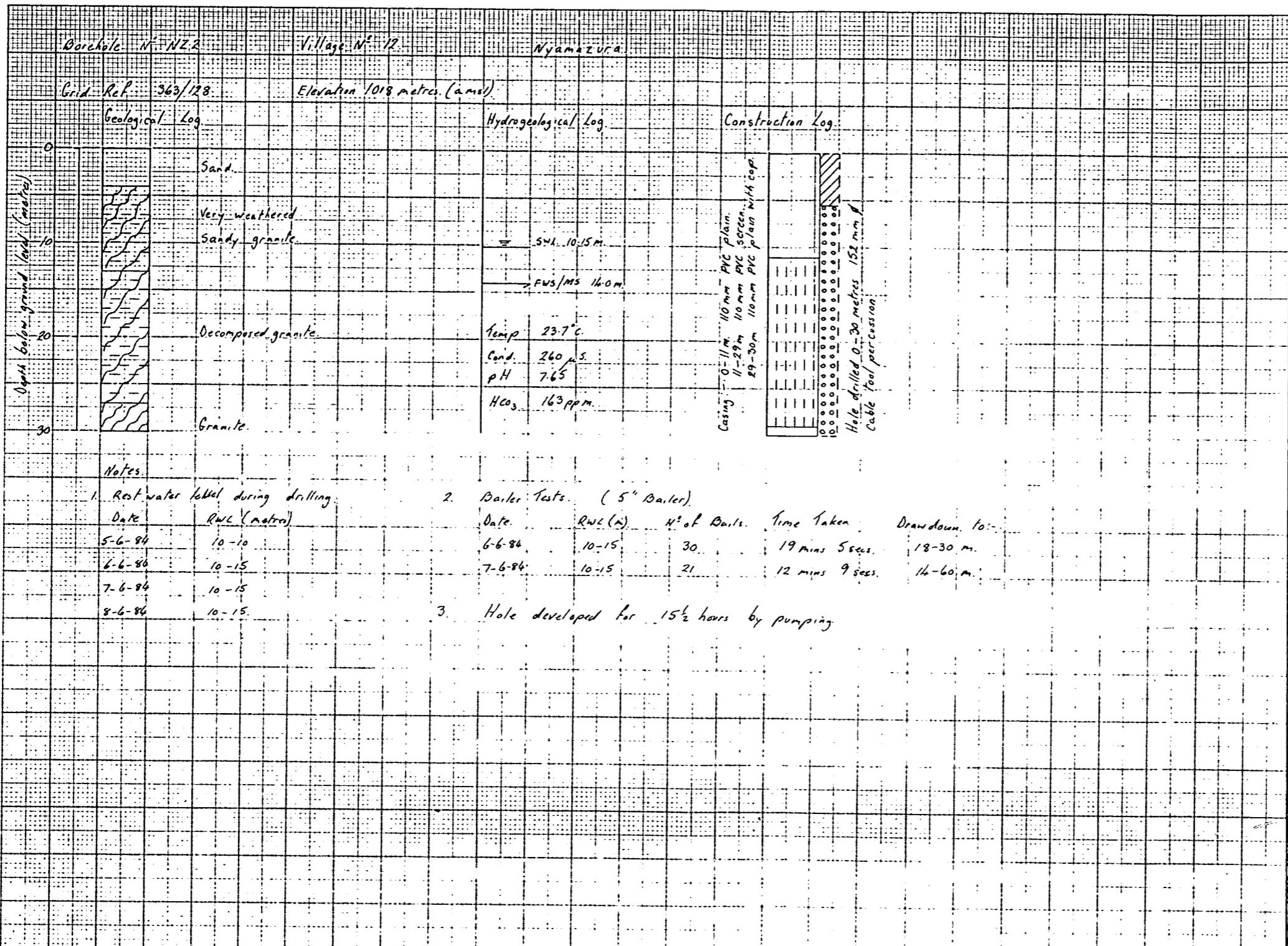


APPENDIX III

BOREHOLE LOGS

1	-	NZ1
2	-	NZ2
3	-	NZ3
4	-	NZ4A
5	-	NZ4B
6	-	NZ5
7	-	NZ6A
8	-	NZ6B
9	-	NZ7A
10	-	NZ7B
11	-	NZ8
12	-	NZ9
13	-	NZ10
14	-	NZ11A
15	-	NZ11B
16	-	NZ12A
17	-	NZ12B





Borehole N° NZ 4 A

Village N° 9

Njamatara

Grid ref: 325/202

Elevation: 10.75 metres (anil)

Geological Log

Hydrogeological Log

Construction Log

Metric scale (metres)

0

?

10

20

30

Notes:

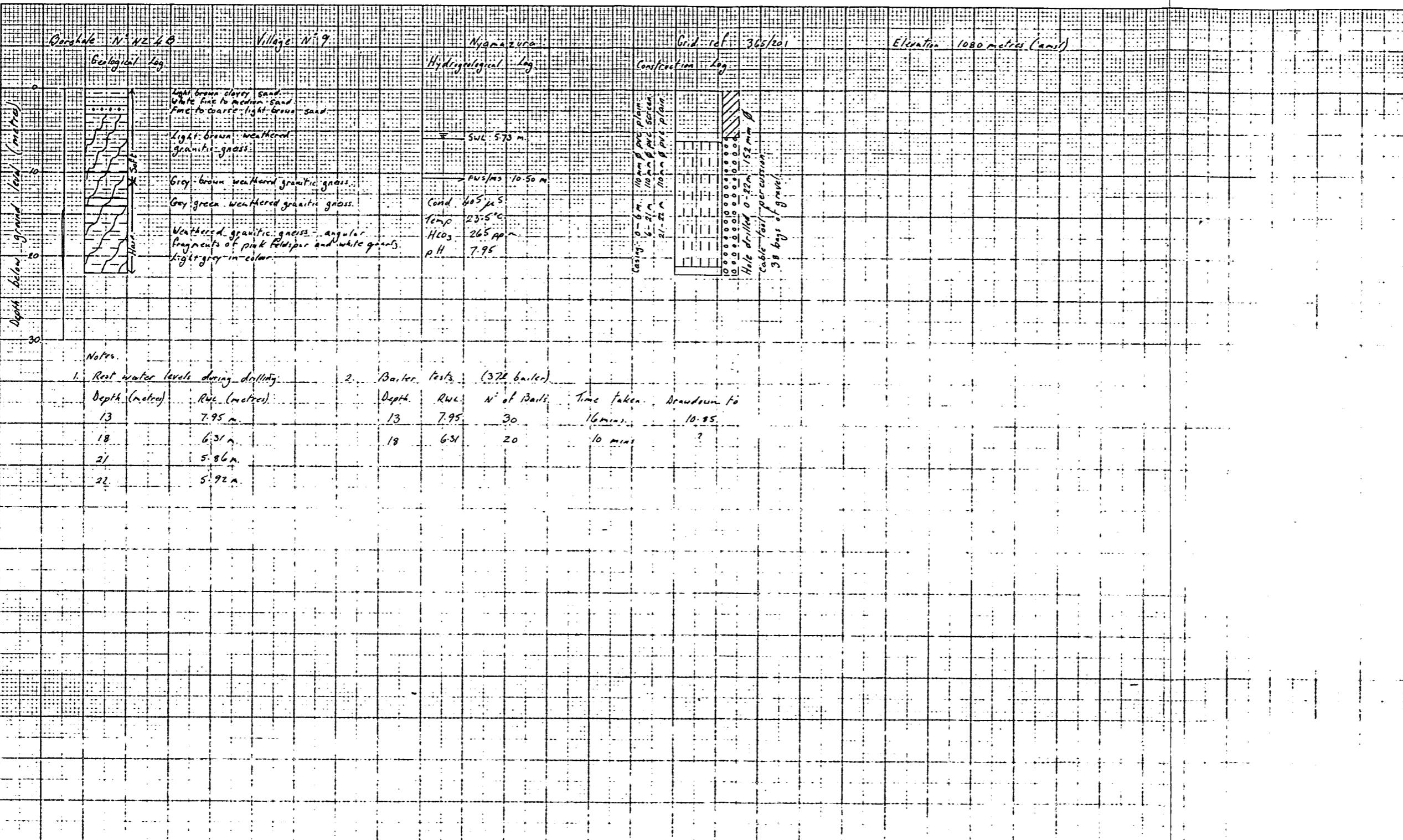
This hole was drilled at a site some 400 metres South of 3/4 N° NZ 4 B.

Hole stopped in  
hard granite

Hole dry

Hole bottomed out  
and bailed off.

Hole bottomed out  
at 152 m (460 ft)  
percussion



Borehole N° NZ 5

Village N° 5

Nianazura

Grid ref: 378/229

Elevation: 1100 metres (a.s.l.)

Ecological Log

Fine to coarse light brown sand with angular frags.  
of white feldspar and granite.

white sand of angular white feldspar and granite.

Very coarse white gneiss.

Medium grained white gneiss.

Light brown weathered white gneiss.

Medium grained white gneiss.

Coarse grained white gneiss.

Medium grained white gneiss.

Medium grained white gneiss. Some pink feldspar.

White medium grained gneiss.

Sparse orange white gneiss. Some pink feldspar.

Very coarse grained white gneiss.

Medium grained white gneiss.

30

Notes

1. Rest water levels during drilling

Date	depth (m)	rwle (m)
18-6-86	8.65	3.06
19-6-86	16.00	3.18
20-6-86	21.00	3.06
22-6-86	25.15	3.10

Hydrogeological Log

Construction Log

SUL 3.6 m  
FWS 1.00 m

MS 11.0 - 13.5 m

Cond 250 μS

Temp 23.9 °C

HCO<sub>3</sub> 140 ppm

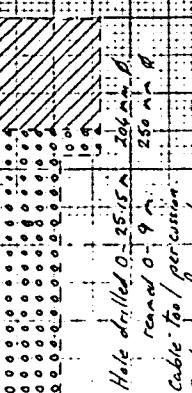
pH 7.80

WS 23-25° ms

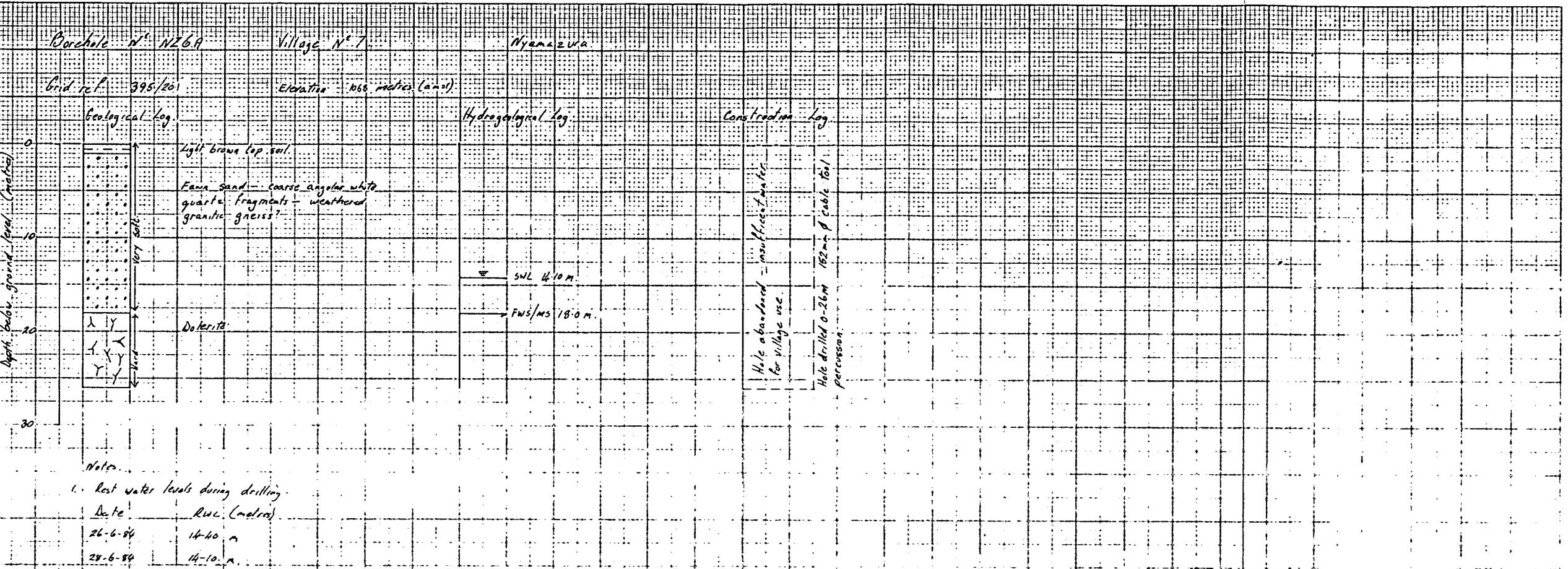
Casing 0 - 8.69 m / 10 mm per plan

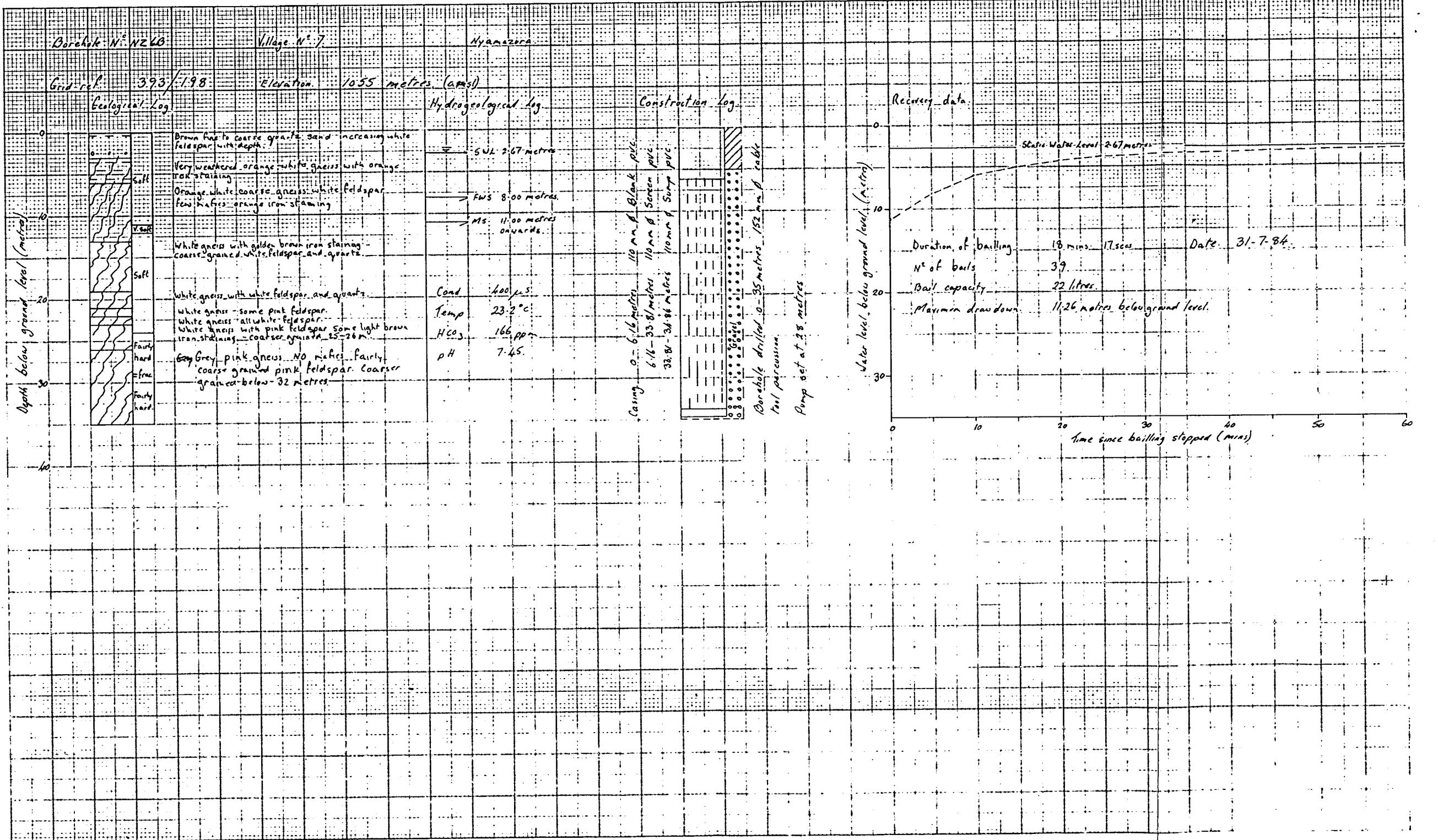
8.69 - 20.6 m / 10 mm per screen

20.6 - 25.15 m / 10 mm per plan



hole drilled 0 - 25.15 m - 26 and  
casing 0 - 9 m - 250 mm  
cable tie / per cent  
30 bags of gravel







Bore hole N° NZ 71

Village N° 6

Nyamizi

Gndscrpt. 426/19

Elevation 1045 metres (a.s.l.)

## Hydrogeological

## Construction

## Recovery Data

			Golden brown medium - coarse angular sand - no c.
		Very soft	Golden brown medium - v coarse clayey sand.
			Golden brown medium to very coarse angular to s. angular sand gritty and small pebbles. Some lithoclasts.
			Golden brown medium to very coarse angular sand iron stained quartz and white feldspar.
0			Very weathered white gneiss - brown iron stain medium grained. No mafics, white feldspar.
10			Orange iron staining reduced with depth
20		soft	White-pink coarse grained gneiss. No mafics, pink feldspar. White gneiss-white feldspar no mafics. Some ep. pink feldspar. 21-22 m.
25		Hard	Weathered white gneiss coarse grained few mafics. White medium grained gneiss 25% mafics white feldspar. Very hard white medium grained gneiss. white feldspar. Some pink feldspar. Very few mafics.
30		soft	White red-coarse gneiss white feldspar no mafics.
		Hard	Grey fine gneiss white feldspar few mafics. Some orange iron staining.

Temp	24.2°C
Cond	320 $\mu$ s
pH	7.35
HCO <sub>3</sub>	200 pp

4-78-205

Rising 0-16 meters 110 m/s blank face  
16-31 meters 110 m/s green glass  
31-32 meters 110 m/s sand glass

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Table 10. Percentage of 1000 individuals available for percutaneous absorption

vol. 10, no. 1, March 1961

A line graph with 'Number of seconds' on the x-axis and 'Level of accuracy (accuracy)' on the y-axis. The x-axis has major ticks at 10, 20, and 30. The y-axis has major ticks at 10, 20, and 30. A single data point is plotted at approximately (15, 25).

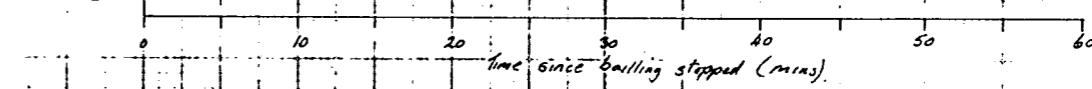
Number of seconds	Level of accuracy (accuracy)
15	25

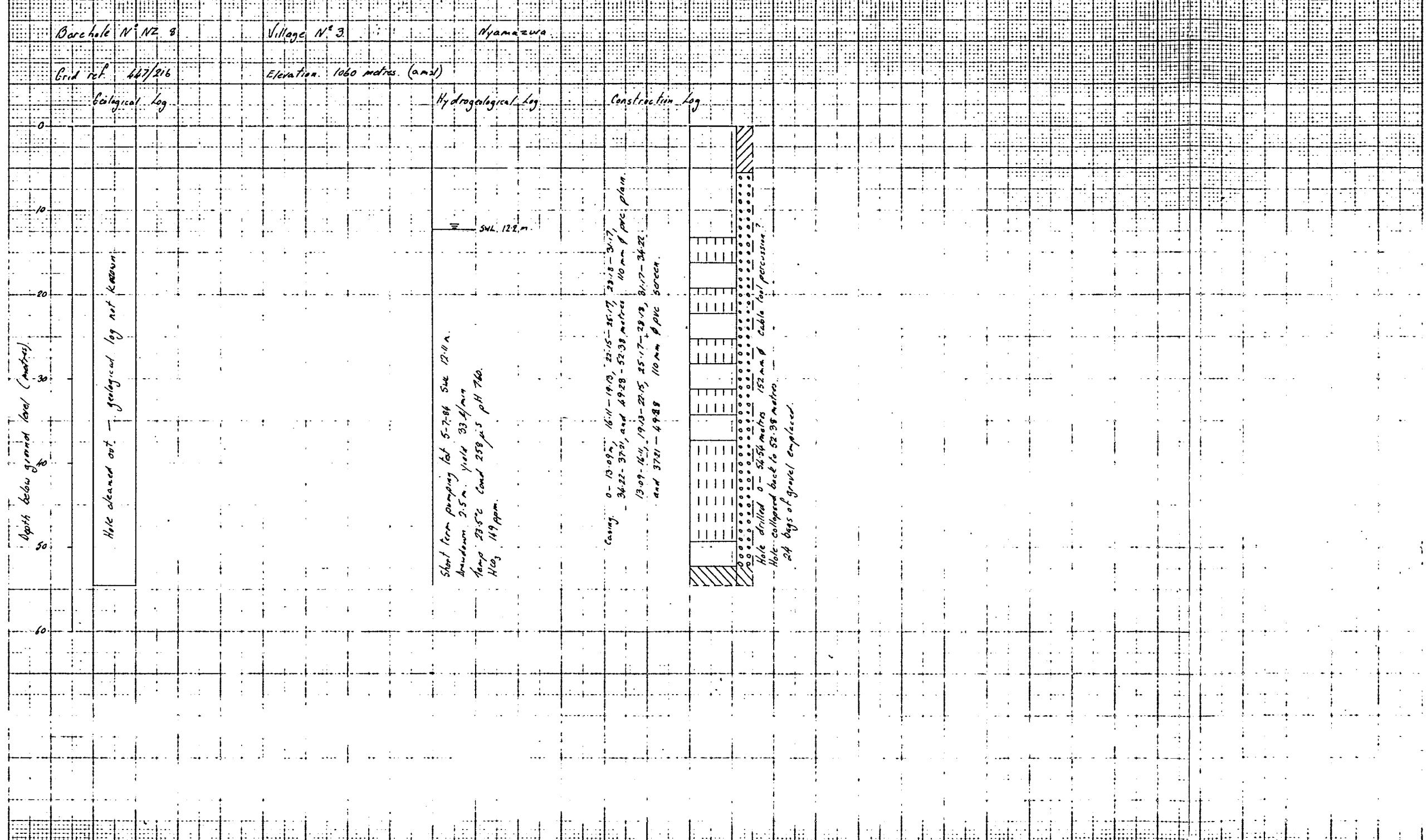
Duration  
No. of bar  
Dial cap  
Maximum  
Yield  
Specific

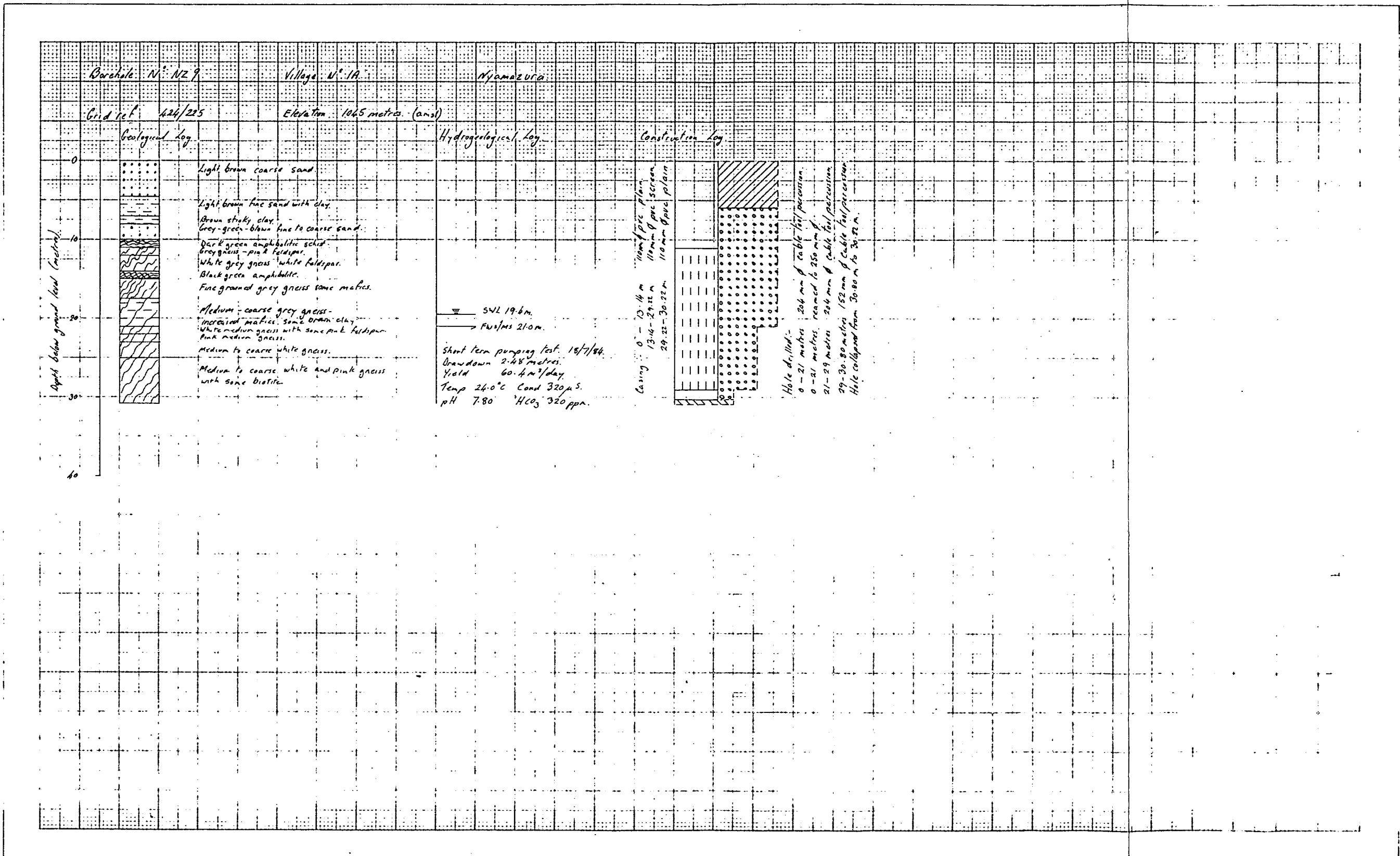
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g	30
	49
	22
own	26
total	51.5
	33.

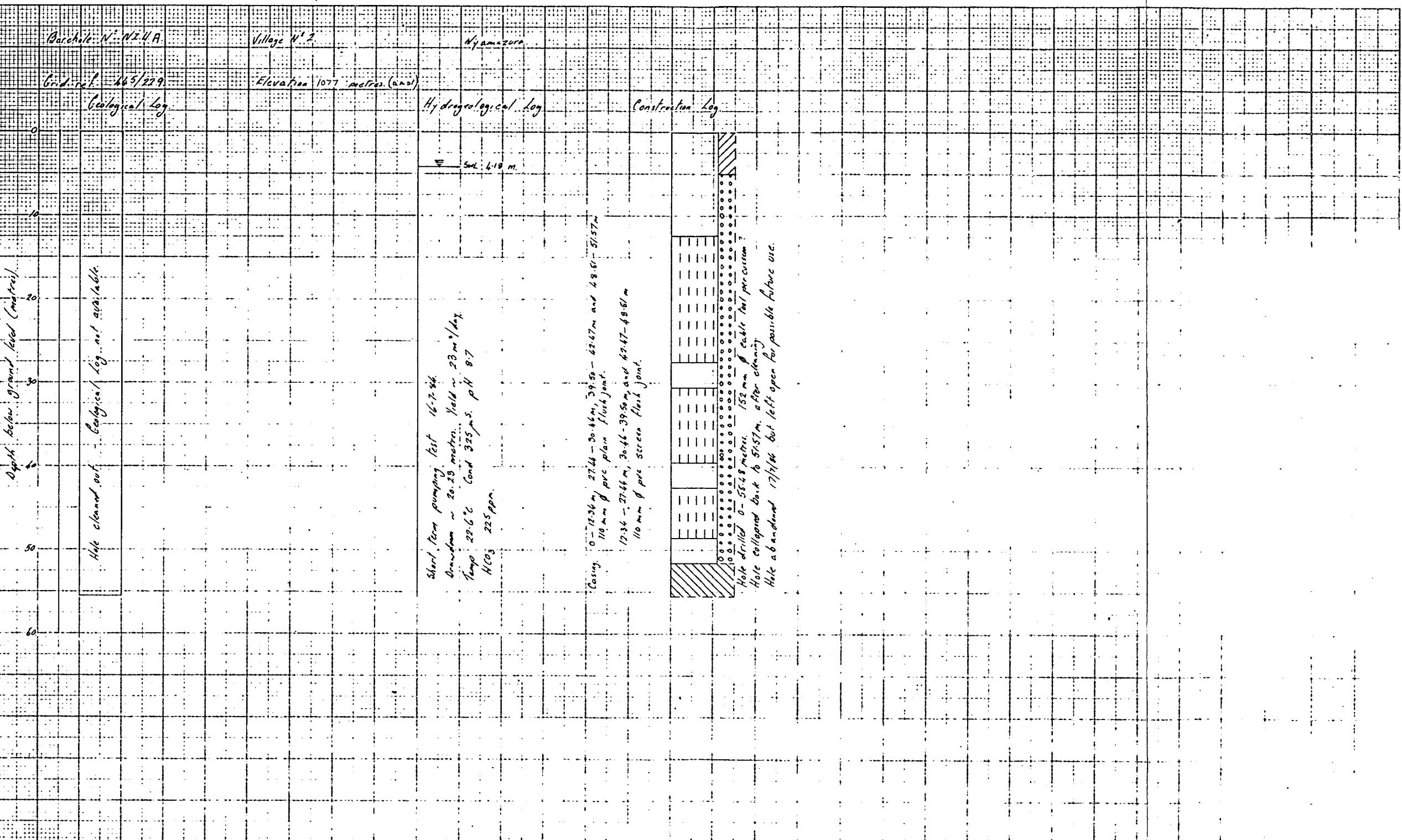
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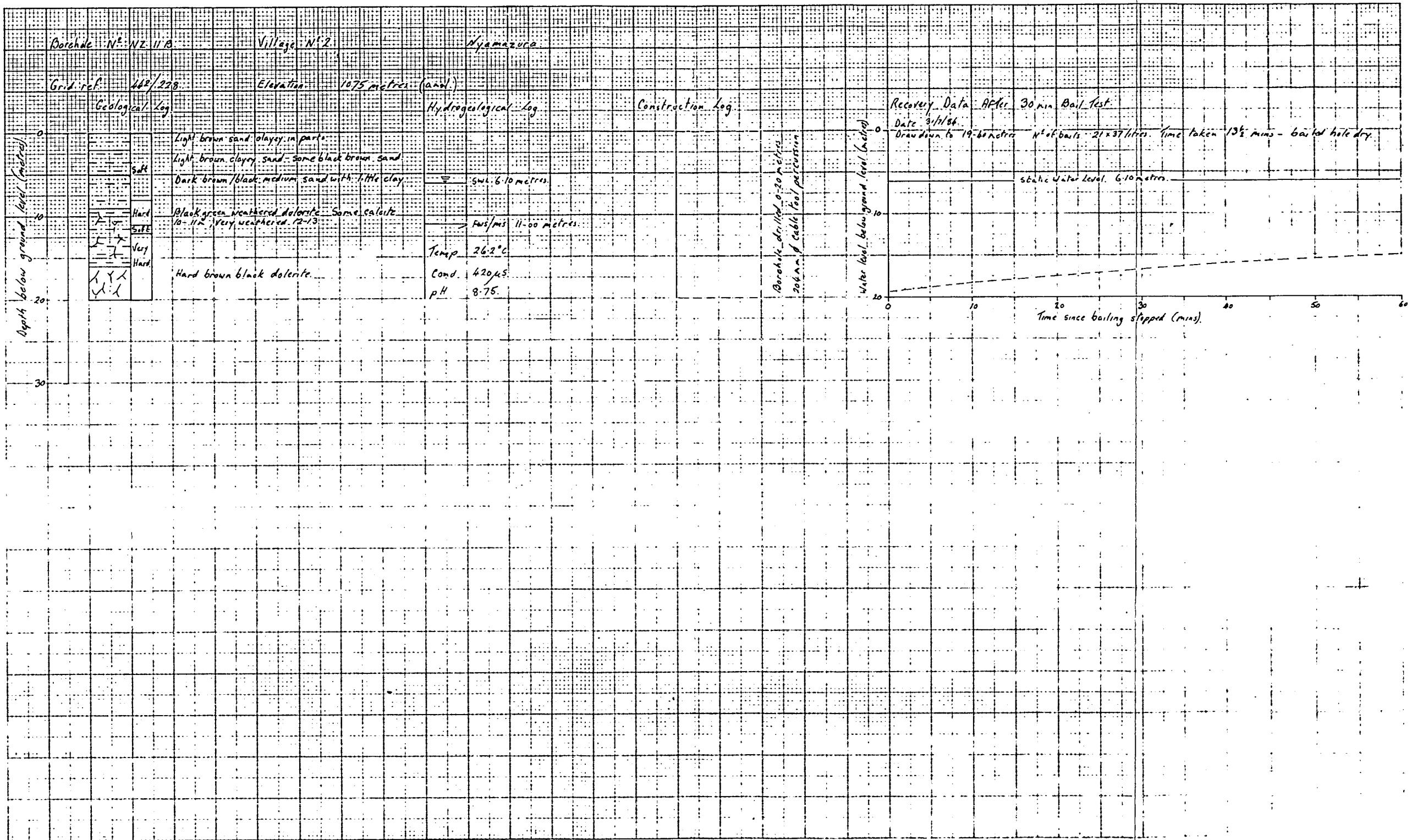


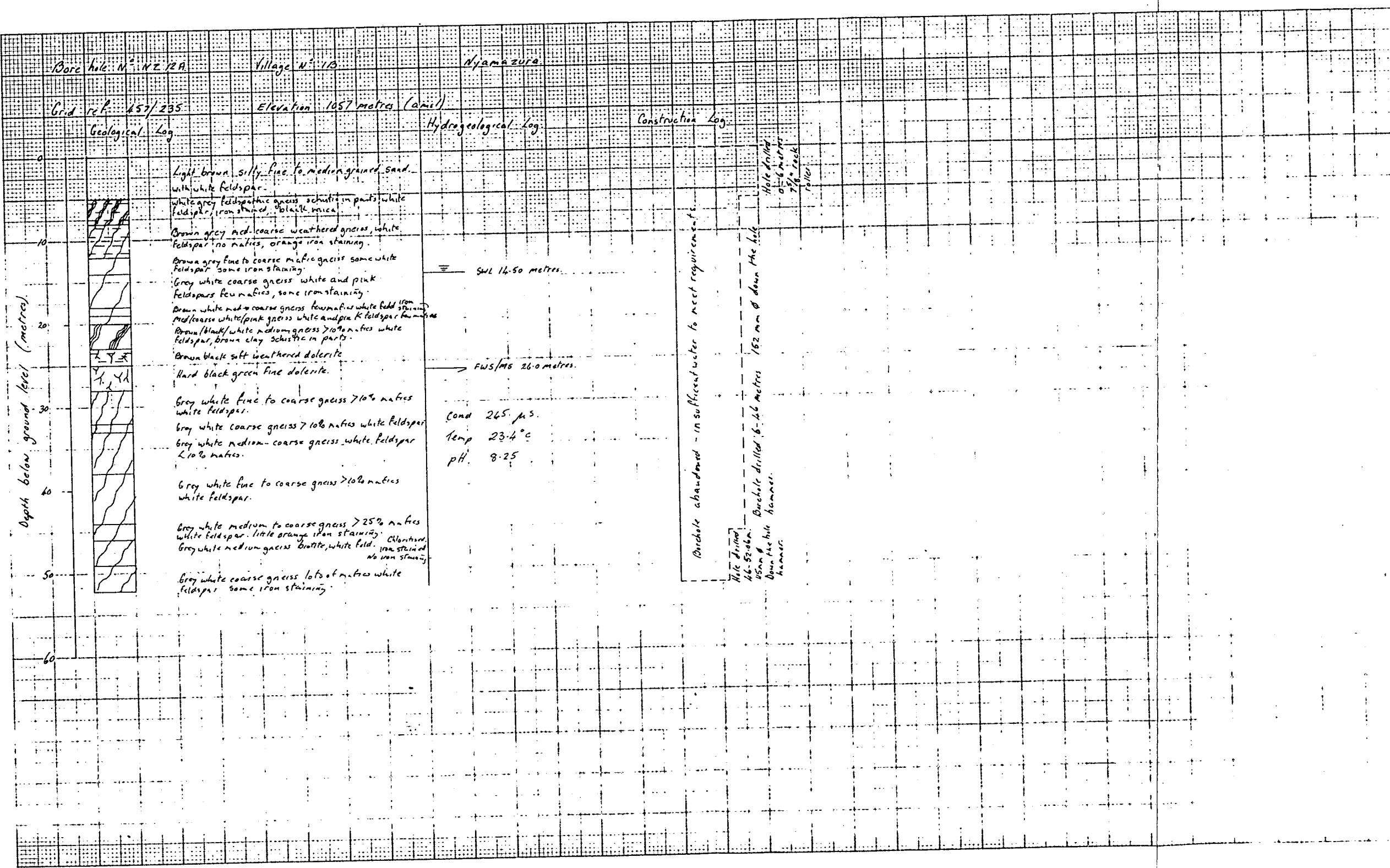


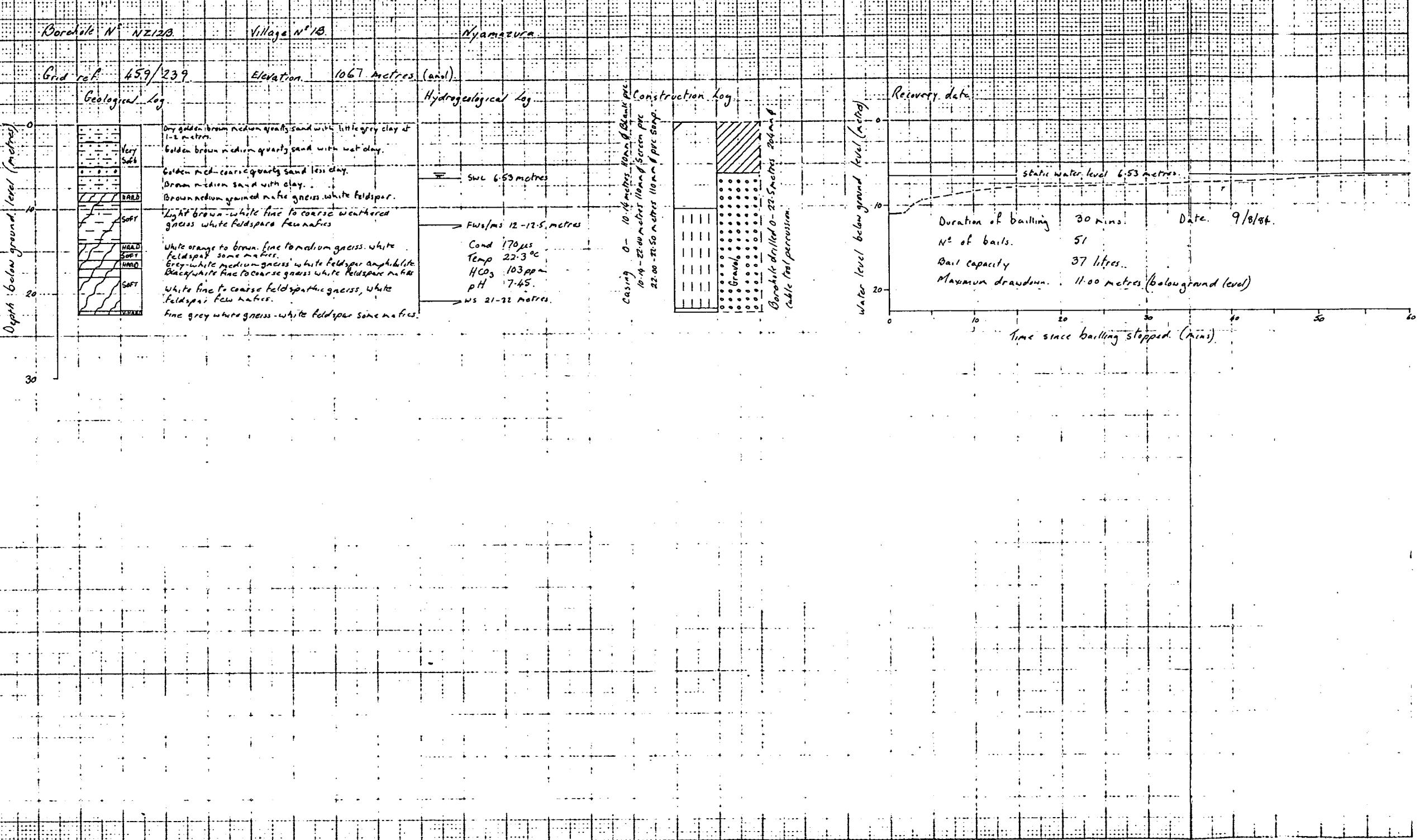












APPENDIX IV  
PUMPING TEST DATA

<u>Borehole No.</u>	<u>Village No.</u>
NZ4B	9
NZ5	5
NZ6B	7
NZ7A	6
NZ7B	6
NZ8	3
NZ9	1A
NZ10	4
NZ11A	2
NZ11B	2
NZ12A	1B
NZ12B	1B

## Constant Yield Pumping Test

Borehole N° NZ4 (13)

Date 27-4-84

Time test started 11-00 am.

Pump cylinder @ 19.64 m Reciprocating Pump

Time (mins)	Water level (metres)	Drawdown (metres)	Yield: per 5 gals.	Temp. °C	Cond. μS.	p.H.
55	5.913					
57	5.915					
00	Pump test started					
00-15	6-11	0.195				
00-30	6-19	0.275				
00-45	6-25	0.345				
1-00	6-33	0.415				
-15	6-40	0.495				
1-30	6-45	0.535				
2-00	6-56	0.655				
2-30	6-62	0.715				
3-00	6-67	0.765				
3-30	6-73	0.825				
4-00	6-80	0.895				
4-30	6-82	0.915				
5-00	6-87	0.965	27 secs.			
5-00	6-91	1.005				
5-00	6-92	1.015				
5-00	6-99	1.035				
6-00	7-00	1.095				
7-00	7-07	1.165		23.8	420	
10-00	7-10	1.195				
11-30	7-13	1.225				
12-00	7-15	1.245			7.25	
14-00	7-19	1.285				
16-00	7-22	1.315				

## Constant Yield Test (Cont.)

Borehole No NZ 4 (c)

	Water level (metres)	Drawdown (metres)	Yield Sus per 5gals	Temp °C	Cond $\mu\text{S}$	pH
7-00	7.23	1.325				
8-00	7.28	1.375	25½			
9-00	7.34	1.435				
10-00	7.37	1.465	26	23.7	420	7.45
11-00	7.42	1.515				
12-00	7.445	1.54				
13-00	7.48	1.575				
14-00	7.50	1.595				
15-00	7.52	1.615				
16-00	7.53	1.625	25	23.8	420	7.25
17-00	7.57	1.685				
18-00	7.61	1.705	25	23.8	420	7.25
19-00	7.65	1.745				
20-00	7.68	1.775	25	23.8	420	7.35
21-00	7.71	1.805	25	23.6	420	7.45
22-00	7.74	1.835	26	23.6	420	7.45
23-00	7.78	1.875	25½	23.6	420	7.45
24-00	7.79	1.885	25	23.6	420	7.45
25-00	7.81	1.905	26	23.6	420	7.45
26-00	7.83	1.925	26	23.6	415	7.45
27-00	7.85	1.945	25	23.6	415	7.50
28-00	7.86	1.955				
Pumping	test stopped at 17-00					

Recovery Pumping Test.

Borehole No. NZ 4 (B).

Date 27-4-84.

Time test started 11-00 pump stopped at 17-00.

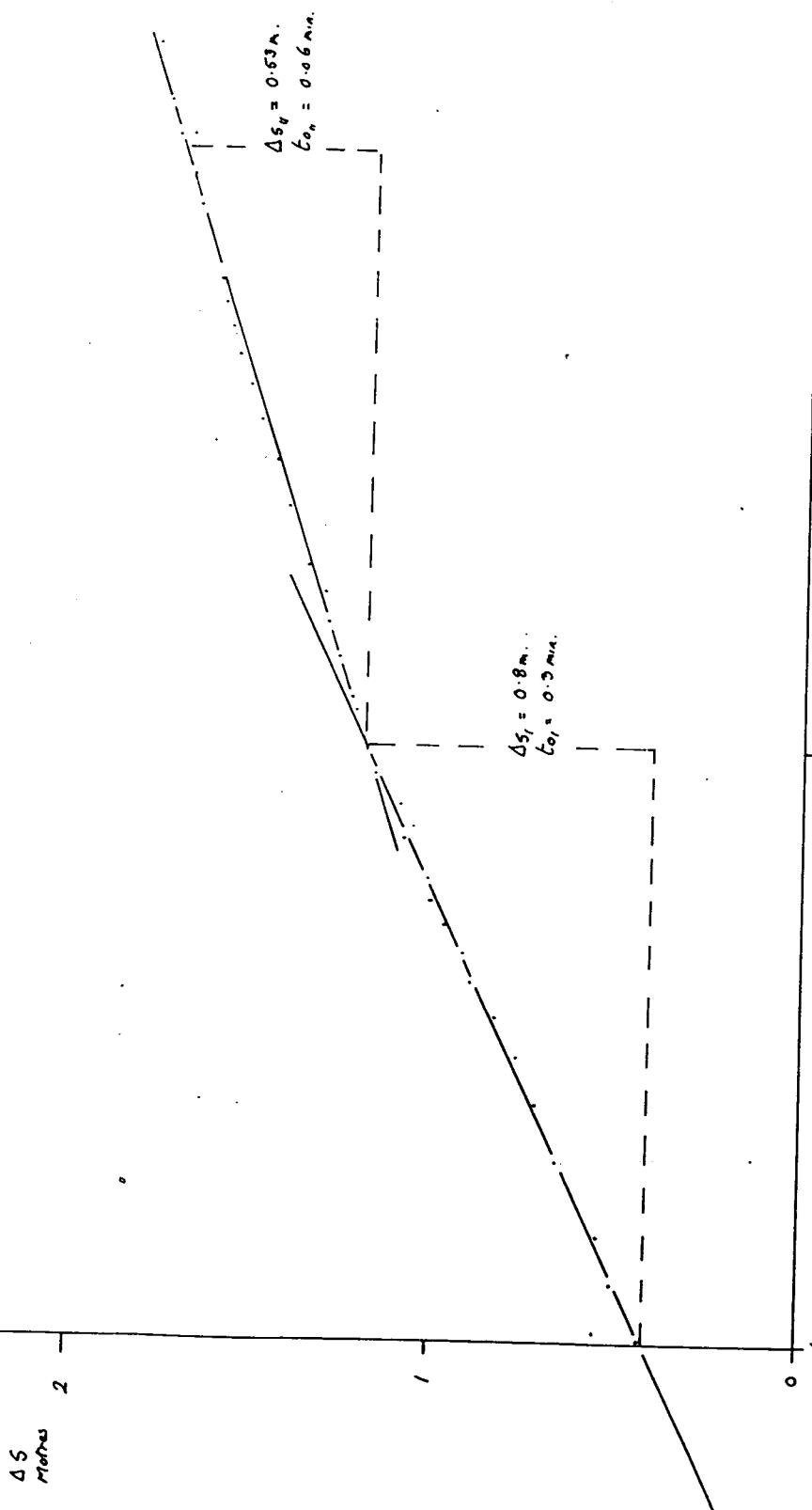
Pump Cylinder At 19.64 m bgl. Static water rest level before test 5.915 metres

Max. min. of water level when pump stopped 7.86 metres below ref. point (1.945m)

Ref point since stopped mins)	Water level (metres)	top of pvc casing 5-5'	Residual Recovery (metres)	Drawdown (metres)	10 cms. agl.		5-5' Water level (metres)	Residual Recovery (metres)	Drawdown (metres)	t/c 19
					t/b	time since pump stopped (mins)				
0-15	7.72	0.14	1.805	1441		20-00	6.55	1.31	0.635	
0-30	7.68	0.18	1.765	721		25-00	6.51	1.35	0.595	15.4
0-45	7.48	0.38	1.565	481		30-00	6.48	1.38	0.565	13
1-00	7.37	0.49	1.455	361		35-00	6.455	1.405	0.54	11.3
1-30	7.22	0.64	1.305	241		40-00	6.43	1.43	0.515	10
2-00	7.09	0.77	1.175	181		45-00	6.41	1.45	0.495	9
2-30	7.00	0.86	1.085	145		50-00	6.39	1.47	0.475	8.2
3-00	6.94	0.92	1.025	121		55-00	6.375	1.485	0.46	7.5
3-30	6.90	0.96	0.985	103.9		60-00	6.36	1.50	0.4485	7
4-00	6.87	0.99	0.955	91	Measurements stopped at 60 mins (18-00)					
4-30	6.84	1.02	0.925	81						
5-00	6.82	1.04	0.905	73						
5-30	6.78	1.08	0.865	61						
6-00	6.74	1.12	0.825	52.4						
6-30	6.72	1.14	0.805	46						
7-00	6.70	1.16	0.785	41						
7-30	6.68	1.18	0.765	37						
8-00	6.64	1.22	0.725	31						
8-30	6.615	1.245	0.695	26.7						
9-00	6.591	1.269	0.676	23.5						
9-30	6.57	1.29	0.655	21						

Constant Yield Pumping Test.  
 Borehole No. N243 Village No. 9 INYAMAZURA.  
 Date 27-4-1988  
 Time Test Started 11-00 A.M.  
 Jacob Straight Line Analysis.

$$Q = \frac{54 \text{ l/min.}}{5.915 \text{ m}^3/\text{min.}} = \frac{2.30 \text{ Q}}{4\pi d_3} \quad K_5 = \frac{2.30 \times 54 \times 60 \times 24}{1600 \times 4\pi \times 0.80} \quad K_6 = \frac{2.30 \times 54 \times 60 \times 24}{1600 \times 4\pi \times 0.53} = 16.38 \text{ m}^3/\text{day/m.} \quad = 24.73 \text{ m}^3/\text{day/m.}$$



Recovery test.

Bore hole N° N240 Village n° 9 MUYAMAZA.

DATE 27-4-1984 Time started pump 1700 hrs

Time stopped pump 11-00 am.

Time recovery started.

Max.  $\Delta s$  = 7.86m below ref level.

Since  $\Delta s$  = 5.95m above below ref level.

3 -

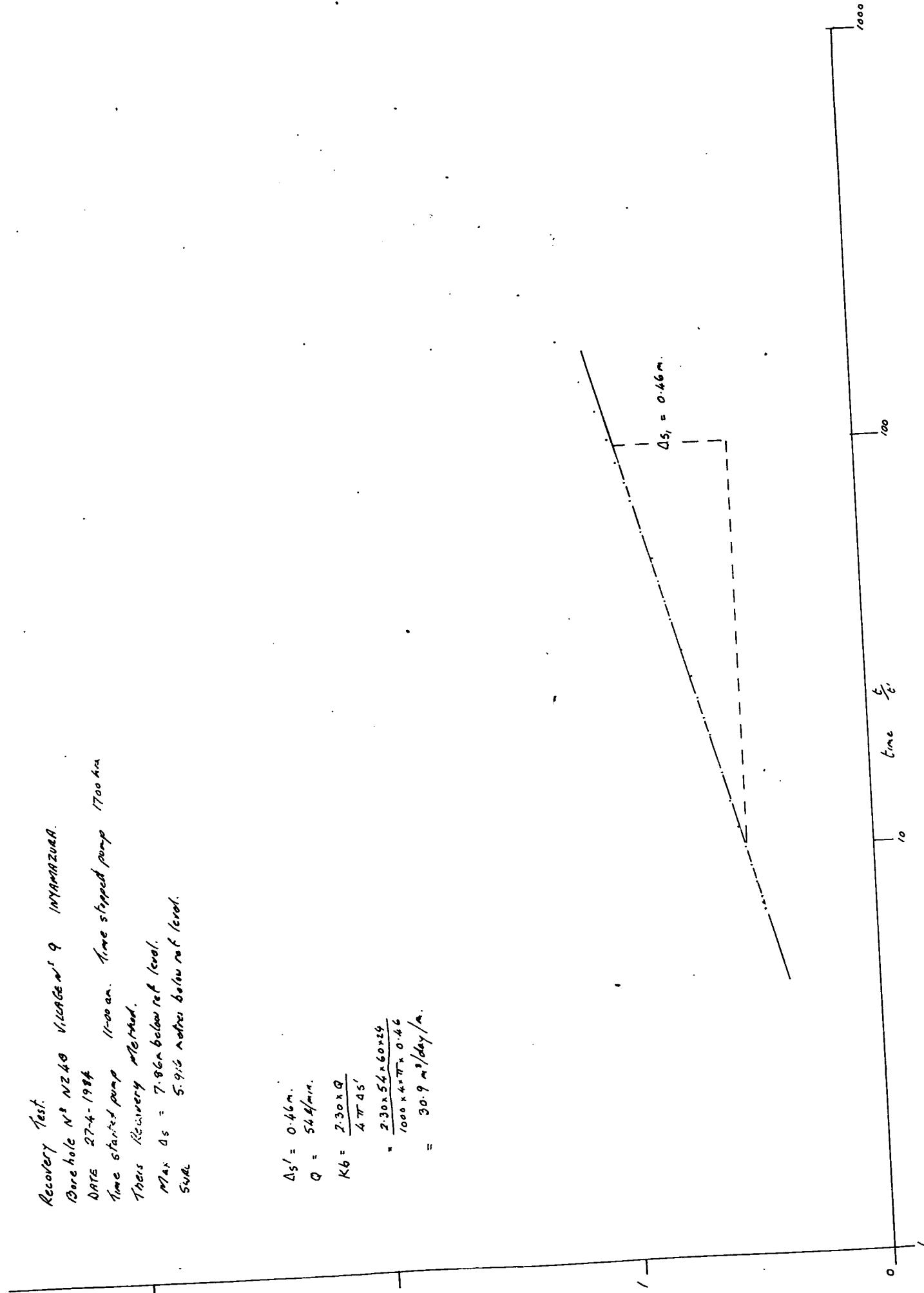
$$\Delta s' = 0.46m.$$

$$Q = 54 \text{ l/min.}$$

$$Kb = \frac{2.30 \times Q}{4 \pi 45'} \\ * \frac{2.30 \times 54 \times 60 \times 24}{1000 \times 4 \pi \times 0.46} \\ = 30.9 \text{ m}^3/\text{day/m.}$$

2 -

Residual drawdown



Constant Yield Pumping Test.

Borehole N° NZ 5. VILLAGE 5 in YAMAZURA.

DATE 2-7-84.

Time Test Started 13:00 hrs.

Pump cylinder ~ 24 metres below ref. point.

Ref point top of casing 0.60 m above ground level

Pump used - Dando operated single action reciprocating pump.

Date	SWL (m)					
11-54	3.665 m.	- before pump erected.				
12-50	3.965					
12-53	3.920	Water level still recovering after 5 minute yield test				
12-55	3.89		to set pump yield.			
12-57	3.88					

Time (mins)	SL (m)	D.D. (m)	Yield <sup>secs</sup> /gal.	Temp °C	Cond $\mu S$ .	pH
00-15	5.52	1.855				
00-30	6.22	2.555				
01-00	7.00	3.335	34.5	24.0	260	7.15
1-15	7.48	3.815				
1-30	8.02	4.355	32	24.0	240	
2-30	8.89	5.225	32	23.8	240	
3-00	9.31	5.645				
3-30	9.62	5.955				
4-00	9.68	6.015				
4-30	9.66	5.995				
5-00	9.73	6.065				
6-00	9.78	6.115				
7-00	9.96	6.295				
8-00	10.46	6.795				
9-00	10.98	7.215				

Constant TIME	Yield Test (Cont.) WL D.O.	Borehole YIELD	N° N25 Temp. 32	Village N° 5 Cond.	pH.
10-00	11.56	7.895			
12-00	12.30	8.635			
14-00	12.87	9.105			
16-00	13.37	9.605			
18-00	13.76	10.095			
20-00	14.04	10.375			
25-00	14.59	10.825	32	23.8	7.25
30-00	15.08	11.315	32		
35-00	15.62	11.955	31		
40-00	16.00	12.335			
45-00	16.46	12.795	32		
50-00	16.76	13.095			
55-00	17.10	13.435	33	24.	23.0
60-00	17.25	13.585	32		
74-00	17.80	14.135	32	24	23.0
80-00	Dipper could not pass		32		
90-00	beyond 17.80 metres -		32		
100-00	pipe socket in the way.		32	24.	23.0
110-00	Continued to measure		32		
120-00	yield until that declined		33		
125-00	and then stabilised -		34	Time (mins)	Yield <sup>secs</sup> /gal.
130-00	took this to be an		34.5	165	36.5
135	indication of a steady		35	170	36.5
140	state yield?		36	175	36.5
145			36	180	Test pump stopped.
150			36.5		
155			36.75		
160					

Recovery Test.

Borehole N° NZ5 Village N° 5 IN YAMAZURA Date 2-7-84.

Borehole diameter 110 mm Ø. Raising Main. 2"Ø C-I s/s pipe.

No water leakage from Foot valve.

Time pump test started 13-00 hrs.

Time pump stopped 16-00 hrs.

Pump cylinder at 24 metres below ref point.

Static water level before start of test. 3.665 metres (b.ref. point)

Maximum water level when pump stopped 20-22 metres. (brp)

Time since pump stopped (Mins)	Water level (metres)	Recovery (S-S <sub>1</sub> )	Residual draw-down (metres)	Time since test started Time since pump stopped		t/t <sub>1</sub>
				Time since test started	t/t <sub>1</sub>	
00 - 15	17.80	17.80	14.135	17.80	1	1
00 - 30	16.88	16.88	13.215	16.88	1	1
00 - 45	16.00	16.00	12.335	16.00	1	1
1 - 00	15.50	15.50	11.835	15.50	1	1
1 - 15	14.75	14.75	11.085	14.75	1	1
1 - 30	14.26	14.26	10.595	14.26	1	1
1 - 45	13.80	13.80	10.135	13.80	1	1
2 - 00	13.00	13.00	9.335	13.00	1	1
2 - 30	12.10	12.10	8.435	12.10	1	1
3 - 00	11.20	11.20	7.535	11.20	1	1
3 - 30	10.50	10.50	6.835	10.50	1	1
4 - 00	9.95	9.95	6.285	9.95	1	1
4 - 30	9.50	9.50	5.835	9.50	1	1
5 - 00	9.10	9.10	5.435	9.10	1	1
6	8.50	8.50	4.835	8.50	1	1
7	8.11	8.11	4.445	8.11	1	1
8	7.83	7.83	4.165	7.83	1	1
9	7.62	7.62	3.955	7.62	1	1

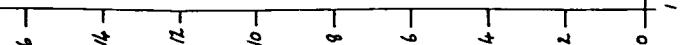
Recovery Test (contd)		B/H N° N.25	Village N° 5	Date 2-7-84
Time since pump stopped. (mins.)	Water level (metres)	Recovery (S-S') (Metres)	Residual Drawdown (Metres)	% E.
10	7.43		3.765	19
12	7.16		3.495	16
14	6.94		3.275	13.9
16	6.75		3.085	12.25
18	6.60		2.935	11
20	6.48		2.815	10
25	6.22		2.555	8.2
30	6.04		2.375	7
35	5.80		2.135	6.1
40	5.62		1.955	5.5
45	5.50		1.835	5
50	5.41		1.745	4.6
55	5.32		1.655	4.3
60	5.25		1.585	4
End of measurements.				
20.22	Q = 47.3	m³/day.		
3.665				
16.565				

Recovery Test  
 Borehole N° 25 Village N° 5 INAMAZURA  
 Date 2-7-84 Their Recovery method.  
 Time pump started 13-00  
 Time pump stopped 16-00  
 Maximum Drawdown ~ 20-00 metres.  
 Swell 3.665 metres.  
 $Q = 4.8 \text{ m}^3/\text{day}$ .

$$K_6 = \frac{2.30 \times Q}{4\pi \Delta s}$$

$$\begin{aligned}
 \text{for } \Delta s = 3.4 \text{ m} & \quad \text{for } \Delta s = 10.5 \text{ m} \\
 K_6 = \frac{2.30 \times 4.8}{4\pi \times 3.4} & \quad K_6 = \frac{2.30 \times 4.8}{4\pi \times 10.5} \\
 & = 1.2 \text{ m}^3/\text{day}
 \end{aligned}$$

Residual drawdown  $\Delta s$  (metres)



1000

100

$t/t'$

$\Delta s = 3.4 \text{ m}$ .

$\Delta s = 10.5 \text{ m}$ .

Constant Yield Pumping Test  
Borehole N° 112 5 Village N° 5: INHAMAZURA  
Date 2-7-81 Jacob Straight Line Analysis.  
Static Water Level 3: 665 metres.

$$\Phi = 37.5 \text{ l/min.}$$

$$= 54 \text{ m}^3/\text{min.}$$

$$Kb = \frac{2.30 \Phi}{4\pi f A_s}$$

$$= \frac{2.30 \times 54}{4\pi f 5}$$

$$= 2.0 \text{ m}^3/\text{day/m.}$$

Level at which pump started to suck air and yield decline.

$\Delta s = 5.0 \text{ m}$  ← Aquifer becomes unconfined?

$t = \text{days}$

1000  
100  
10  
2

Constant Yield Pumping Test.

Borehole N° NZ 63. Village N° 7 Nyamazura. 2-8-84.

Time Test Started. 11-30 a.m.

Pump Cylinder Depth. 31-82 metres.

Ref point. 0-15 m. a.g.l.

Pump used - Dando operated single action reciprocating pump.

Time Rest Water Level (metres) below ref point)

11-00 am. 2-82

11-05 3-07

11-10 3-05

11-15 3-03

11-20 3-02

11-25. 3-00

11-29 2-99.

Time (mins)	Water Level (metres)	Drawdown (metres)	Yield. (Secs / 20l)	Temp. (°C)	Conductivity. μs.	pH.
0-15	4-23	1-41				
0-30	4-85	2-03				
0-45	5-56	2-74				
1-00	5-95	3-13				
1-15	6-30	3-48				
1-30	6-58	3-76				
1-45	6-76	3-94				
2	6-92	4-10				
2-30	7-12	4-30				
3	7-28	4-46				
3-30	7-50	4-68				
4	7-60	4-78				
4-30	7-70	4-88				

Instant Yield Pumping Test Borehole N° NZ63 Village N° 7 2-8-84 (cont'd).

Time (mins)	Water Level. (metres)	Drawdown. (metres)	Yield (secs/20.e)	Tempo (°c)	Conductivity $\mu S$	pH
5	7-80	4-98	32-3	23.2	360	6-65
6	7-96	5-14				
7	8-05	5-23				
8	8-12	5-30				
9	8-19	5-37				
10	8-25	5-43	27-3	23.2	390	7-35
12	8-37	5-55				
14	8-43	5-61				
15	8-46	5-65	27-4	23.2	390	7-45
16	8-485	5-665				
18	8-54	5-72				
20	8-60	5-78	27.5	23.2	390	7-55
22	8-655	5-835				
24	8-72	5-90	27.4	23.2	390	7-55
26	8-76	5-94				
28	8-79	5-97				
30	8-84	6-02	27-3	23.2	390	7-55
35	8-94	6-12		23.2		
40	8-99	6-17	26-5	23.2	390	7-60
45	9-03	6-21	27-5	23.2		
50	9-08	6-26	27-3	23.2	400	7-60
55	9-11	6-29	27-5	23.2	390	7-55
60	9-16	6-34	27-1	23.2	400	7-60
70	9-24	6-42	27-5	23.2	390	7-35
80	9-29	6-47	27-5	23.2	410	7-35
90	9-38	6-56	27-5	23.2	380	7-35
100	9-46	6-64	27-0	23.2	400	7-35

Constant Yield Pumping Test. Borehole No. NZ6B Village No. 7 2-3-84 (cont'd)

Time (mins)	Water Level (metres)	Drawdown (metres)	Yield Secs/20L	Temp (°C)	Conductivity μs	pH
110	9-51	6-69	26-9	23-2	390	7-40
120	9-575	6-755	27-0	23-2	400	7-45
135	9-63	6-81	27-2	23-2	390	7-30
150	9-70	6-88	27-4	23-2	390	7-40
165	9-76	6-94	27-0	23-2	400	7-45
<del>170</del> 177½	9-78	6-96				
179	9-80	6-98				
180	9-81	6-99	27-3	23-2	400	7-45
HCO <sub>3</sub>	166 ppm					

Time - Drawdown Pumping Test Jacob Straight Line Analysis

Borehole No: NZ 60 Village No: Aganaewa Date: 2-8-84

Static Water Level: 2.82 meters below ground level.

Test Duration of hours - pump stopped after 3 hours.

$$K_6 = 2.30 \text{ m}^3/\text{sec}$$

$$A = 4\pi r^2 = 4\pi (1.3)^2 = 53.3 \text{ m}^2$$

$$\theta = 62.86 \text{ m}^3/\text{day} (0.73 \text{ l/sec})$$

$$A_3 = 2.25 \text{ meters. } A_5 = 1.3 \text{ meters.}$$

$$K_{61} = \frac{2.30 \times 62.86}{4\pi \times 2.25} = \frac{2.30 \times 62.86}{4\pi \times 1.3} = 2.25 \text{ m}^3/\text{day/m}$$

$$= 5.11 \text{ m}^3/\text{day/m}$$



Drawdown (meters)

$$A_5 = 2.25 \text{ meters.}$$

$$A_3 = 1.3 \text{ meters.}$$

$$y_{old} \\ (l/b sec)$$



Time t (hours)

1000

Recovery Test:

Borehole N° NZ 6B Village N° 7 Nyamazura 2-8-84.

Pump Cylinder Depth 31-82 metres.

Rest Water Level. 2-82 metres below ref point.

Ref. Point 0-15 metres above ground level.

Pump used - Dando operated single action reciprocating pump.

Maximum drawdown. 6-99 metres.

Yield 0.73 l/sec. 62.84 m<sup>3</sup>/day

Time since p stopped (mins.)	Water Level. (metres)	Residual Drawdown. (metres)	Recovery. (metres)	$t/t^4$
0-15	9-09	6-42	0-57	721
1-30	8-60	5-58	1-41	361
2-45	7-80	4-98	2-01	241
3-00	7-23	4-41	2-58	181
3-15	6-70	3-88	3-11	145
4-30	6-17	3-41	3-58	121
5-45	5-77	2-95	3-4-04	103.9
1	5-49	2-57	4-42	91
2-30	5-10	2-28	4-71	73
3	4-92	2-10	4-89	61
3-30	4-76	1-96	5-03	52.4
4	4-63	1-81	5-18	46
4-30	4-52	1-70	5-29	41
5	4-44	1-62	5-37	37
6	4-26	1-44	5-55	31
7	4-09	1-27	5-72	26.7
8	4-00	1-18	5-81	23.5
9	3-945	1-125	5-865	21
10	3-89	1-07	5-92	19

Recovery Test. Borehole N° NZ 6B		Village N° 7 Nyamazura 2-8-84. (cont'd)		
Time since pump stopped.	Water Level (metres)	Residual Drawdown (metres)	Recovery (metres)	%
12	3-815	0-995	5-995	16
14	3-76	0-96	6-03	13-9
16	3-72	0-90	6-09	12-3
18	3-69	0-83	6-16	11
20	3-73	0-91	6-08	10
21	3-745	0-925	6-065	9-6
22	3-725	0-905	6-085	9-2
23	3-695	0-875	6-115	8-8
24	3-67	0-85	6-14	8-5
26	3-62	0-80	6-19	7-9
28	3-585	0-765	6-225	7-4
30	3-55	0-73	6-26	7
35	3-48	0-66	6-33	6-1
40	3-43	0-61	6-33	5-5
45	3-38	0-56	6-43	5
50	3-33	0-51	6-48	4-6
55	3-30	0-48	6-51	4-27
60	3-27	0-45	6-54	4

Bail Test.

Borehole N° NZ6B Village N° 7 Nyamazura 31-7-84

Rest water level: 2-80 metres below ground level.

Capacity of bailed 22 litres.

N° of Baits 39.

Time taken: 18 mins 17 secs.

Drawdown: 11-26 metres (5-46 metres).

Amount of water bailed: 858 litres

Average yield: 47.0 l/min (0.78 l/sec).

Recovery

ime after  
draining stopped

(mins)

Water Level

(metres)

Residual  
Drawdown

(metres)

Recovery

(metres)

%

—

5

8-15

5-35

0-11

4-65

10

6-00

3-20

2-26

2-825

15

5-00

2-20

3-26

2-22

20

4-28

1-48

3-98

1-9

25

3-84

1-04

4-42

1-73

30

3-60

0-80

4-66

1-61

35

3-36

0-56

4-90

1-52

40

3-25

0-45

5-01

1-46

45

3-16

0-36

5-10

1-41

50

3-08

0-28

5-18

1-37

55

3-04

0-24

5-22

1-33

60

3-00

0-20

5-26

1-30

Feeding to 1 Tidal Advance

$$B_{\text{total}} = 1.4 \times 12.63 = 17.08 \text{ m}^3/\text{day}$$

$$2.8 \times 14 = 39.2 \text{ m}^3/\text{day}$$

$$\text{Static Water Level} = 9.92 \text{ metres below sea level}$$

Maximum drawdown 6.19 metres

$$K_6 = 2.32 \times 0$$

$$4 \pi \Delta S^* = 4.743 \text{ m}^3/\text{day}$$

$$\Delta S^* = 9.9 \text{ metres}$$

$$Q_{\text{all}} = 6.769 \text{ m}^3/\text{day}$$

$$K_6 = 2.32 \times 6.769 = 16.14 \text{ m}^3/\text{day}$$

$$B_{\text{all}} = 4 \pi \times 9.9 = 125.4 \text{ m}^3/\text{day}$$

$$\Delta S^*_{\text{all}} = 9.9 \text{ metres}$$

$$Q = 62.86 \text{ m}^3/\text{day}$$

$$4 \pi \Delta S^* = 14 \text{ m}^3/\text{day}$$

$$K_6 = 2.32 \times 14 = 32.88 \text{ m}^3/\text{day}$$

$$B = 4 \pi \times 14 = 175.84 \text{ m}^3/\text{day}$$

$$\Delta S^* = 14 \text{ metres}$$

$$Q = 2.3 \times 14 = 32.2 \text{ m}^3/\text{day}$$

$$B = 4 \pi \times 32.2 = 401.92 \text{ m}^3/\text{day}$$

$$\Delta S^* = 32.2 \text{ metres}$$

$$Q = 2.3 \times 32.2 = 74.04 \text{ m}^3/\text{day}$$

$$B = 4 \pi \times 74.04 = 914.4 \text{ m}^3/\text{day}$$

$$\Delta S^* = 74.04 \text{ metres}$$

$$Q = 2.3 \times 74.04 = 170.29 \text{ m}^3/\text{day}$$

$$B = 4 \pi \times 170.29 = 1134.56 \text{ m}^3/\text{day}$$

$$\Delta S^* = 170.29 \text{ metres}$$

$$Q = 2.3 \times 170.29 = 417.67 \text{ m}^3/\text{day}$$

$$B = 4 \pi \times 417.67 = 1313.32 \text{ m}^3/\text{day}$$

$$\Delta S^* = 417.67 \text{ metres}$$

Constant Yield Pumping Test.

Borehole No. NZ 7A, Village No. 6. NYAMAZURA.

Date 20-7-84.

Time test started 10-30 Am.

Pump Cylinder Depth ~ 43 metres.

Ref point 10 cms agl.

Pump used - Dando operated single action reciprocating pump.

Time SWL (metres)

10-19 27-77 Water producing zones.

10-26 27-77 34 - 38 metres.

10-29 27-77 40 metres.

Hole diameter 204 mm (8")

Time (mins)	Water Level (metres)	Drawdown (metres)	Yield. (Secs/20L.)	Temperature (°C)	Conductivity μs	pH
0-15	27-90	0.13				
0-30	27-95	0.21				
0-45	27-09	0.32				
1-00	28- <del>22</del> 22.	0.45				
1-15	28-39	0.62				
1-30	28-54	0.77				
1-45	28-67	0.90				
2-00	28-82	1.05				
2-30	29-10	1.33				
3-00	29-57	1.74				
3-30	29-92	2.15				
4-00	30-26	2.49				
4-30	30-69	2.82				
5-00	31-13	3.36				
	32-03	4.26				

Constant Yield Test - Berechile No NZ 7 - Village No 6 - NYANAZURA - 20-7-84 (Cont'd)

Time (mins.)	Water level (metres)	Drawdown (metres)	Yield (Secs/20g)	Temperature (°C)	Conductivity μs	pH
7	32-81	5.04				
8	33-31	5.54				
9	33-68	5.91				
10	34-06	6.29				
11	34-55	6.78				
12	35-02	7.25				
14	36-00	8.23				
16	36-82	9.05				
18	38-24	10.47				
20	41-05	13.28				
22	43-43	15.66				
24	43-43	15.66				

Pump stated sucking air @ 43-43 after 22 minutes of pumping. Yield dropped dramatically... most of water produced during test was taken directly from storage within the 8" well.

Recovery Test.

Borehole N° NZ 7 Village N° 6. NYAMAZURA.

Date 20-7-84.

Time test started - 10-30 hrs.

Time pump stopped 11-30 hrs.

Pump cylinder at ~ 43 metres below gl.

Rest water level. 27.77 metres.

Ref point 10 cms agl.

Maximum drawdown experienced. 15-66 metres.

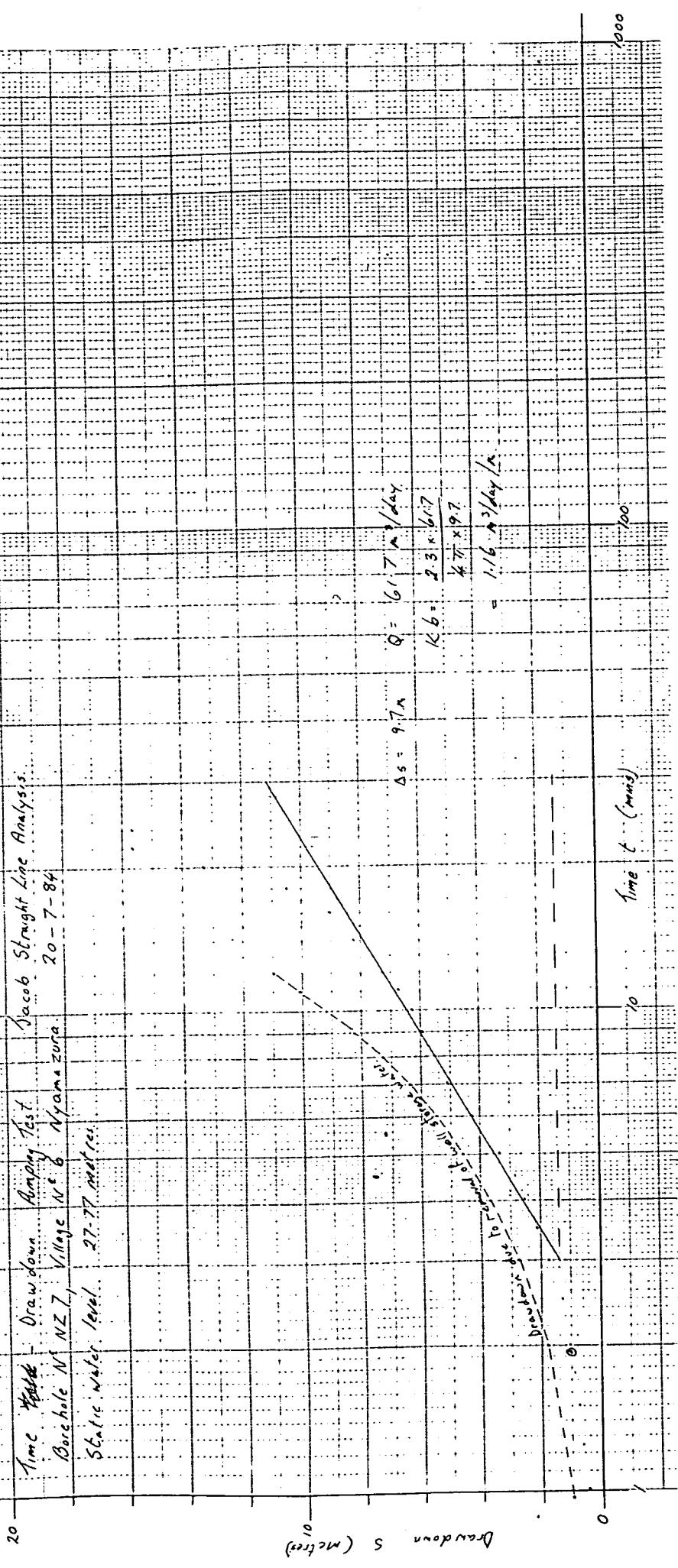
Yield.

Time since p stopped. " (mins)	Water level. (metres)	Residual Drawdown. (metres)	Recovery. (metres)	t/t.
-15				
-30				
-45				
1-00.	44-45	16-68?	-	
1-15				
1-30	43-25	15-48	0-18	41
1-45	43-22	15-45	0-21	35.3
2-00.	43-19	15-42	0-24	31
2-30	43-12	15-35	0-31	25
3-00	43-05	15-28	0-38	21
3-30	42-99	15-22	0-44	18.14
4-00.	42-92	15-15	0-51	16
4-30	42-86	15-09	0-57	14.3
5	42-80	15-03	0-63	13
6	42-68	14-91	0-75	11
7	42-56	14-79	0-87	9.57
8	42-46	14-67	0-99	8.5

## Recovery Test... Borehole No NZ 7 Village N° 6 NYAMAZURA 20-7-84 (cont'd)

Time since pump stopped. <i>t</i> ° (mins)	Water level (metres)	Residual Drawdown. (metres)	Recovery (metres)	<i>t</i> / <i>t</i> "
9	42-32	14-55	1-11	7.7
10	42-20	14-43	1-23	7
12	41-98	14-21	1-45	6
14	41-78	14-01	1-55	5.29
16	41-58	13-81	1-85	4.75
18	41-37	13-60	2-06	4.33
20	41-13	13-36	2-30	4
22	40-95	13-18	2-48	3.73
24	40-75	12-98	2-68	3
26	40-52	12-75	2-81	3.31
28	40-30	12-53	3-13	3-14
30	40-09	12-32	3-34	3
35	39-52	11-75	3-91	2.7
40	38-92	11-15	4-51	2.5
45	38-39	10-62	5-04	2.3
50	37-84	10-07	5-59	2.2
55	37-50	9-73	5-93	2.1
60	37-30	9-53	6-13	2
65.	37-09.	9-32	6-34.	1.92.

Time  $t$  (min) Draw down  $s$  (metre) Jacob Straight Line Analysis  
 Bochale N NZ 7, Village No 6 Yamazora. 20 - 7 - 84  
 Static water level. 27.77 metre



*Recovery Test Thesis Analysis*

Borehole N° N27 Village N° 6 Nyamazura 20-7-84  
 Static water rest level 27.77 m  
 Max draw down. 15.66 m.  
 Yield 4.8 m<sup>3</sup>/day

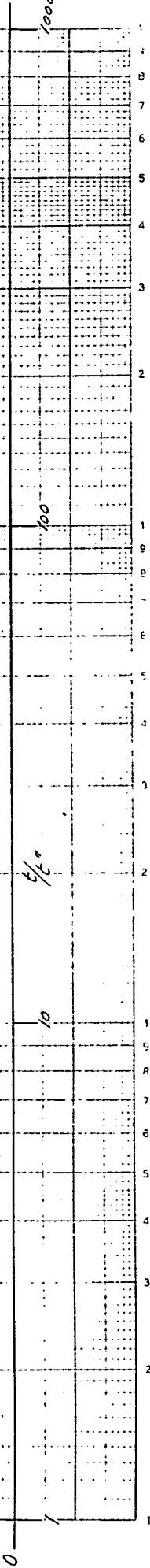
$$\Delta S'' = 18.2 \text{ m.} \quad Q = 0.066 \text{ l/sec}$$

$$= 4.8 \text{ m}^3/\text{day}$$

$$K_6 = \frac{2.30}{4\pi} \times \frac{Q}{\Delta S''}$$

$$= \frac{2.3 \times 4.8}{4\pi \times 18.2}$$

$$= 0.048 \text{ m}^3/\text{day/meter}$$



Bail Test.

Borehole N° NZ7B. Village N° 6. Nyamazura. 10-8-84.

Rest water level. 24.78 metres (bgl)

Capacity of baster 22 litres.

N° of basls 49.

Time taken 30 minutes.

Drawdown: 1.52 metres.

Amount of water bailed. 1.078 m³.

Yield: 0.60 l/sec. 51.57 m³/day

Specific capacity 33.92 m³/day / metre of drawdown.  
time after  
bailing stopped. Water Level. Residual  
Drawdown. Recovery.

(mins)	(metres)	(metres)	(metres)	t/km
0 - 30	26.20	1.42	0 - 10	61
1 - 00	26.05	1.27	0 - 25	31
2	25.86	1.08	0 - 44	16
4	25.61	0.83	0 - 69	8.5
6	25.41	0.63	0 - 89	6
8	25.28	0.50	1 - 02	4.75
10	25.21	0.43	1 - 09	4.
15	25.13	0.33	1 - 19	3.
20	25.06	0.28	1 - 24	2.5
25	25.02	0.24	1 - 28	2.2
30	25.00	0.22	1 - 30	2
35	24.96	0.18	1 - 34	<del>1.86</del>
40	24.95	0.17	1 - 35	<del>1.75</del>
45	24.94	0.16	1 - 36	<del>1.67</del>
50	24.93	0.15	1 - 37	1 - 60
55	24.92	0.14	1 - 38	1 - 55
60	24.91	0.13	1 - 39.	1 - 5.

Bathes - Recovery Data Thesis analysis

Batch No. 3	1/19/64	6 days, 24 hrs.	10-8-84
Test meter rate	24.78	metres	
Maximum draw down	1.52	metres	
Flow	51.67	m <sup>3</sup> /day	
Duration of test	30	mins	

$$K_b = 2.30 \cdot 8$$

$$47.4 \cdot 5$$

$$2.30 \cdot 5.57$$

$$17 \cdot 0.92$$

$$10.26 \text{ m}^3/\text{day/meter}$$

10

Residual Drawdown, S, metres

5

0

60

1000

45' = 0.92 metres.

# Constant Yield Pumping Test.

Village N° 3 Iiyamazura.

Bore hole N° NZ 8.

Date: 10-7-84.

Cylinder at 33 metres.  $Q = 57.6 \text{ m}^3/\text{day}$

Yield measured as the number of seconds to fill a 20 litre capacity bucket.

Time Water level. Date.

11-29 am. 12-11 m. 5-7-84

13-25 12-10 m. 10-7-84

13-28 12-09 m. 10-7-84

13-29 12-095 m. 10-7-84

Pumping Test Started at. 13-30.

Time after pump started (min.)	Water level (metres)	Drawdown (metres)	yield (Secs/20 litres)	Cond. $\mu\text{s}$ .	Temp °C.	pH.
0-15	13-13	1.03	31.6			
0-30	13-47	1.37				
0-45	13-76	1.66				
1-00	13-92	1.82				
1-15	14-02	1.92				
1-30	14-17	2.07				
1-45	14-22	2.12				
2-00	14-33	2.23				
2-30	14-40	2.30				
3-00	14-50	2.40				
3-30	14-59	2.49				
4-00	14-69	2.59				
4-30	14-75	2.65				
5-00	14-80	2.70	29.2	270	23.4	7.35
6	14-92	2.82				
7	14-99	2.89				
8	15-05	2.95				

Time after pump started (min.)	Yield, Pumping Test Water level (metres.)	Drawdown (metres)	Borehole No. N28 (cont'd.)			
			Yield (Secs/20 ltrs)	Cond. $\mu\text{s}$	Temp $^{\circ}\text{C}$	pH
9	15-11	3-01				
10	15-16	3-06	28.6			
12	15-24	3-14				
14	15-31	3-21	29.5			
16	15-37	3-27				
18	15-43	3-33				
20	15-47	3-37	29.7	265	23.4	7.75
22	15-51	3-41				
24	15-54	3-44	29.6			
26	15-58	3-48				
28	15-61	3-51				
30	15-63	3-53	29.4	265	23.2	7.85
35	15-66	3-56				
40	15-69	3-59				
45	15-77	3-67	29.2	265	23.4	7.70
50	15-82	3-72	<del>30.0</del>	<del>265</del>	<del>23.2</del>	<del>7.75</del>
55	15-84	3-74				
60	15-87	3-77	30.0	265	23.4	7.85
70	15-91	3-81	<del>30.0</del> 29.8	260	23.4	7.90
80	15-98	3-88				
90	16-04	3-94	30.1	255	23.4	8.00
100	16-07	3-97	29.9	255	23.4	8.05
120	16-12	4-02	30.0	255	23.2	8.05
150	16-16	4-06	30.8	255	23.2	8.00
180	16-19	4-09	30.6	255	23.2	7.85
		$\text{HCO}_3^- - 149 \text{ pp.m}$				

Recovery Test.

Village N° 3. Inyanazura. Borehole N° N2 8.

Date 10-7-84.

Cylinder at 33 metres below ground level.

Rest water level. 12-10 metres.

Maximum drawdown of 4-09 metres to 16-19 metres.

Yield

0.67 l/sec.

57.6 m<sup>3</sup>/day.

Time since pump stopped (mins)	Water level (metres)	Residual Drawdown (metres)	Recovery (metres)	t/t <sub>0</sub>	Time since pump stopped (mins)	Water level (metres)	Residual Drawdown (metres)	Recovery (metres)	t/t <sub>0</sub>
0 - 15	15.38	3.28	0.81	721	14	12.95	0.85	3.24	13.9
0 - 30	14.53	2.73	1.36	361	16	12.915	0.815	3.275	12.25
0 - 45	14.45	2.35	1.74	241	18	12.875	0.775	3.325	11
1 - 00	14.27	2.17	1.92	181	20	12.84	0.74	3.35	10
1 - 15	14.08	1.98	2.11	145	22	12.81	0.71	3.38	9.2
1 - 30	13.94	1.84	2.25	121	24	12.785	0.685	3.405	8.5
1 - 45	13.84	1.74	2.35	103.9	26	12.76	0.66	3.43	7.9
2 - 00	13.75	1.65	2.44	91	28	12.735	0.635	3.455	7.4
2 - 30	13.62	1.52	2.87	73	30	12.715	0.615	3.475	7
3 - 00	13.53	1.43	2.66	61	35	12.665	0.565	3.535	6.1
3 - 30	13.46	1.36	2.73	52.4	40	12.625	0.525	3.575	5.5
4 - 00	13.40	1.30	2.79	46	45	12.59	0.49	3.60	5
4 - 30	13.36	1.26	2.83	41	50	12.56	0.46	3.63	4.6
5 - 00	13.32	1.22	2.87	37	55	12.53	0.43	3.66	4.3
6	13.24	1.14	2.95	31	60	12.51	0.41	3.69	4
7	13.19	1.04	3.00	26.7					
8	13.15	1.045	3.045	23.5					
9	13.105	1.005	3.035	21					
10	13.07	0.97	3.12	19					
12	13.005	0.905	3.135	16					

Recovery test Borehole N° 8 Village N° 3 Inyamazura.

Date 10-7-94 Thesis Recovery Method.

Time pump started 13-30

Time pump stopped 16-30

Maximum drawdown 4-09 metres

Surface 12-10 metres.

$Q = 57.6 \text{ m}^3/\text{day}$

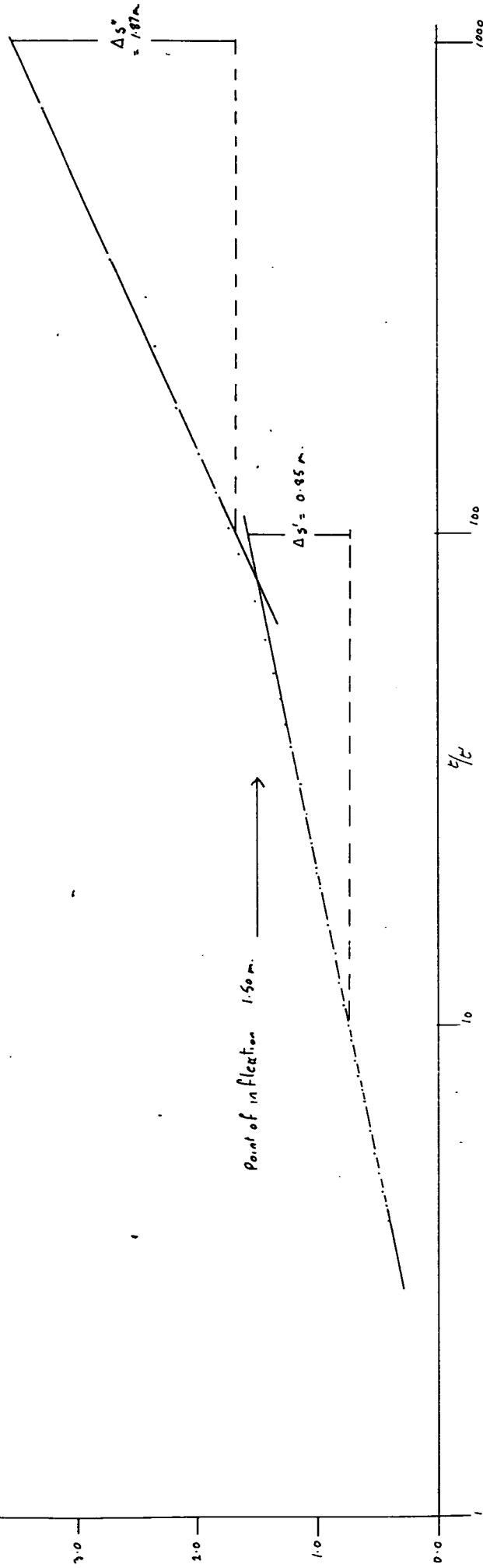
$$K_6 = \frac{230 \times Q}{4\pi A_s}$$

for  $A_s' = 0.85 \text{ m}^2$

$$K_6' = \frac{230 \times 57.6}{4\pi \times 0.85} \text{ m}$$

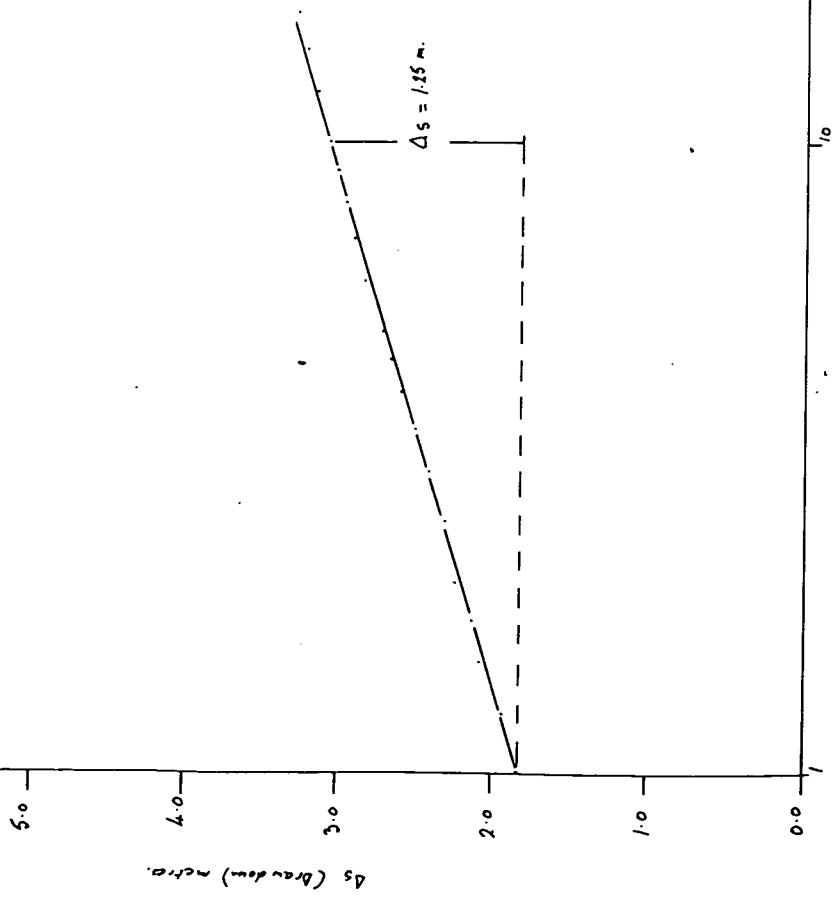
$$= 12.40 \text{ m}^3/\text{day}$$

Residual drawdown  $\Delta s'$  (metres)



Constant Yield Pumping Test. Borehole N° NZ8 Village N° 3 Inyamazura.  
 Date 10-7-84 Jacob Straight Line Analysis.  
 Static Water Level 12.10 metres.  
 $Q = 57.6 \text{ m}^3/\text{day}$ .

$$\begin{aligned}
 K_6 &= \frac{2.30 Q}{4 \pi \Delta s} \\
 &= \frac{2.30 \times 57.6}{4 \pi \times 1.25} \\
 &= 8.43
 \end{aligned}$$



Constant Yield Pumping Test.

Borehole N° NZ 9. Village N° 1 A Inyamazura 18-7-84

Time test started 13-00 hrs.

Pump cylinder 28.50 metres below ground level

Ref point ~ 30 cms a.g.l.

Pump used - Dando operated single action reciprocating pump.

Time	Swirl (m)	Hole depth 30-80 metres.
12-42	16-09	
12-44	16-10	
12-49	16-09	
12-52	16-09	
12-55	16-09	
12-58	16-09	

Time (mins)	Water level. (metres),	Drawdown. (metres)	Yield. (Gms/20 s)	Temp. (°C)	Conductivity μS	pH
-15	16-86	0-77				
-30	17-15	1-06				
-45	17-31	1-22				
1	17-41	1-32	29.7	24.2	340	7.8
1-15	17-45	1-36				
1-30	17-50	1-41				
1-45	17-53	1-46				
2	17-63	1-54				
2-30	17-64	1-55				
3	17-70	1-61				
3-30	17-74	1-65				
4	17-78	1-69				
4-30	17-86	1-75				
5	17-94	1-77	29.0	24.2	340	8.00
6	17-97	1-85				

Constant Yield Pumping Test. NZ 9 18-7-84 (Contd)

Time (mins)	Water level (metres)	Drawdown (metres)	Yield (secs/20l)	Temp °C.	Conductivity μs.	pH.
7	17.97	1.88				
8	18.05	1.915				
9	18.045	1.955				
10	18.065	1.975	28.5	24.0	335	8.05
12	18.115	2.025				
14	18.16	2.07	28.5	24.0	335	8.05
16	18.187	2.097				
18	18.205	2.115				
20	18.23	2.14	28.5	24.0	335	8.05
22	18.25	2.16				
24	18.27	2.17	28.6	24.0	330	8.05
26	18.28	2.19				
28	18.29	2.20				
30	18.302	2.21	28.6	24.0	330	8.05
35	18.345	2.255	28.6	24.0	330	8.05
40	18.37	2.28	28.6	24.0	335	8.10
45	18.39	2.30	28.5	24.0	325	8.05
50	18.405	2.315	28.6	24.2	325	8.05
55	18.42	2.33	28.6	24.0	330	8.05
60	18.432	2.342	28.6	24.0	325	8.05
65	18.45	2.36				
70	18.45	2.36	28.5	24.0	325	8.05
75	18.46	2.37				
80	18.48	2.39	28.6	24.0	320	8.00
85	18.465	2.375				
90	18.46	2.37	28.6	23.8	320	7.70

Constant Yield Pumping Test. N.Z.G. 18-7-84 (cont'd)

$\text{HCO}_3^- + 200 \text{ ppm } @ 160 \text{ m/s.}$

Recovery Test.

Borehole N° NZ 9 Village N° 1A Inyanazua. 18-7-84.

Pump cylinder at 28°50' m. below ground level.

Rest water level. 16-09 metres. (below ref point)

Ref point: 30 cms. above ground level.

Maximum drawdown. 2.48 metres.

Time since pump stopped. (Mins)	Yield 0.699 l/sec	Water Level (metres)	Residual Drawdown. (metres)	Recovery. (metres)	60.4 m³/day lit.
0 - 15	18-18	2-09	0-39	721	721
0 - 30	17-95	1-86	0-62	361	361
0 - 45	17-72	1-63	0-85	241	241
	17-49	1-40	1-08	181	181
0 - 15	17-43	1-34	1-14	145	145
0 - 30	17-32	1-23	1-25	121	121
0 - 45	17-16	1-07	1-41	103.9	103.9
1	17-05	0-96	1-52	91	91
2 - 30	16-89	0-80	1-68	73	73
3	16-76	0-67	1-81	61	61
3 - 30	16-64	0-55	1-93	52.4	52.4
4	16-57	0-48	2-00	46	46
4 - 30	16-52	0-43	2-05	41	41
5	16-48	0-39	2-09	37	37
6	16-42	0-33	2-16	31	31
7	16-33	0-293	2-287	26.7	26.7
8	16-36	0-27	2-21	23.5	23.5
9	16-34	0-25	2-23	21	21
10	16-328	0-238	2-242	19	19
12	16-305	0-215	2-265	16	16

Recovery time since pump stopped: (mins)	Test: NZ 9	Village N° 1A	18-7-84	(cont'd.)
	Water level. (metres)	Residual Drawdown. (metres)	Recovery. (metres)	$t/t_1$
14	16-29	0-20	2-28	<del>13.9</del> 13.9
16	16-278	0-188	2-292	12.25
18	16-27	0-18	2-30	11
20	16-26	0-17	2-31	10
22	16-25	0-16	2-32	9.2
24	16-245	0-155	2-325	8.5
26	16-239	0-149	2-331	7.9
28	16-23	0-14	2-34	7.4
30	16-225	0-135	2-345	7
35	16-214	0-124	2-356	6.1
40	16-204	0-114	2-366	5.5
45	16-20	0-11	2-37	5
50	16-19	0-10	2-38	4.6
55	16-184	0-094	2-386	4.27
60	16-18	0-09	2-39	4.

Recovery test - Thesis Analysis.  
 Borehole N° N29 Village 1B. Inyamazuro. 18/7/84.  
 static water level 16.09 metres  
 $Q = 60.4 \text{ m}^3/\text{day}$ .  
 Maximum draw down = 2.68 metres.

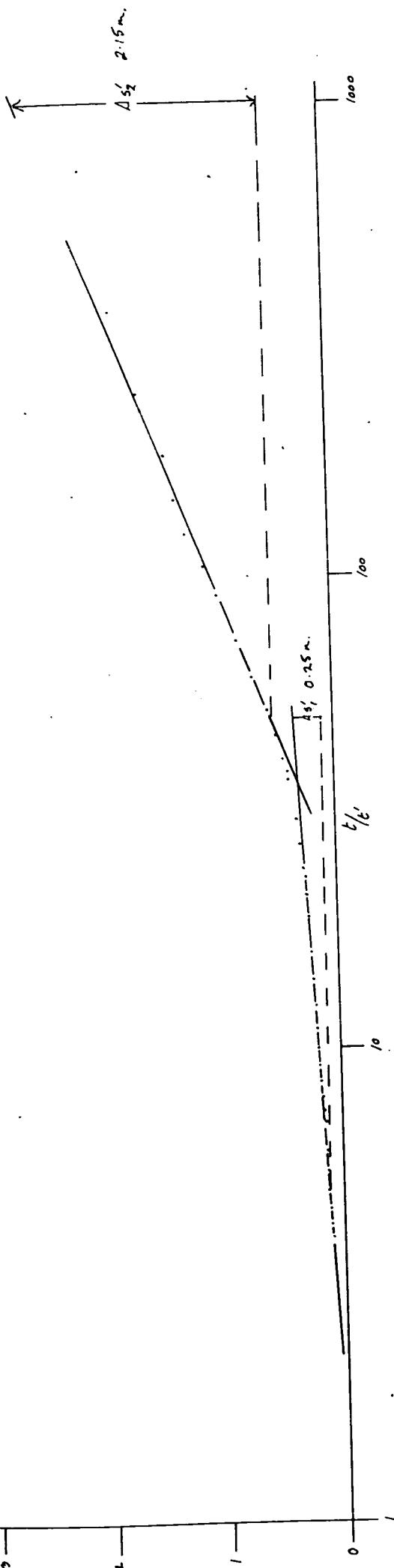
$$Kb = \frac{2.30 + Q}{4\pi \Delta s'}$$

$$\text{for } \Delta s'_1 = 0.25 \text{ metres.}$$

$$\begin{aligned}
 Kb'_1 &= \frac{2.3 + 60.4}{4\pi \cdot 0.25} \\
 &= 44.23 \text{ m}^3/\text{day/m}.
 \end{aligned}$$

$$\text{for } \Delta s'_2 = 2.15 \text{ metres.}$$

$$\begin{aligned}
 Kb'_2 &= \frac{2.3 + 60.4}{4\pi \cdot 2.15} \\
 &= 5.14 \text{ m}^3/\text{day/m}.
 \end{aligned}$$



Yield - Drawdown Test.  
Jacob Straight Line Analysis.

Borehole N° N29 Village N° 1A Layamazure 19-7-84

Static Water Level ~ 16.09 Aetres.

$Q = 60.4 \text{ m}^3/\text{day}$ .

$$Kb = \frac{2.30}{4 \pi} \frac{Q}{\Delta s_1}$$

$$\text{for } \Delta s_1 = 0.67 \text{ m.}$$

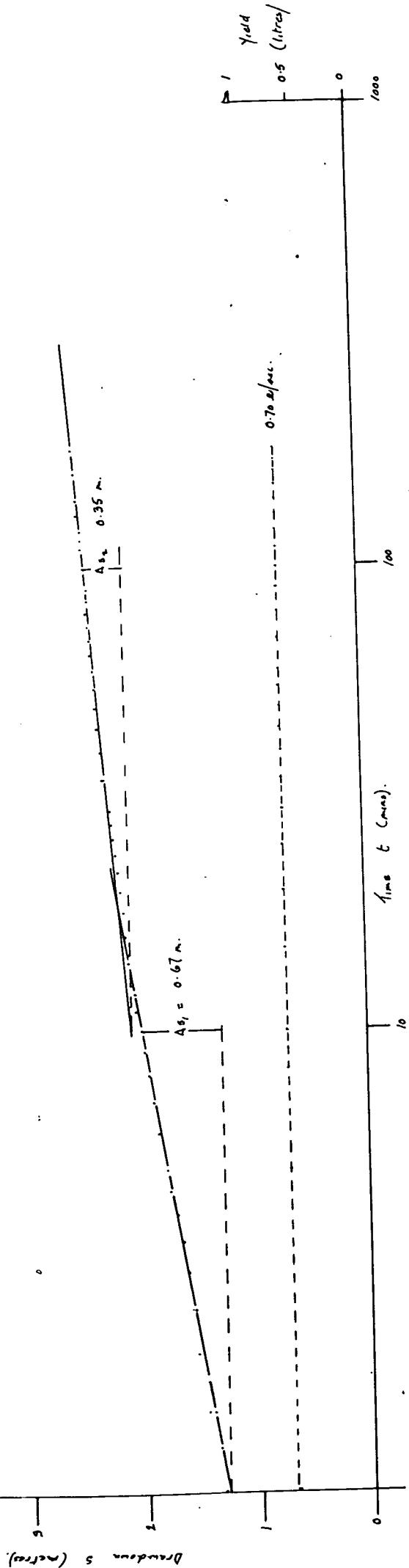
$$Kb_1 = \frac{2.3 \times 60.4}{4 \pi \times 0.67}$$

$$= 16.50 \text{ m}^3/\text{day/m.}$$

for  $\Delta s_2 = 0.35 \text{ m.}$

$$Kb_2 = \frac{2.3 \times 60.4}{4 \pi \times 0.35}$$

$$= 31.59 \text{ m}^3/\text{day/m.}$$



Constant Yield Pumping Test.

Village N° 4 INYAMAZURA.

Borehole N° NZ 10

Date 11-7-84

Cylinder at 25.5 metres below ground level.

Yield measured in number of seconds to fill a 20 litre capacity bucket.

Time Water level.  $Q = 49.25 \text{ m}^3/\text{day}$

14-11 6.88 metres

14-14 6.88 metres

Pumping test started at 14-15.

Time after pump started (mins)	Water level (metres)	Drawdown (metres)	Yield (secs/200)	Cond $\mu\text{s}$	Temp °C	pH
0-15	7-74	0-86				
0-30	8-07	1-19				
0-45	8-31	1-43				
1-00	8-39	1-51				
1-15	8-50	1-62				
1-30	8-54	1-66				
1-45	8-60	1-72				
2-00	8-65	1-77				
2-30	8-73	1-85	33.9			
3-00	8-76	1-88				
3-30	8-79	1-91				
4-00	8-825	1-945				
4-30	8-86	1-98				
5	8-88	2-00	33.6			
6	8-905	2-025				
7	8-93	2-05				
8	8-96	2-08				
9	8-97	2-09				

Constant Yield Test. Time after pump started (mins).	Water level (Metres)	NZ 10' Cont'd.	Drawdown (Metres)	Yield (Secs/20.0)	Cond μS	Temp °C	pH
10	8-99	2-11		33.2	410	22.6	7.45
12	9-025	2-145		33.2	410	22.6	7.45
14	9-05	2-17		34.0	"	"	"
16	9-07	2-19					
18	9-085	2-205					
20	9-09	2-21		33.9	390	22.6	7.95
25	9-125	2-245		34.1			
30	9-15	2-27		34.2	400	22.6	8.05
35	9-165	2-285		34.4			
40	9-19	2-31		34.4	400	22.6	8.10
45	9-21	2-33		34.2			
50	9-20	2-32		34.2	400	22.4	8.20
55	9-23	2-35		34.2			
60	9-23	2-35		34.2	400	22.4	8.20
70	9-22	2-34		34.6	395	22.4	8.25
80	9-245	2-365		34.4	395	22.2	8.45
90	9-265	2-385		34.4	395	22.2	8.50
100	9-275	2-395		34.4	395	22.2	8.50
110	9-29	2-41		34.5	390	22.0	8.50
120	9-30	2-42		34.5	390	22.0	8.45
150	9-35	2-47		34.6	390	22.2	8.35
180	9-375	2-495		34.5	390	22.2	8.40
				HCO <sub>3</sub> <sup>-</sup> = 217 ppm			

Recovery Test.

Village N° 4. Inyamazura.

Borehole N° NZ 10.

Date: 11-7-84.

Cylinder at 25.5 metres below ground level.

Rest water level 6.88 metres.

Maximum drawdown 2.495 metres to 9.375 metres.

Yield 0.57 l/sec. 49.25 m<sup>3</sup>/day.

Time since  
pump stopped

(mins).

Water level. (Metres)	Residual Drawdown (Metres)	Recovery (Metres)	%
8-60	1-72	0.775	721
8-10	1-22	1.275	361
7-83	0-95	1.545	241
7-66	0-78	1.715	181
7-56	0-68	1.815	145
7-49	0-61	1.885	121
7-44	0-56	1.935	103.4
7-405	0-525	1.970	91
7-36	0-48	2.015	73
7-33	0-45	2.045	61
7-31	0-43	2.065	52.4
7-29	0-41	2.085	46
7-275	0-395	2.100	41
7-26	0-38	2.115	37
7-24	0-36	2.135	31
7-22	0-34	2.155	26.7
7-21	0-33	2.165	23.5
7-195	0-315	2.180	21
7-19	0-31	2.185	19
7-17	0-29	2.205	16

Recovery Test Time since pump stopped. (mins.)	Water level. (metres).	Residual Drawdown (metres).	Recovery.	<del>Cont'd</del> E/I.
14	7.156	0-276	2.219	<del>267</del> 13.9
16	7.143	0-263	2.232	<del>255</del> 12.25
18	7.135	0-255	2.240	<del>24</del> 11
20	7.127	0-247	2.248	<del>22</del> 10
25	7.11	0-23	2.265	<del>19</del> 9.2
30	7.10	0-22	2.275	<del>18</del> 7
35	7.09	0-21	2.285	<del>15</del> 6.1
40	7.08	0-20	2.295	<del>10</del> 5.5
45	7.07	0-19	2.305	<del>9</del> 5
50	7.06	0-18	2.315	<del>8</del> 4.6
55	7.06	0-18	2.315	<del>7</del> 4.3
60	7.055	0-175	2.320	<del>7</del> 4.

Constant Yield Pumping Test      Borehole N° NZ 10      Village N° 4 Inyanazura.  
 Date 11 - 7 - 84      Jacob Straight Line Analysis.  
 Static Water Level      6.88 metres.  
 $Q = 49.25 \text{ m}^3/\text{day}$ .

$$Kb = \frac{2.30}{4 \pi} \frac{Q}{\Delta s}$$

$$\begin{aligned}
 \Delta s' &= 0.65 \text{ m.} \\
 Kb' &= \frac{2.30 \times 49.25}{4 \pi \times 0.65} \\
 &= 13.9 \text{ m}^3/\text{day/m.}
 \end{aligned}$$

$$\begin{aligned}
 \Delta s'' &= 0.425 \text{ m.} \\
 Kb'' &= \frac{2.30 \times 49.25}{4 \pi \times 0.425} \\
 &= 21.2 \text{ m}^3/\text{day/m.}
 \end{aligned}$$

5.0

4.0

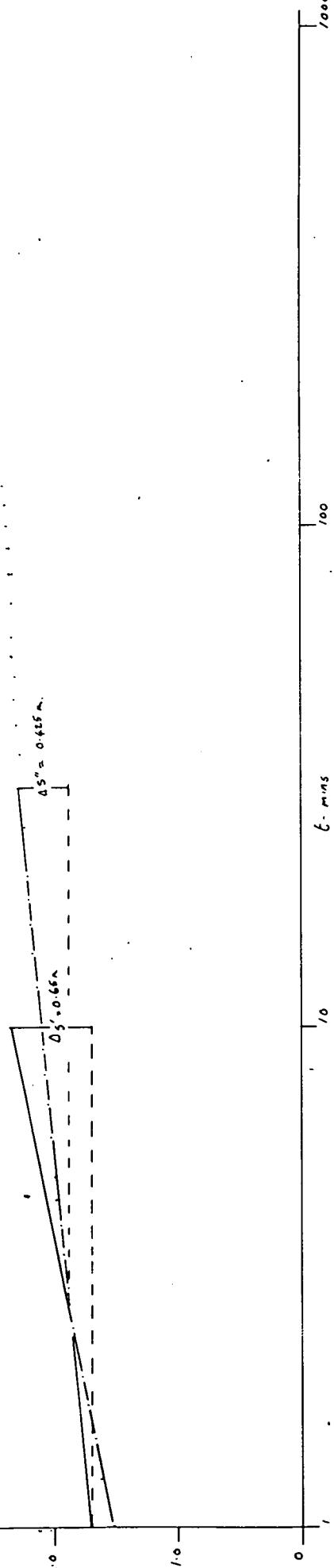
3.0

2.0

1.0

0

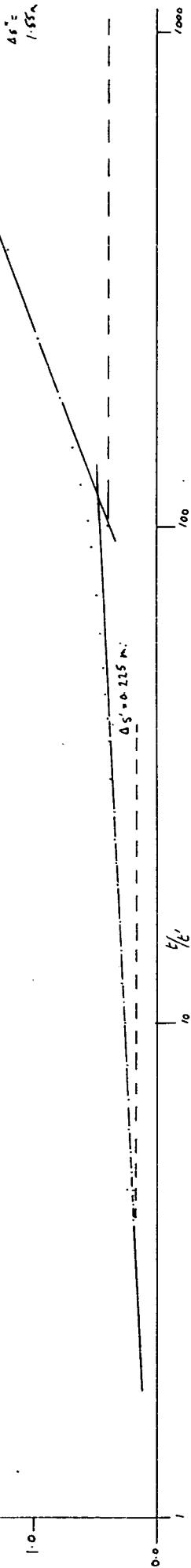
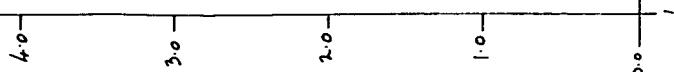
Drawdown Δs in metres.



Recovery Test      Borehole N° NZ 10      Village N° 4      Tayamazura.  
 Date 11-7-84      Then's Recovery Method.  
 Time pump started 16-15  
 Time pump stopped 17-15  
 Maximum Drawdown 2.495 metres.  
 Static Water Level 6.88 metres.  
 $Q = 4.925 \text{ m}^3/\text{day}$ .  
 $\Delta s' = 0.225 \text{ m}$ .

$$\begin{aligned}
 Kb' &= \frac{2.30 \times q}{4 \pi \Delta s'} \\
 &= \frac{2.30 \times 4.925}{4 \pi \times 0.225} \\
 &= 40.07 \text{ m}^3/\text{day}/\text{m}
 \end{aligned}$$

Residual Drawdown at 24 hrs



Constant Yield Pumping Test.

Borehole N° NZ 11 A Village N° 2 IN YAMAZURA.

Date 16-7-34

Time Test Started 15-00 hrs.

Pump Cylinder 24.62 metres below ground level.

Dipper would not pass beyond 23.72 metres - Top of the cylinder.

Ref point 0.50 cms agl.

Pump used - Dando operated single action reciprocating pump.

Time SWL (m).

14-18 4-17

14-43 4-17

14-49 4-17

Time (mins.)	Water level. (metres)	Drawdown (metres.)	Yield. (secs/20 ltrs)	Temp °C.	Conductivity μS	pH
0 - 15	6-00	1-83	24	23.8	340	7.7
0 - 30	6-93	2-76				
0 - 45	7- <del>5</del> 62	3-45				
1	8-33	4-16				
1-15	8-97	4-80				
1-30	9-50	5-33				
1-45	9-96	5-89				
2	10-24	6-07				
2-30	11-04	6-87				
3	11-82	7-65				
3-30	12-37	8-20				
4	12-97	8-80				
4-30	13-32	9-15				
5	13-66	9-45	29	23.0		

## Constant Yield Test - NZ II 16/7/84 cont'd.

Time (mins.)	Water level (metres)	Drawdown (metres)	Yield (Secs/20 litres)	Temp (°C)	Conductivity (μs)	pH
5-30						
6	14-49	10-32				
6-30						
7	15-31	11-14				
8	16-10	11-93				
9	17-08	13-91				
10	18-50	14-33	34	22.8	330	7.9
12	18-64	14-47				
13	19-21	15-04				
14	20-02	15-85				
15	20-83	16-66	35	22.6	300	8.05
16						
18	21-05	16-88				
20	21-48	17-31	36	22.6	300	8.4
22	22-77	18-60				
24	23-35	19-16				
25	23-72	19-55	35.4	22.6	310	8.4
30			34.8			
35			55			
40			59.8	22.6	325	8.2
45			62.5			
50			62.6	22.4	325	8.67
55			62.6	22.6	330	8.70
60			62.6	22.6	330	8.70
65			64.7	22.6	330	8.75
75			65.5	22.6	343	8.85
80			65.8	22.6	330	8.95
85			68.0	22.6	330	8.95
95			68.6	22.6	330	8.94

Constant Yield Test N 211 16/7/84 (cont'd)						
	Water level (metres)	Drawdown (metres)	Yield (Saci/20litres)	Temp (°c)	Conductivity (μs)	pH
			71	22.6	340	8.98
			71.6	22.6	330	8.85
			73.0	22.6	330	8.85
			74.5	22.6	330	8.85

Recovery Test.

Borehole N°. NZ II Village N° 2 Inyamazura.

Date. 16-7-84.

Pump cylinder at 24.62 metres below ground level.

Rest Water level 4.17 metres (below ref. point).

Ref. point 50 cm. above ground level.

Maximum drawdown. — about 20-23 metres.

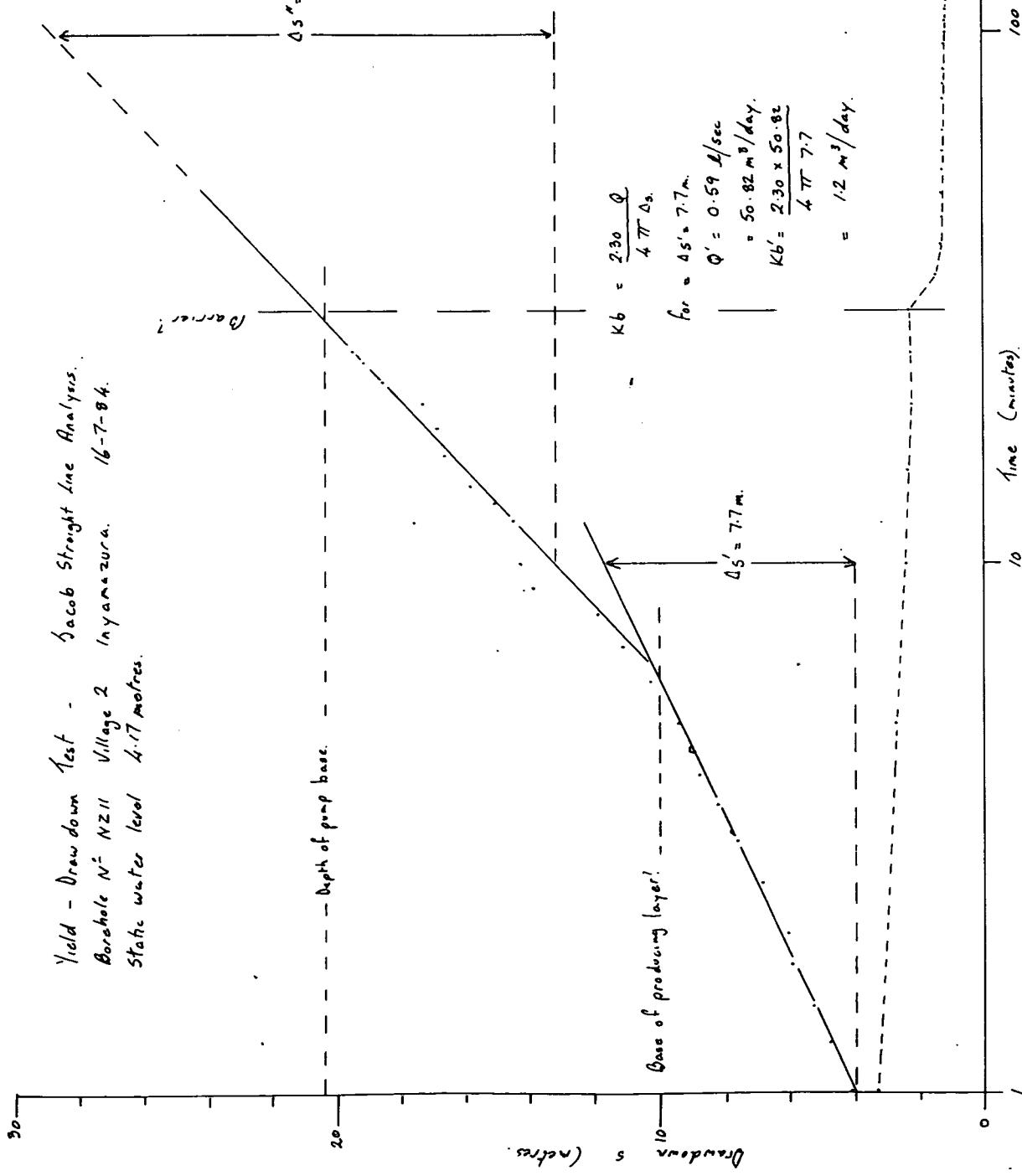
Yield - Variable.

Time since pump stopped. (mins.)	Water level (metres)	<del>Residu.</del> drawdown (metres)	Recovery (metres)	t/gi.
0-15				481.
0-30				241
0-45				161.
0-1.00				121
1-05	23.72	19.55		110.1
1.15	23.36	19.19		97
1-30	23.46	19.02		81
1-45	23.00	18.83		69.6
2-00	22.81	18.64		61
2-30	22. <del>4</del> 49	18.32		49.
3	<del>22-18</del>	18.01		41
3-30	21.88	17.71		35.3
4	21.62	17.45		31
4-30	21.46	17.29		27.7
5	21.455	17.285		25
5-30	21.45	17.28		22.8
6	21.30	17.13		21
6-30	21.10	16.93		19.5
7	20.99	16.72		18.1

Recovery Test B/H N° 12 II 16-7-82 (cont'd)

Time since pump stopped (mins)	Water level (metres)	Residual Drawdown (metres)	Recovery (metres)	0/6'
8	20-60	16-43	16	
9	19-85	15-68	14.3	
10	19-13	14-96	13	
12	18-20	14-03	11	
14	17-72	13-55	9.6	
16	17-30	13-13	8.5	
18	16-54	12-37	7.7	
20	-	-		
22	-	-		
24	-	-		
26	14-20	10-03	5.6	
27	14-10	9-93	5.4	
28	13-90	9-73	5.3	
30	13-53	9-36	5.0	
35	12-74	8-57	4.4	
40	12-01	7-84	4.0	
45	11-32	7-15	3.7	
50	10-89	6-72	3.4	
55	10-48	6-31	3.2	
60	10-14	5-97	3.0	
65	9-81	5-64	2.8	
8-00 AM (17/7/82)	4-60	0-43		

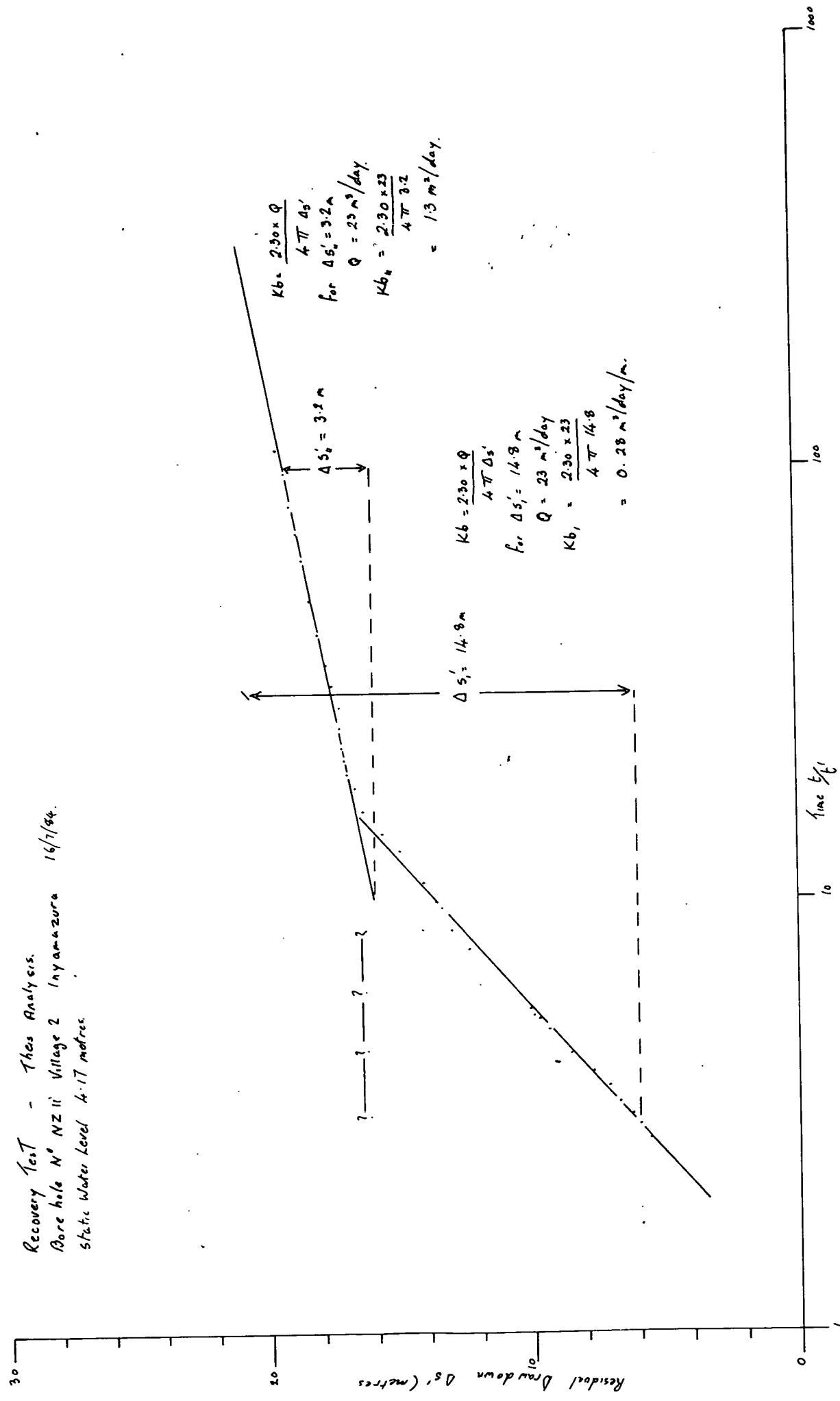
Yield - Drawdown Test - Jacob Straight line Analysis.  
 Borehole No: NZ11 Village 2 Inyamazura. 16-7-84.  
 Static water level 4.17 metres.



Recovery Test - Thesis Analysis.

Bore hole No. NZ II Village 2 Inyamazura 16/7/84.

Static Water Level 4.17 metres.



Bail Test.

Borehole N° NZ 11.B Village N° 2 Nyamazura 31-7-84

REST WATER LEVEL. 6-20 metres.

Capacity of baster 37 litres.

N° of bails. 21.

Time taken. 13 mins 30 secs

Draw down. 13-20 metres.

Amount of water bailed. 777 litres.

Yield 0.96 l/sec. 82.88 m³/day - NB well bailed dry  
after 13½ minutes!

Specific capacity - less than 6.28 m³/day /metre of drawdown				
Time after bailing stopped (mins.)	Water level (metres)	Residual Drawdown (metres)	Recovery (metres)	t/t'
1	19-28	13-0.8	0-12	14.50
2	19-14	12-94	0-26	7.75
3	19-03	12-83	0-37	5.5
4	18-91	12-77	0-43	4.38
5	18-79	12-59	0-61	3.7
10	18-26	12-06	1-14	2.35
15	17-73	11-53	1-67	1.9
20	17-41	11-21	1-99	1.675
25	17-27	11-07	2-13	1.54
30	17-07	10-87	2-33	1.45
35	16-65	10-45	2-75	1.39
40	16-20	10-00	3-20	1.34
45	15-85	9-65	3-55	1.30
50	15-48	9-28	3-92	1.27
55	15-15	8-95	4-25	1.25
60	14-99	8-79	4-41	1.225

Recovery Test = Soil Test. Theis Analysis

Borehole N° NZ 110 Village N° 2 Myanmore 31-7-84

Assume S.T. = State Water Level 6' 20 meters

Manometric Drawdown 13-20 meters

Borehole Borehole D = 1.32 meters

$$K_6 = 2.30 \text{ ft} \quad Q = 182.88 \text{ m}^3/\text{day}$$

$$4 \pi r^2 = 4 \pi (4.5)^2 = 214.0 \text{ m}^2$$

$$K_6' = 2.30 \text{ ft} \quad K_6' = 2.30 \text{ m}^3/\text{day}$$

$$4 \pi r^2 = 4 \pi (5.7)^2 = 321.7 \text{ m}^2$$

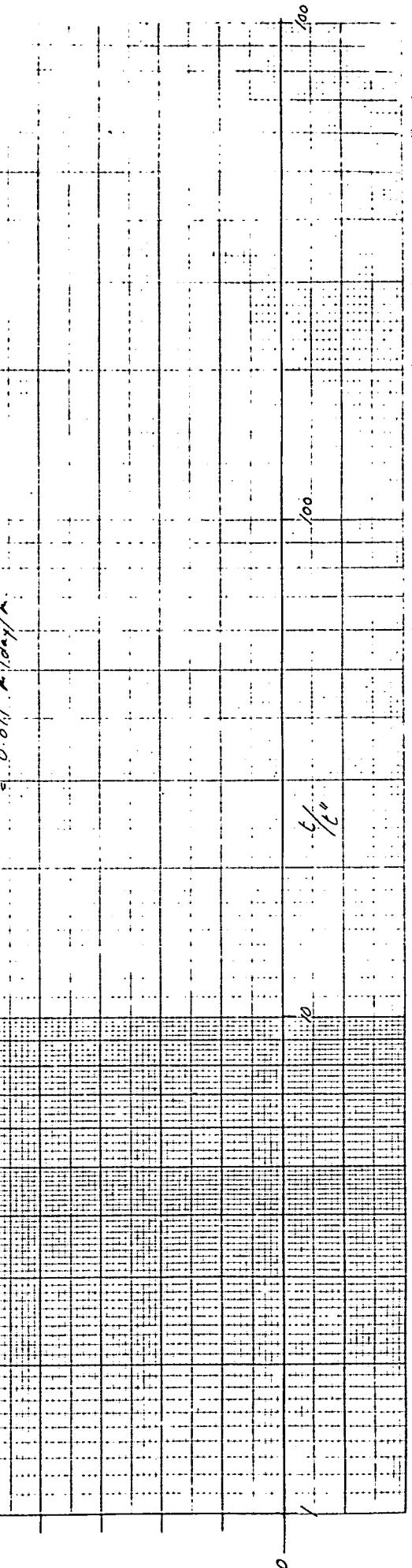
$$Q = 0.49 \text{ m}^3/\text{day/m} \quad 2.66 \text{ m}^3/\text{day/m}$$

$$1/0. Sustained yield of 1445 holes is probably 2.66 m^3/m^2 = 2.85$$

$$K_6 = 2.30 \text{ ft} \quad 2.85$$

$$4 \pi r^2 = 4 \pi (31)^2 = 301.7 \text{ m}^2$$

$$Q = 0.017 \text{ m}^3/\text{day/m}$$



Constant Yield Pumping Test.

Borehole N° NZ 12 A Village. N° 1B NYAMAZURA.

Date 17-7-84

Time test started 15-30 hrs.

Pump cylinder depth 30 metres with 1½" pipe with foot valve to 40 metres.

Ref point — at ground level.

Pump used:- Dando operated single action reciprocating pump.

Time	SWRL (metres)	Time	SWRL	Time	SWRL
15-14	14-04	15-22	13-25	14-12	13-55 - 14-12
15-29	14-12		13-35	14-12	
13-20	14-13		13-45	14-12	

Time (mins.)	Water level (metres)	Drawdown. (metres)	Yield. (secs/20 l)	Temp. (°C)	Conductivity μS.	p.H.
0-15	15-77	1-65	38-7	23-4	310	8-25
0-30	16-12	2-00				
0-45	16-72	2-60				
1-00	17-13	3-01				
1-15	17-53	3-41				
1-30	18-06	3-94				
1-45	18-40	4-28				
2-00	18-73	4-61				
2-30	19-45	5-33				
3-00	20-13	6-01				
3-30	20-94	6-82				
4-00	21-55	7-43				
4-30	22-27	8-15				
5	22-90	8-78	37.5	23-4	310	
6	24-20	10-08				7.95

Constant Yield Pumping Test. Borehole NZ 12. Village 1B. 17-7-84 (cont'd).

Time (mins)	Water level. (metres)	Drawdown. (metres)	Yield (Secs/20L)	Temp. °C.	Conductivity. μs	pH
6-30	24-89	10-77				
7	25-68	11-36				
7-30	26-09	11-97				
8	26-80	12-68				
9	27-80	13-68				
10	29-40	15-28	36-4	23-6	300	7.85
12	32-00	15-88				
15	Dipper stuck under base of first cylinder.		35-6	23-6	240	7.85
20			107-06	23-6	240	7.85
25			133-05			
30			137	23-4	245	8-25
35			133-04	23-4	245	8-25

Test stopped at 16-10 hrs., due to large drawdown and lack of water.

Time - Drawdown Test - Jacob Straight Line Pump  
Bore hole N° NZ 12 - Village 10. Marambaia  
Static Water Level 16-12 metres

Date of P.D.

Drawdown  $S$  (metres)

10

0

Pump Starts To Suck Air

07 m<sup>3</sup>/day

Minimum yield reached

13 m<sup>3</sup>/day

1000  
9  
8  
7  
6  
5  
4  
3  
2  
1

1000  
9  
8  
7  
6  
5  
4  
3  
2

1000  
9  
8  
7  
6  
5  
4  
3  
2

1000  
9  
8  
7  
6  
5  
4  
3  
2

20

Bail Test.

Borehole N° NZ 123. Village N° 13 Nyanazura. 9-8-84.

Rest water level. 6-53 metres.

Capacity of bails. 37 litres.

N° of bails. 51.

Time taken. 30 minutes.

Drawdown. 4-47 metres.

Amount of water bailed. 1.887 m<sup>3</sup>.

Yield. 1.05 l/sec. 90.58 - m<sup>3</sup>/day

Spec. capacity 20.26 m<sup>3</sup>/day / metre of drawdown.

Time after bailing stopped (mins.)	Water level. (metres)	Residual Drawdown. (metres)	Recovery. (metres)	t/sec.
1	10-98	4-45	0-02	31
2	10-96	4-43	0-04	16
3	9-94	3-41	1-06	11
4	9-37	2-84	1-63	8.5
5	9-12	2-69	1-78	7
10	8-27	1-74	2-73	4
15	7-98	1-45	3-02	3
20	7-80	1-27	3-20	2.5
25	7-67	1-14	3-33	2.2
30	7-56	1-03	3-44	2
35	7-46	0-93	3-54	1.86
40	7-35	0-82	3-65	1.75
45	7-11	0-58	3-89	1.67
50	6-82	0-29	4-18	1.60
55	6-72	0-19	4-28	1.55
60	6-68	0-15	4-32	1.50

Perf Test - Record date: Thurs. Aug 26  
Borehole #5 - 12' 2" Village 1/2 mile N.W. 9-8-86  
Reff water level - 53 feet  
Pumped discharge = 10 cubic  
/sec 90.58 x 10<sup>6</sup>  
Duration of test 30 minutes.

Residual drawdown 5'

$$0.5' = 2.9 \text{ meters}$$