

I N S T I T U T E
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H Y D R O L O G Y

A REVIEW OF THE FLOOD STUDIES REPORT
RAINFALL-RUNOFF MODEL PARAMETER
ESTIMATION EQUATIONS

by

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ABSTRACT

The Flood Studies Report, published in 1975, presents a method of design flood estimation based on a rainfall-runoff model and a statistically-derived rainfall input. At the heart of the rainfall-runoff model (the unit hydrograph and losses model) are equations relating model parameters to physical and climatological features of the drainage basin that can be abstracted from maps. These parameter estimation equations have been reviewed in the light of an expanded data set and experience gained in ten years' application of the procedure. New equations are presented that ease application and prove more robust under extreme conditions.



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1 INTRODUCTION

The Flood Studies Report (NERC, 1975) presented two methods of design flood estimation suitable for use at ungauged sites. One of these is based on a rainfall-runoff model and uses a statistically derived rainfall event as input to give an estimate of the flood magnitude of required return period. Since publication of the report more event data have been collected and analysed enabling a review of the method and in particular its parameter estimation equations. The collection of new data was partly a response to comments made at the Flood Studies Conference (Institution of Civil Engineers, 1975) and was an attempt to collect more data on catchments which had provided few events to the published analyses and also data from extra catchments of types not represented in the FSR data set. This report contains the background to the development of the revised set of recommendations summarised in Flood Studies Supplementary Report Number 16.

At this stage in the life of the Flood Studies Report (FSR) it was thought best to introduce improvements whilst maintaining as much as possible of the existing methodology; if changes were to be recommended then they should be easily slotted in to the present framework. This restriction implies a review of the model parameter estimation equations only and this is what was undertaken. The opportunity of such a review has been taken to consolidate all recommendations concerned with the rainfall-runoff method previously published in the FSR and the Flood Studies Supplementary Reports (FSSR's). In addition, the choice of dependent and independent variables used in the regression analyses has been appraised in an attempt to ease the abstraction of catchment characteristics and solve problems encountered in application under extreme conditions.

The analyses presented in this report parallel those given in the FSR and indeed assume a certain knowledge of the FSR (Volume I, Chapter 6 in particular*) The model used in analysis, the unit hydrograph and losses model, comprising a loss rate rainfall separation changing with catchment wetness and unit hydrograph derivation using matrix inversion with smoothing, is exactly as used previously in the FSR.

2 DATA

The data required for this study are of two types: catchment characteristics obtained from maps and event parameters obtained from observations and analyses. For those catchments and events used in the FSR, these data already existed in a well collated form. For new events, flow, rainfall and soil moisture deficit data were collected and processed as required before analysis. Each event yielded parameters relating to the event, such as rainfall and antecedent flow, and model parameters, such as unit hydrograph time to peak and percentage runoff. Where the new events were from catchments for which event data had not previously been collected then the pertinent catchment characteristics were abstracted.

*References to the Flood Studies Report will be given in the form Volume, chapter, section eg FSR I,6.1

To aid the storage, collation and analysis of these data a computer data base management system, the Flood Event Data Archive, was developed. This facility, which is described in Appendix B, contained many features not available at the time of the FSR. To aid abstraction of catchment characteristics, catchment boundaries were digitised and stored, thus enabling the production of computer constructed maps with precisely located catchments. The accuracy of these maps showed up many discrepancies in maps used for the FSR and led to the checking of all map derived catchment characteristics. Similarly, the new event analyses revealed that the haste with which the original analyses had been performed had allowed many events to pass the data checking stage that had poorly defined rainfalls or a possible snowmelt contribution. All event data were inspected and coded as being of quality suitable for derivation of a unit hydrograph, only suitable for assessing volumes of rainfall and flow, or of poor quality and not suitable for use in the current study.

These reviews, it was hoped, left a well organised and carefully vetted data set ready for the next stage in the analysis, the generalisation of the model parameters to the ungauged site.

Two tables in Appendix A contain the data. Table A.1 lists all the catchment characteristics abstracted for catchments in this study. Because previously derived values have been reviewed, values may differ from those published earlier. Table A.2 gives the event information and contains both event details and derived model parameters.

At the start of the second phase of data collection it had been hoped that a large increase in the total number of useful events would result. The outcome was somewhat disappointing; although over 900 new events were selected, fewer events were available for unit hydrograph analysis than had been used in the FSR. The table below summarises data availability.

TABLE 2.1 NUMBER OF EVENTS AND CATCHMENTS FOLLOWING DATA VALIDATION

Events	Total (including rejects)	2564 (1631)
	Volume analysis	1910 (1488)
	Unit hydrograph	1306 (1351)
Catchments	Total (at least one event)	210 (138)
	With at least one unit hydrograph event	181
	With at least five volume analysis events	174 - 1822 events
	With at least five unit hydrograph events	128 - 1159 events

Figures in brackets are for the equivalent FSR data set

The table shows that only half of all events selected were suitable for unit hydrograph modelling and that on many catchments only a few events were available (82 of the 210 catchments were providing less than five events suitable for full analysis). The new data set is larger in terms of numbers of catchments and events suitable for volume analysis and there has been a very small decrease in events suitable for unit hydrograph analysis. That there should be any decrease

at all having collected so much new data reflects the very much more stringent checking and acceptability criteria that were applied in the review.

Before deriving the ungauged catchment regressions a small data set was withdrawn to provide an independent sample for testing. This comprised six catchments with 59 events, 42 of which were suitable for unit hydrograph studies. Table 2.2 details the test data set.

TABLE 2.2 THE TEST DATA SET

Catchment	unit hydrograph events	loss study events
19001	5(5)	5(5)
29001	7(6)	9(6)
39012	6(8)	11(10) *
40004	11(15)	15(15)
54016	4(12)	7(16)
67010	9(12)	12(12)
Total	42(58)	59(64)

Figures in brackets are for the equivalent FSR data set

*Replaces 29003 withdrawn in FSR

3 UNIT HYDROGRAPH PARAMETER ESTIMATION

The parameters abstracted from the derived unit hydrographs were peak in hours (T_p), peak in cumecs per 100 sq km (Q_p), and the width of the peak (W), in hours; their definitions are shown graphically in Fig. 3.1. If in analysis a data interval other than one hour was used an adjustment was applied to give equivalent one hour unit hydrograph parameter values (FSR I, 6.4.8).

The FSR showed the strong interdependence of the unit hydrograph parameter values and gives expressions for Q_p and W in terms of T_p alone. Figures 3.2 and 3.3 show catchment average values of Q_p and W plotted against T_p with the lines representing the FSR equations superimposed. As can be seen the equations fit the data reasonably well and there seems to be no need for change. Table 3.1 gives details of regression equations fitted to the new and the original data sets but are intended for information only and are not recommended for use.

Users of the FSR will be aware that the loss model recommended for design use is a percentage one yet a loss rate based model has been used in analysis.

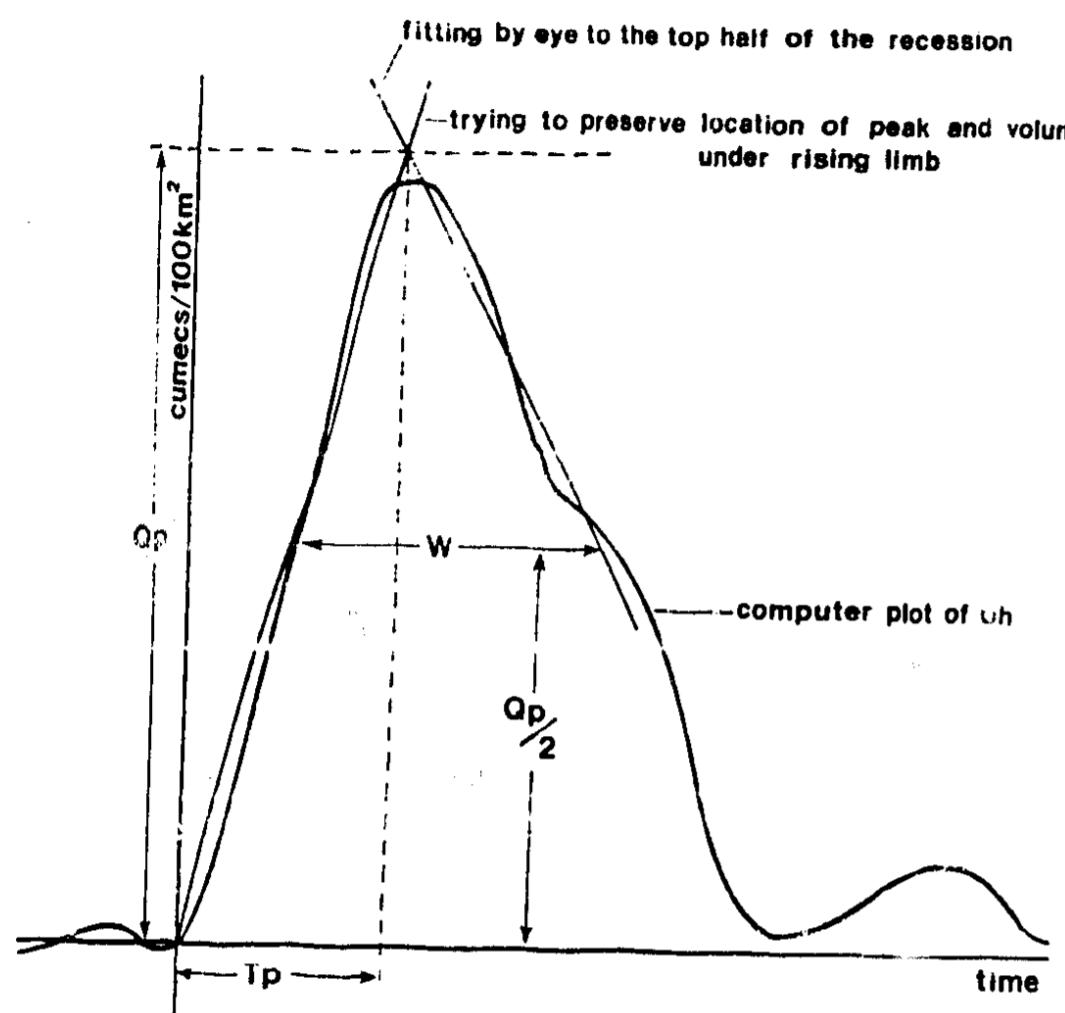


FIGURE 3.1 Definition and fitting of the FSR parameters of the unit hydrograph T_p , Q_p and W

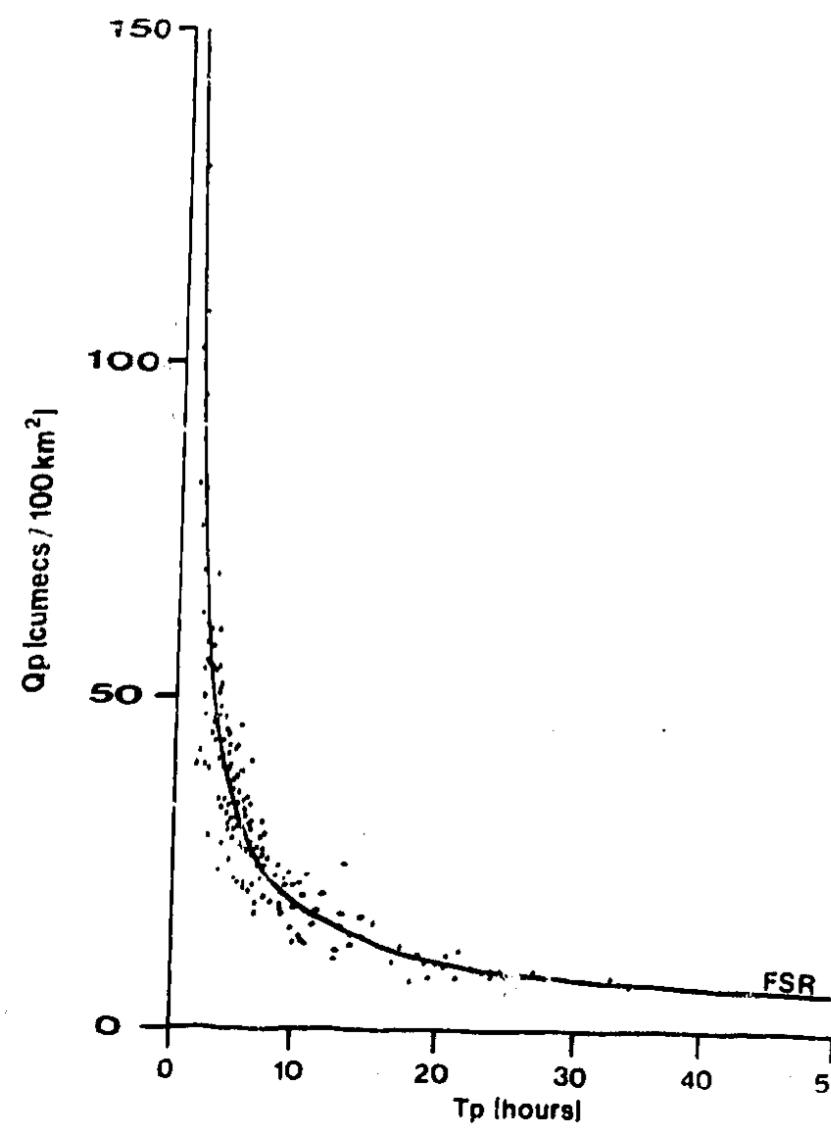


FIGURE 3.2
Catchment average values of Q_p plotted against T_p

TABLE 3.1 REGRESSION OF SECONDARY UNIT HYDROGRAPH PARAMETERS

Independent variable	Estimate	se	t	R^2	see	observations
Catchment averages						
T_p	2.809	0.392	7.17	0.229	347	175
Constant	157.1	4.130	38.03	-	-	-
FSR						
T_p	2.587	0.407	6.35	0.241	334	129
Constant	162.2	4.832	33.6	-	-	-
Dependent variable : W/T_p						
Catchment averages						
T_p	- 0.0154	0.00354	4.35	0.10	0.314	175
Constant	1.483	0.0374	38.6	-	-	-
FSR						
T_p	- 0.00834	0.00325	2.57	0.049	0.267	129
Constant	1.399	0.0386	36.2	-	-	-

Headings in this and subsequent tables contain the following abbreviations

- se standard error of the coefficient
- t t statistic
- R^2 coefficient of determination
- see standard error of the estimate
- sfe standard factorial error of the estimate.
- RMS root mean square

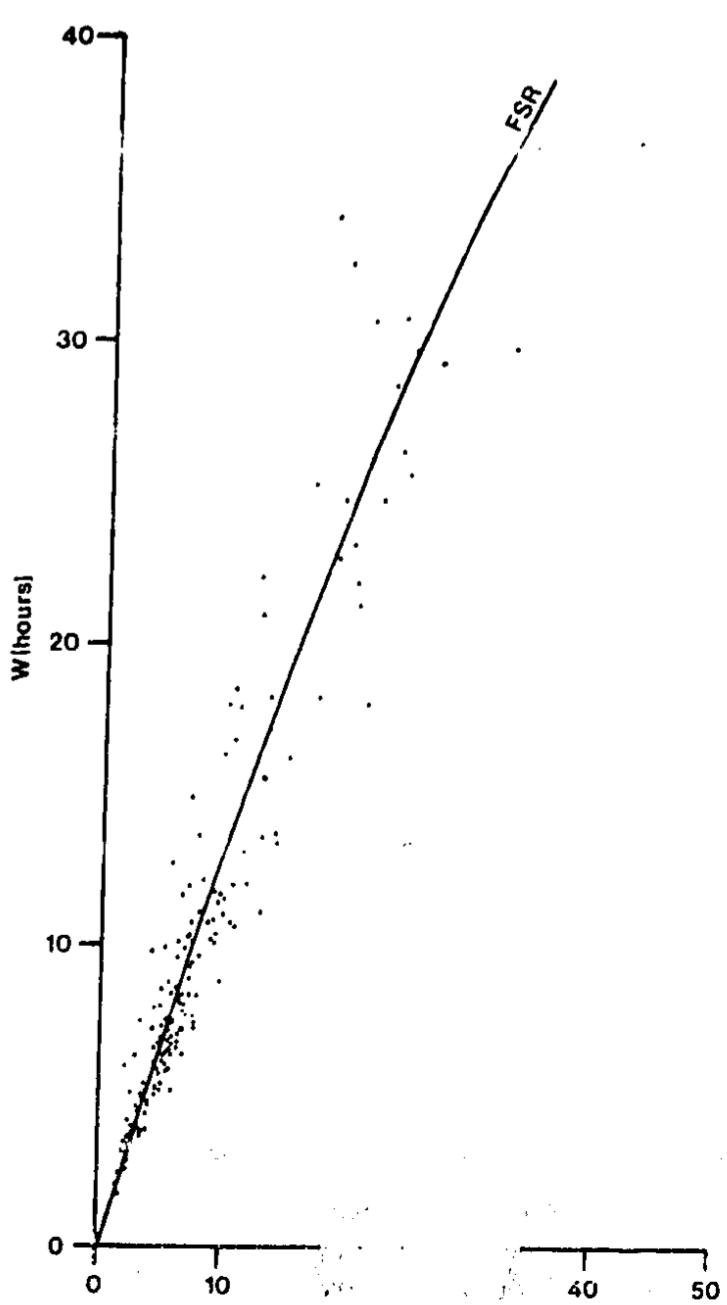


FIGURE 3.3
Catchment average values of W plotted against T_p

The interaction between unit hydrograph shape and loss model is such that an adjustment must be applied to allow for the change in rainfall separation technique. FSR I,6.5.9 details the effect this may have. The simplifying assumption of using a triangular unit hydrograph and modifying the QpTp relation given in FSR I,6.5.2 is still considered valid as the original equations relating internal parameters are given support by the new data set.

In the FSR, Tp is estimated from a regression equation that has streamslope, urban fraction, mainstream length and RSMD as the independent variables. The meanings of the first three of these are obvious, although it is important that they are abstracted in the prescribed manner (FSR I,4.2). RSMD is an effective 5 year return period 1 day rainfall that is laborious to calculate and easy to mis-calculate. Since it is highly correlated with SAAR, the standard period average annual rainfall, which is much easier to derive, it was decided to replace RSMD with SAAR if this could be achieved without loss of accuracy in estimation.

The use of Tp, the 1-hour unit hydrograph time to peak, can also cause problems in application of the estimation equation on fast responding catchments. In such cases the required data interval may be a half or quarter hour and the 1-hour Tp must be adjusted. As the lower bound of estimated Tp's is zero it is clearly possible for adjusted Tp's to be negative, although it is more likely that they will just be unreasonably small. This problem can be avoided by developing a regression equation for time to peak of the instantaneous unit hydrograph. Whereas the 1-hour unit hydrograph represents the response to 10 mm of rainfall occurring in one hour, the instantaneous unit hydrograph is the response of an equal, but instantaneous, rainfall at the start of the hour. The time to peak of the instantaneous unit hydrograph Tp(0) can be estimated from the 1-hour time to peak, Tp(1), by $Tp(0) = Tp(1) - 1/2$.

Table 3.2 gives details of regression equations derived from the new data set and incorporating these amendments. While the equations based on the new data set and using $Tp(1)-1/2$ as the independent variable appear inferior to the original FSR equation the change in independent variable makes the comparison almost meaningless. In terms of estimating $Tp(1)$ then the RMS errors from using equation 1 and equation 3 (in Table 3.2) are 4.03 and 4.00 respectively; the equations perform equally well. Replacing RSMD by SAAR has not led to any loss in accuracy in estimating Tp. Equation 4 in Table 3.2 gives details of a regression identical in form to Equation 3 only on a restricted data set; only catchments averages coming from at least five events were included. This considerably reduces the number of observations and the fit of the equation is slightly better than before. The coefficients differ by less than one standard deviation between regression equations. If catchments with more than 5% urbanisation are excluded from the data set then the resulting three variable equation has coefficients very similar to those shown in Table 3.2.

It has therefore been possible to incorporate the desired changes in the Tp estimation equation without any loss in accuracy; the recommendation is that $Tp(T)$, where T is the required data interval, should be estimated from

$$Tp(0) = 283.0 \cdot S1085^{-0.33} \cdot (1+URBAN)^{-2.16} \cdot SAAR^{-0.54} \cdot MSL^{0.23} \quad 3.1$$

$$Tp(T) = Tp(0) + T/2 \quad 3.2$$

Figure 3.4 shows observed catchment average values of Tp(1) plotted against values estimated from the above equations.

For situations where stage data are available, but no rating exists to

TABLE 3.2 REGRESSION OF Tp ON CATCHMENT CHARACTERISTICS

Independent variable	Estimate	se	t	R ²	see	sfe	observations
1. FSR							
Log(S1085)	- 0.381	0.066	.77	0.780	0.150	1.41	130
Log(1+URBAN)	- 1.995	0.284	7.02	-	-	-	-
Log(RSMD)	- 0.417	0.118	3.52	-	-	-	-
Log(MSL)	0.139	0.050	2.77	-	-	-	-
Constant	1.669	0.140	11.91	-	-	-	-
Note: MSL forced into this four-variable equation.							
Dependent variable : log(Tp(1)-1/2)							
2. Catchment averages							
Log(S1085)	- 0.288	0.063	4.59	0.742	0.17	1.48	175
Log(1+URBAN)	- 2.152	0.298	7.20	-	-	-	-
Log(RSMD)	- 0.653	0.128	5.09	-	-	-	-
Log(MSL)	0.249	0.047	5.25	-	-	-	-
Constant	1.804	0.159	11.33	-	-	-	-
3. Catchment averages							
Log(S1085)	- 0.327	0.0597	5.47	0.736	0.17	1.48	175
Log(1+URBAN)	- 2.164	0.303	7.15	-	-	-	-
Log(SAAR)	- 0.538	0.115	4.69	-	-	-	-
Log(MSL)	0.228	0.0465	4.89	-	-	-	-
Constant	2.452	0.299	8.21	-	-	-	-
4. Catchments with at least 5 events							
Log(S1085)	- 0.272	0.062	4.06	0.800	0.15	1.41	120
Log(1+URBAN)	- 2.156	0.0317	6.80	-	-	-	-
Log(SAAR)	- 0.619	0.124	4.99	-	-	-	-
Log(MSL)	0.308	0.053	5.78	-	-	-	-
Constant	2.553	0.318	8.04	-	-	-	-

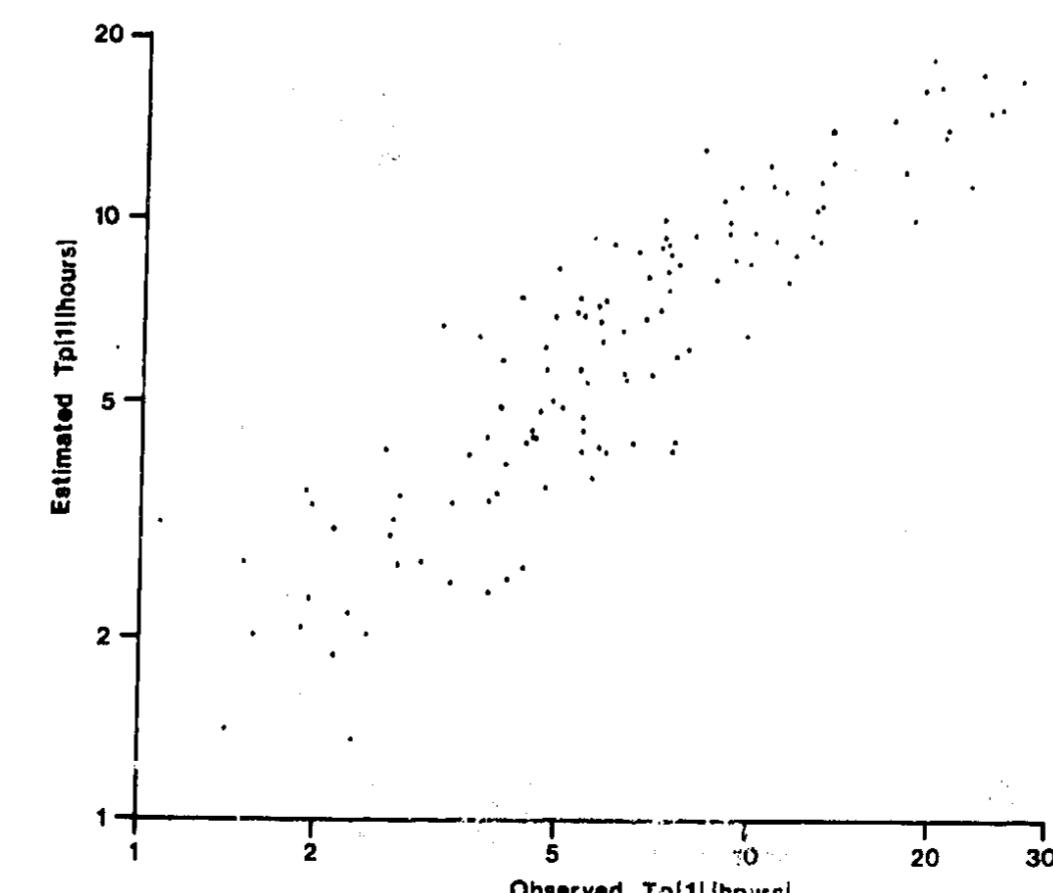


FIGURE 3.4

Estimated against observed (catchment average) values of the 1 hour unit hydrograph time to peak

obtain flows, the FSR gives an equation which can be used to estimate T_p from the catchment lag, as defined in FSR I, 6.4.2. The equation is

$$T_p(1) = 0.9 \text{ Lag} \quad 3.3$$

Figure 3.5 shows catchment average values of Lag against T_p for the new data set; also plotted on this figure is the FSR equation, which represents the observed data reasonably well. However, the use of equation 3.3 would be inconsistent with the earlier recommendation to estimate $T_p(T)$ via $T_p(o)$ and could lead to the same problems in estimating time to peaks on quickly responding catchments. $T_p(o)$ may be estimated from the catchment lag using

$$T_p(o) = 0.604 \text{ Lag}^{1.144} \quad 3.4$$

which has been derived by regression analysis using catchment average data. Table 3.3 gives details of regression analyses on the new data set.

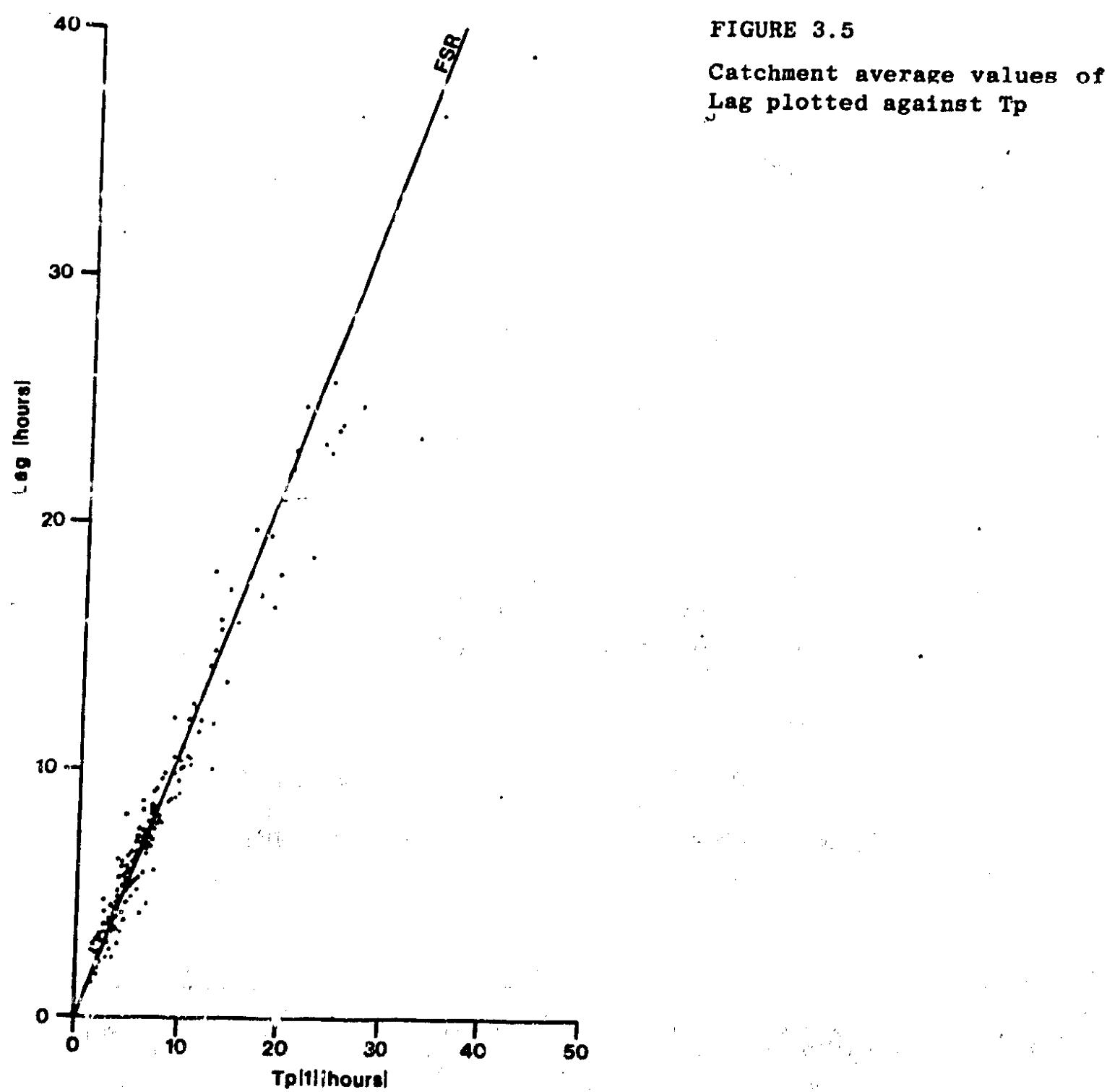


TABLE 3.3 REGRESSION OF T_p ON LAG

Dependent variable : Log($T_p(1)$)							
Independent variable	Estimate	se	t	R^2	see	sfe	observations
1. FSR							
Log(Lag)	0.99	0.174	57.01	0.962	0.061	1.15	129
Constant	- 0.36						
2. All catchments							
Log(Lag)	1.041	0.0216	48.14	0.930	0.0790	1.20	175
Constant	- 0.085	0.0194	4.39	-	-	-	-
3. All events							
Log(Lag)	0.973	0.0145	67.05	0.78	0.158	1.44	1264
Contant	- 0.042	0.0127	3.34	-	-	-	-
Dependent variable Log($T_p(1)$) - 1/2							
4. All catchments							
Log(Lag)	1.144	0.025	46.41	0.926	0.090	1.23	175
Gons'tant	- 0.219	0.022	9.91	-	-	-	-

4 PERCENTAGE RUNOFF ESTIMATION

In the FSR the estimation of percentage runoff is seen as the most important and yet most uncertain part of the design flood estimation procedure. The usefulness of local data in refining percentage runoff estimates cannot be emphasized too strongly. While the equations presented below are applicable in situations in which no data are available they should be considered as a basis for preliminary flood estimation only.

Variations in percentage runoff can be divided into two components. On a single catchment, percentage runoff will vary with storm characteristics (depth, duration and profile of rainfall) and with catchment state at the onset of rainfall (what stores or deficits of water exist in the catchment eg. soil moisture, surface storage). Thus we expect a larger percentage response from a large storm on a wet catchment than from a small storm on a dry one. When percentage runoffs are compared between catchments then these 'dynamic' variations are superimposed on physical (constant) differences between the basins (soils, geology, land use etc). A catchment located on chalk will give very much lower runoff than one situated on clay. While it is easy to express these intuitive concepts in this general way, in practice it is difficult to define precise relationships supported by real data. This is because of the difficulties in collecting event data that cover the full range of combinations of catchment state, storm variability and catchment type, and in finding suitable indices for these variables. Difficulties may also accrue from a poor choice of rainfall-runoff model or poor model parameterisation but in the present context these are considered relatively unimportant.

The FSR presents an equation that assesses percentage runoff as a standard term, dependent on a soil index and the urban fraction, and a dynamic term that varies with total storm depth (P) and a catchment wetness index (CWI).

$$PR = 95.5 \text{ SOIL} + 12 \text{ URBAN} + 0.22(\text{CWI-125}) + 0.10(\text{P-10.0})$$

4.

where **CWI** = $125 + \text{API} - \text{SMID}$

AP I is an antecedent Precipitation Index.

and **SMD** is the prevent soil moisture deficit.

SOIL = $0.15S1 + 0.30S2 + 0.40S3 + 0.45S4 + 0.50S5$

This recommendation was updated in FSR 5, 'Design flood estimation in catchments subject to urbanisation', which split the FSR data into urban and rural subsets. Apart from the urban term the revised percentage runoff equation has the same general structure as the one presented in the FSR.

$$\text{P}_{\text{rural}} = 102.4 \text{ SOIL} + 0.28(\text{CWI}-125) + 0.1(\text{P-10}) - 1.9$$

An urban ~~correction~~ can be applied, when required, such that

$$PR = PR_{rural}(1 - 0.3 \text{ URBAN}) + 21,0 \text{ URBAN}$$

In this relationship a distinction is made between those areas mapped as being urbanised that are actually impervious, and the surfaces within the urban areas that are in fact pervious. Equation 4.5 represents urban areas as having a 70% pervious component which responds as the natural part of the catchment and a 30% impervious component which gives a constant 70% runoff. These values come not from the analysis of FSR data but from fully sewered catchment data (Packman, (1977) Kidd and Lowing, (1979)). With this form of urban correction the urban effect is more evident on catchments with low SOIL values whereas a constant addition results from the FSR equation. The removal of the urban catchments from the PR regression has led to a larger coefficient for the CWI term (compare equations 4.1 and 4.4). This accords with the expectation that soil moisture deficits are a more important indicator of between event response variation on natural catchments than on those that are urbanised.

As well as having an enhanced data base on which to calibrate a new percentage runoff equation there are other factors that may affect the derived equations. The original WRAP* map as presented with the FSR left some (mainly urban) areas unclassified; these have subsequently been classified. Other relatively minor changes were also made to the WRAP map leading to a revised version of the map being published (FSSR 7). Using the new map and the accurately defined catchment boundaries has led to a set of soil fractions quite different in some cases from those used previously. Since the SOIL index was calibrated on the original FSR data set and because it is the most useful variable in explaining observed differences, it seemed reasonable to review the form of this index as well as the PR equation itself. At a preliminary stage of the analysis for the current review a further modification to the WRAP map was indicated. This is detailed in FSSR 17 and described more fully in Appendix C.

Prior to recalibration of the soil multipliers, percentage runoffs were adjusted for urban effects by inverting equation 4.5 to yield a rural percentage runoff. It was then hoped to determine coefficients for the soil fractions and dynamic terms in a single regression. Unfortunately this proved unreliable because of the limited occurrence of large rainfall events. Omitting the dynamic term dependent on rainfall gave

$$\text{rural} = -0.01 + 29S2 + 37S3 + 47S4 + 52S5 + 0.25(\text{CNI}-125) \quad 4.6$$

an equation slightly different from, but consistent with, the FSR result. To assess the urban correction a regression based on observed PR's from nearly rural (< 5% urbanisation) was carried out and gave

$$PR = 11S1 + 31S2 + 38S3 + 48S4 + 53S5 + 0.24 \text{ (CWI-125)}$$

a result in close agreement with equation 4.6. Regressions similar to equation 4.7 but based on different urban thresholds yielded comparable results. Equations 4.6 and 4.7 are therefore seen as a reasonable basis for a new PR equation. Details of the regressions are in Table 4.1. Convenient values of the coefficients were taken giving

$$PR_{rural} = 10S1 + 30S2 + 37S3 + 47S4 + 53S5 + 0.25(CWI-125) \quad 4.8$$

Examination of the residuals reveals a significant correlation with the event rainfall which was again fitted by regression analysis (see Table 4.2).

TABLE 4.1 PERCENTAGE RUNOFF REGRESSIONS

Independent variable	Estimate	se	t	R ²	see	observations
1. All events						
SOIL1	10.01	1.46	6.8	0.46	14.9	1851
SOIL2	28.71	1.11	25.8			
SOIL3	36.93	1.31	28.2			
SOIL4	46.62	0.73	64.1			
SOIL5	52.46	0.71	73.6			
(CWI-125)	0.253	0.013	19.9			
2. All events from catchments with urban < 5%						
SOIL1	10.77	1.36	5.8	0.37	14.9	1460
SOIL2	31.10	1.22	25.6			
SOIL3	38.24	1.59	24.1			
SOIL4	47.61	0.88	54.3			
SOIL5	52.53	0.73	72.2			
(CWI-125)	0.242	0.02	14.0			

TABLE 4.2 PERCENTAGE RUNOFF RESIDUAL REGRESSIONS

Dependent variable : PR _{rural} residuals							
Independent variable	Estimate	se	t	R ²	see	observations	
P	0.129	0.016	8.20	0.04	14.6	1851	
Constant	- 5.256	0.670	7.84				

* WRAP = Winter Rain Acceptance Potential

The resulting model for PR is represented by the three equations

$$SPR = 10S1 + 30S2 + 37S3 + 47S4 + 53S5 \quad 4.9$$

$$PR_{rural} = SPR + 0.25(CWI-125) + 0.13(P - 40) \quad 4.10$$

$$PR = PR_{rural} (1 - 0.3 URBAN) + 21.0 URBAN \quad 4.11$$

These equations represent a slight improvement over the PR formulae given in the FSR and in FSSR 5. The two main differences between the new equations and those presented in the FSR are the reduction in response from type 1 soils and the increase in magnitude of the dynamic terms. The first of these is explained by looking at the catchments located mainly on type 1 soils. Over half of the catchments mainly located on type 1 soils that were analysed in the FSR had significant urban areas ($>10\%$ urbanisation) and yet no adjustment was made prior to calibration of the SOIL index; the current analysis does have such a correction and gives a lower natural response. The increase in the dynamic term coefficients is again seen as a product of the urban adjustment and partly perhaps because of the combined fitting of soil and CWI multipliers.

The three equations 4.9 - 4.11 give a percentage runoff estimation method based on the current data set and data from small urban (sewered) catchments. While their accuracy is limited (see. of 15%) they are thought to provide a sound blend of empirical analysis and hydrological theory and could be used with due caution in conditions similar to those used to derive the equations. Restriction on their application is warranted on catchments where there is a large urban fraction and especially where this is concentrated rather than distributed throughout the catchment. The dynamic conditions encountered in the data set cover the full range of values for CWI that are likely to be experienced in even the most extreme design case; the same cannot be said for the event rainfalls of which less than 2% are greater than 100 mm. This introduces reservations over extrapolating equation 4.10 to apply to rainfall totals much greater than 100 mm. Unfortunately this is unavoidable when seeking to estimate probable maximum floods (PMF's) for which the design rainfall may be as high as 400 mm. Clearly these relationships derived from common events are not easily extrapolated to extreme situations. In estimating PMF's it is necessary to consider the general philosophy behind the estimate which in the case of the FSR was a reasonable maximisation of all contributory factors. The absolute maximum for percentage runoff is 100% so it seems reasonable to limit percentage runoff to this value under the most extreme conditions likely to occur, say a rainfall of 400 mm on a catchment with a CWI of 200 mm and, if possible, to approach this limit asymptotically.

It is therefore recommended that the rainfall term in equation 4.10 be replaced by

$$0 \text{ for } P < 40 \text{ mm}$$

$$\text{and } 0.45(P-40)^{0.7} \text{ for } P > 40 \text{ mm}$$

This adjustment gives close agreement with the fitted equation (4.10) over the rainfall range 30 mm - 110 mm and yields 99.5% runoff from an event of 400 mm occurring on a soil type 5 catchment with CWI of 200 mm. This form of relationship will also alleviate problems of negative percentage runoffs estimated for small events on catchments with mainly type 1 soils. Although this is a most unlikely occurrence in design use it could happen in other applications and indeed should be expected in conditions where the percentage runoff is less than the standard error of estimate of the PR equation. Users who are concerned

about the arbitrary nature and dubious conception of this correction should remember that estimates of PR using the regression equation have a standard error of estimate of 15%.

The percentage runoff model can therefore be summarised by the equations

$$PR_{rural} = SPR + DPR_{CWI} + DPR_{RAIN} \quad 4.12$$

where

$$SPR = 10S1 + 30S2 + 37S3 + 47S4 + 53S5 \quad 4.9$$

$$DPR_{CWI} = 0.25 (CWI-125) \quad 4.13$$

$$DPR_{RAIN} = 0.45(P-40)^{0.7} \quad \text{for } P > 40 \text{ mm} \\ = 0 \quad \text{for } P \leq 40 \text{ mm} \quad 4.14$$

$$PR = PR_{rural} (1.0 - 0.3 URBAN) + 21.0 URBAN \quad 4.11$$

Figure 4.1 shows catchment average values of percentage runoff estimated using these equations plotted against the observed values.

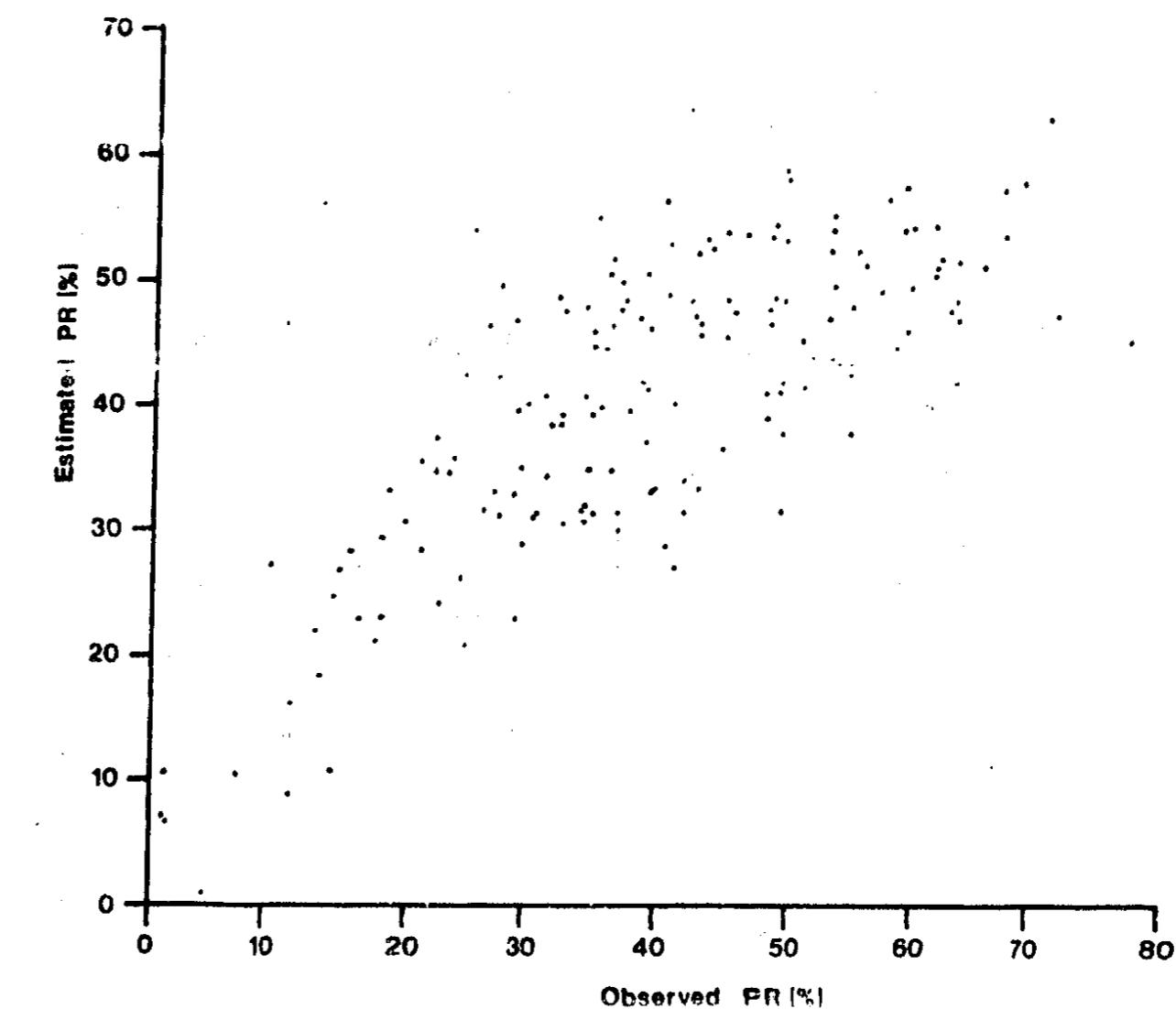


FIGURE 4.1 Catchment average values of estimated PR plotted against observed PR

It is possible to revise the estimates of percentage runoff obtained from the above equations by using data available at the site of interest.

If event data (ie flow and rainfall data at a short time period) are available in sufficient quantity then it may be possible to derive a relationship based on these data alone. More normally though it is only possible to replace the value of SPR obtained from the WRAP map. To do this requires the careful abstraction of the flow hydrograph and corresponding rainfall for at least five big flood events. The hydrograph should be separated as described in FSR I.6.4.3 and the volume of quick response runoff calculated. PR is the ratio of quick response runoff to total rainfall expressed as a percentage. If the catchment contains an urban fraction, adjust PR to give PR_{rural} using equation 4.11 (otherwise PR_{rural} = PR). The dynamic adjustments are given by equations 4.13 and 4.14 and once calculated should be subtracted from PR_{rural} to give SPR. SPR's calculated from several events should be averaged and the resulting value becomes the new estimate of SPR for use in design flood estimation.

Many sites however may just have daily mean flow data; this can be used to refine a SPR estimate by using the base flow index (BFI) derived from these daily flows. The BFI is the ratio of base flow to total flow using a particular form of flow separation (Low Flow Studies, NERC) and is well correlated with SPR. Table 4.3 details a regression analysis which gives the equation

$$SPR = 72.0 - 66.5 BFI$$

4.15

TABLE 4.3 SPR - BFI REGRESSIONS

Independent variable	Estimate	se	t	R ²	see	observations
1. All catchments						
BFI	- 64.24	4.47	14.36	0.56	9.4	166
Constant	71.03	2.18	32.54	-	-	-
2. All catchments weighted by number of available events						
BFI	- 66.49	4.28	15.55	0.59	8.97	166
Constant	72.05	2.10	34.38	-	-	-

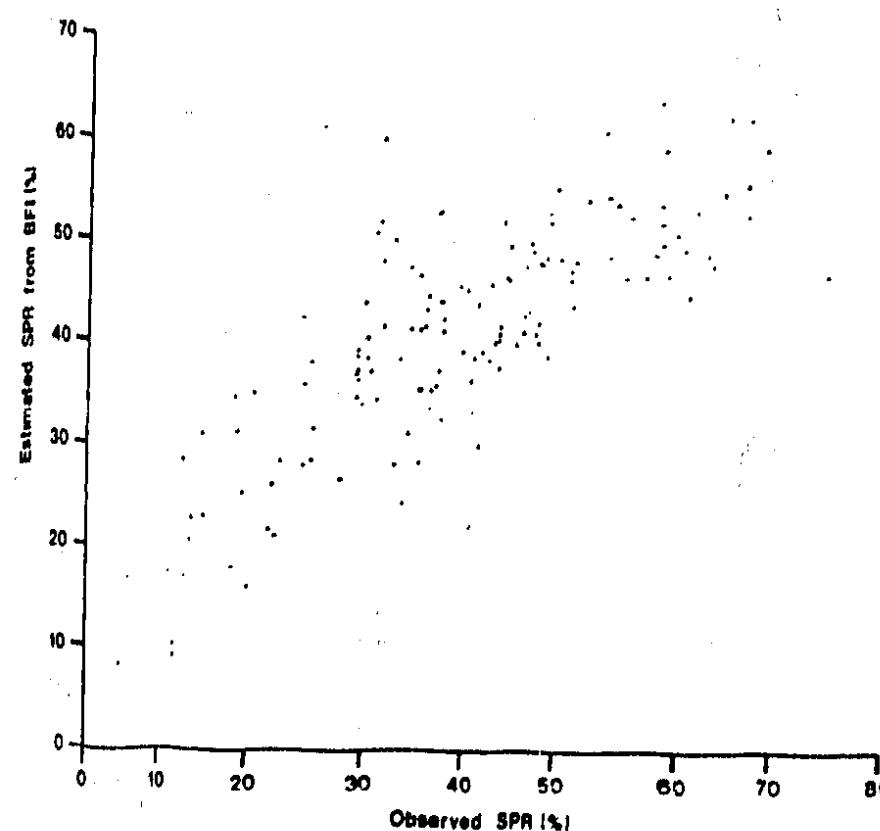


FIGURE 4.2

SPR estimated from BFI plotted against catchment average values calculated from event data

This equation should be used in preference to the soil equation (equation 4.9) as it is based on hydrological data at the site of interest but will yield inferior estimates to those obtained from event analysis. Figure 4.2 shows SPR's estimated using equation 4.15 plotted against observed values.

5 BASE FLOW ESTIMATION

The equations given in the previous two sections allow the response hydrograph to be estimated. The final step in the formulation of the design flood hydrograph is the addition of a certain flow to represent the flow in the river before the event started and to a lesser extent the start of slow response runoff from the event itself. This flow was labelled average non-separated flow (ANSF) by the FSR, a term that serves as a reminder that the hydrograph was separated as an expedient for analysis and does not represent a separation of flow generated by different runoff processes.

Regression analysis yields an equation for ANSF in terms of the variables CWI and SAAR

$$ANSF = (33(CWI-125) + 3.0 SAAR + 5.5) \times 10^{-5}$$

5.1

where ANSF is the additional flow in cumecs/km² that should be added to each ordinate in the design flood. Details of the equation can be found in Table 5.1.

TABLE 5.1 THE ANSF REGRESSION

Independent variable	Estimate	se	T	R ²	see	observations
CWI	0.000328	0.0000216	15.63	0.420	0.025	1851
SAAR	0.0000298	0.0000011	28.10	-	-	-
Constant	- 0.04059	0.002598	15.20	-	-	-

In the design situation CWI is determined directly from SAAR using Figure FSR I.6.62. ANSF is therefore solely dependent on SAAR and the value obtained from the equation can be checked against the graphed relationship given in Fig. 5.1 which shows ANSF against SAAR.

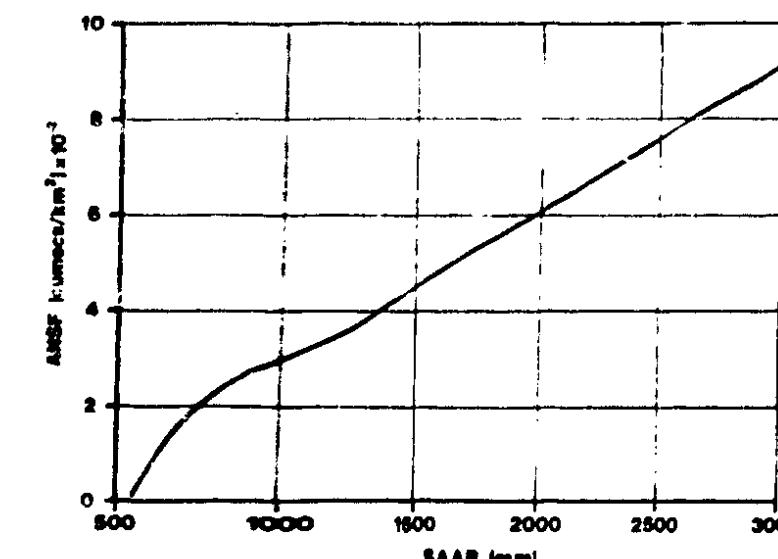


FIGURE 5.1

Graphical representation of ANSF-SAAR relationship for design use

6 ASSESSMENT OF PROCEDURES

Although a comparison of observed and estimated parameter values for catchments used in the regression studies is not a particularly rigorous assessment, for the equations given in sections 3, 4 and 5, a list of observed average, maximum and minimum values together with their estimates is contained in Table 6.1. An asterisk in the table indicates that some events on the catchment have been used for volume analysis only.

TABLE 6.1 OBSERVED AND ESTIMATED PARAMETER VALUES

CATCHMENT	PERCENTAGE RUNOFF			TIME TO PEAK(H)		
	OBSERVED MEAN	OBSERVED MAX	OBSERVED MIN	ESTIMATED MEAN	ESTIMATED MAX	ESTIMATED MIN
19002	62-70	13.57	14.57	47.44	10.27	*
19003	57-83	71.72	46.53	49.20	50.01	*
20001	32-37	40.39	18.39	19.27	31.65	*
21030	33-51	46.58	26.54	33.30	35.52	*
23002	35-73	50.13	20.38	30.45	42.45	*
23005	53-77	60.07	45.27	52.40	53.58	*
23006	40-74	40.76	40.74	54.97	52.49	*
23010	49-55	53.12	31.37	53.13	54.98	*
24003	48-70	50.58	42.19	54.66	50.31	*
24007	28-75	44.01	13.36	46.71	37.39	*
25003	36-79	57.93	25.10	46.32	39.91	*
25014	27-71	40.47	25.10	51.51	51.40	*
25026	47-73	50.55	17.02	44.66	37.35	*
25011	40-72	53.91	40.34	53.59	55.29	*
23012	65-89	72.16	59.63	51.22	56.99	*
25810	52-90	55.20	43.02	47.05	56.99	*
27001	42-25	64.12	21.70	48.16	38.36	*
27010	34-78	34.19	34.18	32.38	21.26	*
27026	35-38	53.96	21.32	39.80	43.16	*
27027	53-74	90.20	29.01	55.16	49.27	*
27031	42-53	59.80	25.47	41.83	47.34	*
27035	35-69	51.41	23.30	44.48	38.43	*
28016	27-52	34.47	22.03	47.46	50.19	*
24023	17-50	32.31	9.02	21.21	26.91	*
28026	48-29	50.05	41.48	21.10	25.48	*
25033	25-30	41.33	10.52	46.49	39.06	*
24041	45-50	58.16	29.07	53.79	46.92	*
29004	40-59	70.39	20.55	48.81	43.71	*
30001	27-50	36.61	11.63	20.63	34.80	*
30004	20-31	27.12	9.60	26.69	19.92	*
31003	54-58	60.17	27.66	43.40	19.40	*
31010	38-53	51.09	23.14	41.91	36.08	*
31021	14-33	22.62	7.04	30.59	26.14	*
32001	25-15	25.73	25.15	30.23	53.25	*
33014	38-69	66.61	15.98	46.15	43.30	*
33015	13-66	18.20	7.27	22.11	20.51	*
33029	34-50	45.50	18.38	39.19	34.05	*
33045	7-26	11.11	3.11	10.54	5.62	*
33049	18-25	28.50	3.09	33.34	22.19	*
34005	56-74	62.07	30.61	57.71	51.33	*
34007	30-05	53.23	27.07	33.09	35.90	*
34011	12-04	17.09	9.58	16.19	18.08	*
35008	41-67	65.15	21.66	35.97	43.86	*
37001	45-00	61.88	10.94	36.62	45.97	*
37003	33-24	52.56	3.24	37.07	34.24	*
37008	36-22	49.44	10.00	34.86	44.00	*
37008	25-61	32.89	24.34	31.59	31.51	*

TABLE 6.1 OBSERVED AND ESTIMATED PARAMETER VALUES - Contd

CATCHMENT	OBSERVED PERCENTAGE RUNOFF			ESTIMATED PERCENTAGE RUNOFF			TIME TO PEAK(H)		
	MEAN	MAX	MIN	MEAN (FROM SOIL)	MEAN (FROM BF1)	ESTIMATED MEAN	MEAN	MAX	MIN
37331	23.92	33.06	12.31	44.12	-1.69	*	4.85	4.90	4.80
38003	1.10	1.53	0.87	7.25	39.77	*	3.67	5.75	2.25
38007	32.57	65.23	9.58	30.66	11.20	*	1.21	3.25	1.25
19004	1.14	2.62	0.75	5.27	10.55	*	1.21	3.25	1.25
39005	19.34	33.01	12.90	30.73	10.55	*	3.21	5.85	1.75
39007	23.15	36.56	12.97	34.63	30.83	*	12.60	16.00	10.10
39317	53.66	80.96	0.32	43.20	59.64	*	9.31	11.30	6.50
39022	30.45	55.07	25.28	33.30	21.63	*	20.54	32.20	13.50
39025	24.50	50.38	7.96	42.57	37.30	*	15.00	17.90	12.10
39026	34.81	60.02	20.36	44.71	46.64	*	20.34	25.60	14.10
39036	3.37	3.37	3.37	9.61	*	*	*	*	*
39052	27.46	39.63	6.40	42.12	43.10	*	5.53	8.60	3.90
39057	1.11	2.11	0.74	7.44	44.11	*	3.04	5.14	2.14
39813	47.94	65.55	34.26	47.64	4.30	*	6.39	8.50	4.19
39814	35.26	55.01	21.14	41.33	*	*	1.89	2.35	1.25
39820	48.55	71.24	32.66	48.69	*	*	6.55	7.10	3.61
39830	13.89	19.57	3.73	18.40	*	*	3.21	4.05	2.45
39831	14.97	27.25	3.83	10.70	*	*	1.48	1.90	1.25
40004	66.15	68.66	63.87	47.79	47.79	*	10.15	10.20	9.10
40006	22.43	36.86	7.22	24.39	27.63	*	7.19	8.60	5.40
40007	65.16	52.75	25.93	43.56	42.54	*	*	*	*
40008	36.52	54.95	25.64	31.51	36.96	*	*	*	*
40009	43.05	61.01	14.30	46.59	41.18	*	8.77	14.20	5.10
40010	49.03	70.86	10.92	41.18	52.44	*	15.64	22.90	11.60
41005	45.59	64.55	25.11	47.45	40.59	*	17.32	21.40	11.76
41006	59.32	80.93	15.15	45.97	43.76	*	12.85	16.50	10.50
41007	77.20	93.70	44.21	74.39	74.39	*	11.21	14.20	11.21
41015	1.17	2.75	0.59	10.72	4.80	*	4.10	5.30	2.75
41021	53.13	67.43	39.18	43.72	60.05	*	*	*	*
41022	40.30	63.57	42.74	37.79	47.78	*	7.24	12.00	4.00
41025	58.43	66.19	43.94	44.71	59.63	*	*	*	*
41029	49.39	58.76	35.36	48.37	49.04	*	8.53	11.30	6.30
41801	36.05	76.64	14.47	40.65	*	*	3.73	5.60	2.37
45002	34.56	61.13	8.58	34.90	39.65	*	7.69	8.00	6.25
45003	51.17	59.15	17.49	27.06	35.14	*	11.65	15.	

TABLE 6.1 OBSERVED AND ESTIMATED PARAMETER VALUES - Contd

CATCHMENT	PERCENTAGE RUNOFF						TIME TO PEAK(H)					
	OBSERVED			ESTIMATED			OBSERVED			ESTIMATED		
	MEAN	MAX	MIN	MEAN (FROM SOIL)	MEAN (FROM EFI)	MEAN	MAX	MIN	ESTIMATED MEAN			
56005	34.77	69.80	22.02	45.96	39.85	*	5.25	7.30	4.10	4.04		
56006	50.84	73.30	19.16	45.11	47.04	*	4.56	6.00	3.50	5.54		
56011	30.28	40.20	23.92	42.72	33.23	*	6.58	10.50	4.00	6.92		
57004	40.56	59.34	25.40	52.92	48.42	*	6.32	9.60	4.60	5.67		
57005	46.51	52.32	29.56	53.60	46.80	*	6.13	5.20	4.50	5.45		
57006	43.70	68.71	16.44	53.35	51.48	*	2.50	4.60	1.70	4.07		
58001	36.57	61.26	20.59	49.87	47.75	*	4.43	6.50	3.30	4.73		
59002	36.58	49.35	25.78	57.47	56.77	*	4.87	7.60	3.40	4.80		
59003	36.64	51.77	26.49	29.99	34.28	*	6.81	9.00	3.80	5.44		
58006	44.06	62.10	27.61	52.55	48.83	*	3.82	5.20	2.80	3.44		
58005	55.82	71.96	34.21	51.17	46.70	*	4.12	5.50	3.20	3.55		
58009	24.71	-	-	4.00	4.00	*	4.00	4.00	4.00	4.00		
59002	51.56	55.56	39.09	43.87	40.69	*	5.32	6.40	4.50	5.25		
60003	44.51	42.51	44.51	32.97	33.31	*	6.93	9.00	4.40	8.23		
60006	29.19	29.19	29.19	33.70	44.36	*	14.00	14.00	14.00	8.37		
61001	56.29	62.74	49.35	57.77	51.39	*	3.05	3.50	2.60	3.76		
61003	27.13	38.05	12.61	33.07	30.23	*	7.09	9.50	4.30	9.20		
62002	59.35	50.37	26.48	31.44	34.56	*	5.44	6.80	4.00	3.67		
64001	5.71	5.25	5.71	42.09	42.32	*	4.20	4.50	4.00	3.67		
65001	47.84	56.03	30.17	63.64	62.68	*	4.86	5.35	4.00	6.97		
66002	50.05	52.76	45.05	60.11	61.19	*	6.20	2.40	2.40	2.60		
66004	15.86	19.58	10.47	27.24	36.76	*	4.57	6.70	2.10	1.69		
66006	4.35	6.96	2.74	1.16	0.48	*	3.85	5.10	1.80	3.13		
68001	6.80	68.28	23.59	32.19	38.43	*	6.73	9.30	4.60	6.19		
70003	63.40	30.57	25.90	47.07	52.22	*	5.22	5.20	2.70	4.19		
70005	93.17	76.05	41.00	41.91	45.51	*	5.28	6.50	4.50	4.38		
67008	28.43	30.12	25.51	46.59	44.56	*	5.40	5.53	5.00	5.21		
68006	45.19	60.37	10.86	29.42	34.37	*	7.06	9.80	6.00	9.30		
68010	51.17	51.17	31.95	45.49	42.36	*	5.63	6.20	4.80	7.07		
68002	43.05	52.76	24.93	48.80	40.00	*	4.00	4.00	4.00	4.16		
69004	29.18	58.82	3.57	23.04	30.00	*	3.07	6.15	1.35	6.54		
69011	32.13	35.04	20.21	46.55	9.00	*	10.00	8.00	6.00	6.36		
69012	53.84	64.27	41.22	27.17	4.75	*	4.90	4.60	4.50	4.50		
69013	14.83	41.92	5.02	24.60	4.80	*	5.30	6.10	5.47	5.47		
69018	52.53	47.47	44.72	20.55	3.50	*	4.50	4.20	4.50	4.50		
69027	31.04	43.30	20.23	40.68	2.40	*	2.90	1.70	3.37	10.00		
69031	46.53	53.35	33.43	46.05	7.44	*	5.10	6.10	4.10	5.34		
69034	36.07	50.14	7.13	51.81	5.19	*	4.14	4.13	1.17	2.07		
69006	56.47	56.36	46.58	53.79	3.80	*	4.60	3.00	3.00	3.00		
71003	57.47	32.35	26.72	56.61	51.61	*	3.86	4.60	2.00	5.73		
71004	33.21	52.52	21.39	46.96	44.44	*	2.97	3.75	1.65	2.66		
71008	38.84	54.31	14.35	50.30	5.16	*	6.40	4.40	7.23	9.35		
71892	69.02	80.82	57.20	57.77	59.24	*	7.22	8.60	6.20	7.53		
71304	34.95	50.45	19.53	55.03	2.07	*	2.55	1.75	3.02	5.7		
72002	61.52	80.22	44.15	51.02	53.00	*	5.69	7.00	4.60	7.23		
72006	61.45	82.45	47.59	54.66	5.44	*	6.44	7.00	5.30	8.65		
72918	26.52	72.94	18.52	46.39	62.08	*	6.66	8.80	4.60	6.74		
72820	32.10	43.87	6.51	48.61	48.12	*	2.23	4.65	0.95	1.35		
73005	35.79	53.12	19.00	47.65	43.21	*	6.19	7.20	4.00	5.36		
73007	42.85	57.52	25.60	52.17	3.58	*	4.30	3.60	1.28	1.00		
73008	31.39	43.58	21.59	34.31	41.55	*	6.80	5.60	4.20	6.33		
73803	65.25	77.14	51.22	37.73	9.90	*	9.90	9.90	4.40	5.0		
73804	71.15	90.45	57.82	62.84	7.44	*	9.60	9.50	4.21	10		
74001	65.56	84.95	43.98	51.14	57.14	*	7.44	2.90	3.09	1.9		
75006	71.84	94.57	54.43	47.17	57.23	*	3.23	3.80	2.90	11.59		
75007	51.23	64.47	39.70	41.49	55.16	*	3.73	5.00	2.90	3.96		
76005	55.87	65.57	65.47	52.13	66.87	*	5.00	6.30	3.56	11		
76008	58.62	64.80	52.65	50.42	54.31	*	6.25	7.00	5.50	9.21		
76011	67.42	83.96	12.93	53.52	62.21	*	2.59	4.75	1.45	3.14		
76305	53.10	75.11	4.45	48.43	51.66	*	3.84	5.10	2.90	4.75		
77002	55.11	56.52	39.14	52.13	51.94	*	1.87	4.05	0.85	3.48		
83002	58.48	58.48	58.48	58.09	64.16	*	5.40	6.30	4.60	9.19		

5	10.4		50.6	20.75	71.61	37.01	56.3
6	10.0		34.9	25.45	44.22	32.82	116.7
7	9.0		39.2	27.76	47.58	35.43	106.4
8	8.0		45.7	29.81	64.65	44.52	80.6
9	-		67.2	32.88	73.00	38.05	63.0
10	5.3		45.3	29.14	71.01	45.44	75.1
11	10.8		42.1	29.34	60.59	41.67	81.3
12	7.0		44.1	30.46	78.35	49.66	75.8
13	-		14.4	3.54	73.34	11.18	23.5
14	-		37.9	20.20	217.91	52.01	61.2
15	-		51.8	33.91	89.32	59.56	72.6
<hr/>							
54016							
1	22.5	19.97	25.2	28.58	10.16	15.52	158.4
2	-		25.2	27.51	7.42	13.97	198.6
3	30.5		24.6	16.52	11.32	12.42	94.2
5	23.5		13.6	17.82	6.98	12.93	169.9
8	-		27.8	27.24	10.02	16.42	153.8
9	24.5		19.1	17.91	10.27	16.20	119.1
12	-		23.8	30.28	11.31	19.08	152.9
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67010							
1	3.3	3.53	63.7	53.44	11.46	10.65	99.7
2	2.8		46.3	54.83	12.61	16.96	122.6
3	4.4		46.9	53.75	11.87	11.63	123.8
4	3.8		46.1	56.42	11.16	14.44	137.8
5	1.7		45.7	57.77	18.02	17.24	129.6
6	3.8		60.0	58.06	11.74	9.40	106.9
7	2.7		46.8	56.71	13.60	17.47	125.0
8	-		62.6	61.28	12.30	17.34	105.5
9	2.7		59.5	56.16	14.52	15.27	100.4
11	-		58.7	59.04	15.01	14.15	120.8
13	-		45.4	55.95	10.53	12.87	154.5
14	2.6		64.5	55.83	11.29	11.20	87.0

19001 Almond at Craigie Hall

The model equations have performed remarkably well on this catchment. The event plotted in Figure 6.1(a) illustrates an event where the peak has been estimated very closely although there is an overestimation of runoff volume by 17%, an error partly explained by the shorter duration of the estimated response hydrograph and an overestimate of ANSF.

29001 Waithe Beck at Brigsley

With the exception of just one event all peaks have been grossly overestimated. Observed percentage runoffs are in the range 1.0 to 3.3% yet the catchment located mainly on type 1 soils is assigned a modelled SPR of 19%. Observed values of T_p are from 5.1 to 12.0 hours with the estimated value at the top of this range. Event 2 in Figure 6.1(b) shows the overestimation typical of events on this catchment. In trying to improve the estimate by using all the other events (ie 8 events) as available data an 'observed' value of SPR can

values. A similar procedure could be applied to the estimation of ANSF. Figure 6.1(b) illustrates the estimated hydrographs when these expedients have been adopted.

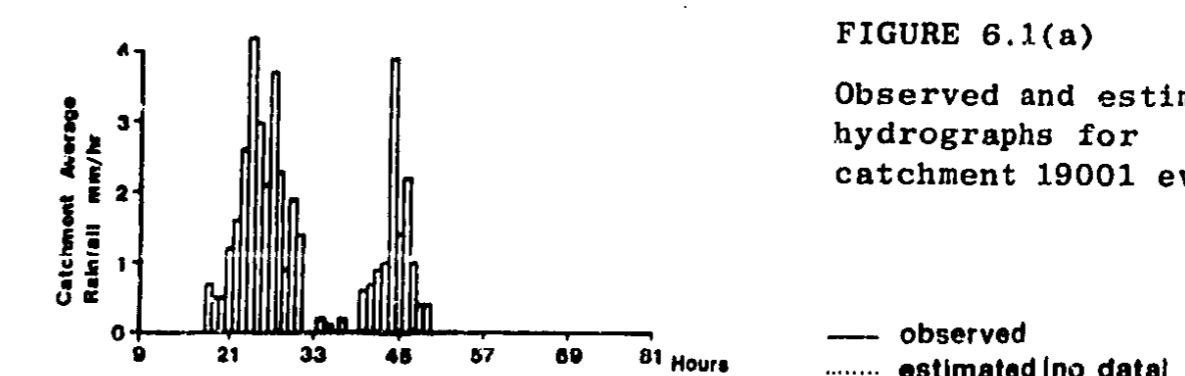


FIGURE 6.1(a)
Observed and estimated
hydrographs for
catchment 19001 event 8

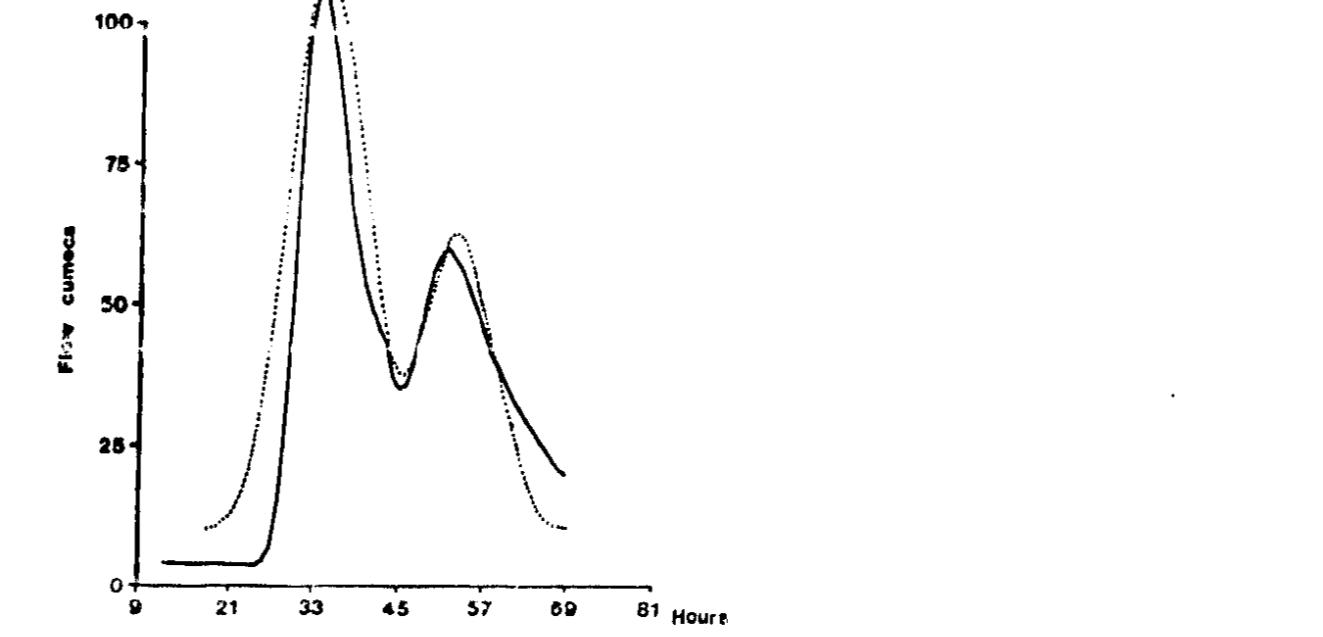
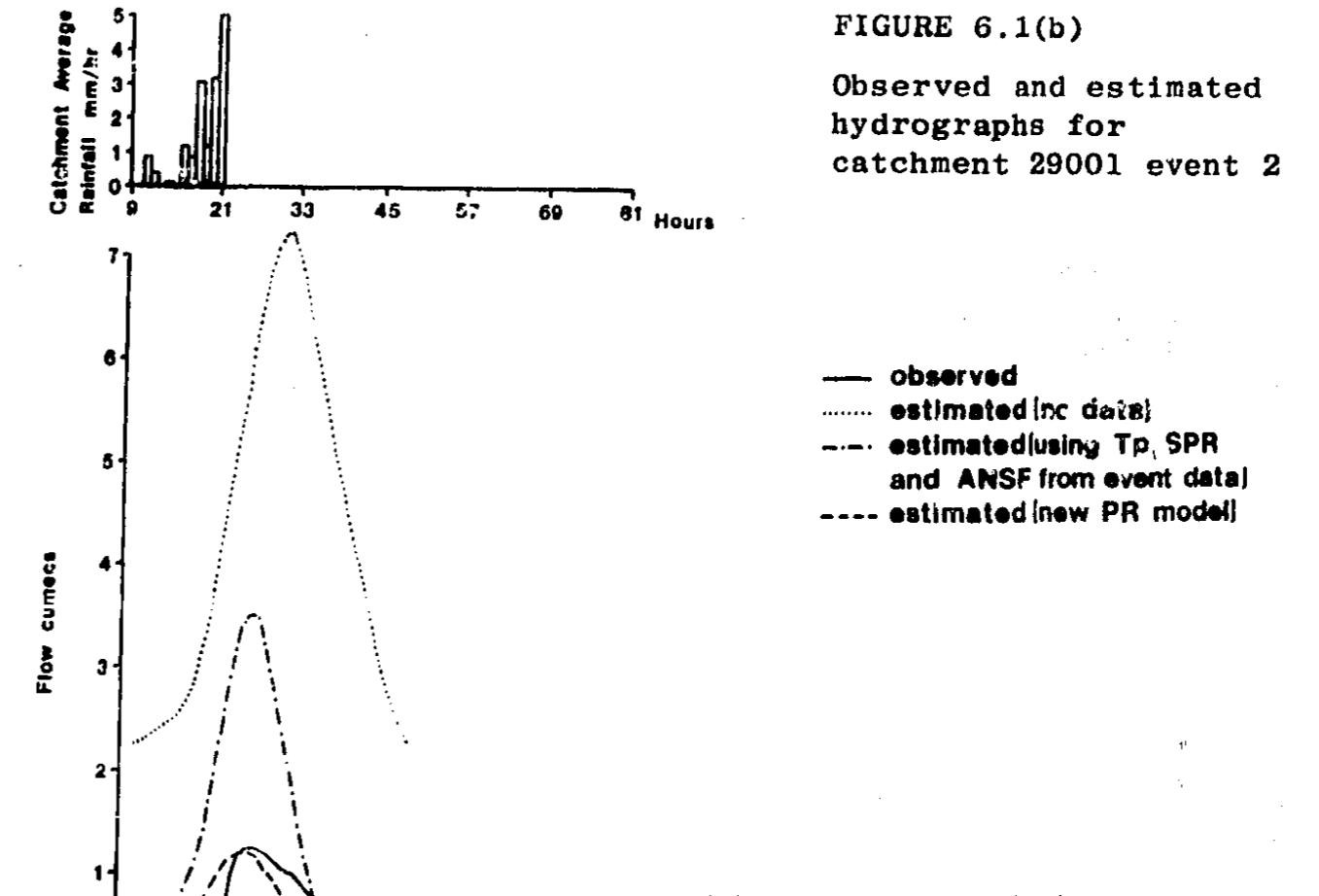


FIGURE 6.1(b)
Observed and estimated
hydrographs for
catchment 29001 event 2



39012 Hogsmill at Kingston-upon-Thames

On this catchment, with an urban fraction of about 0.5, peak flows are again overestimated. The illustrated event (Figure 6.1(c)) is one of the worst on the catchment. Refining the estimate of SPR using the other event data gives 19%, considerably less than the 34% from the soil map, and in good agreement with the value obtained from using BFI of 17%. The timing of the peak and duration of the response flow are good. Although catchments with a larger urban fraction are included in the calibration data set, the method is not recommended for application in such conditions.

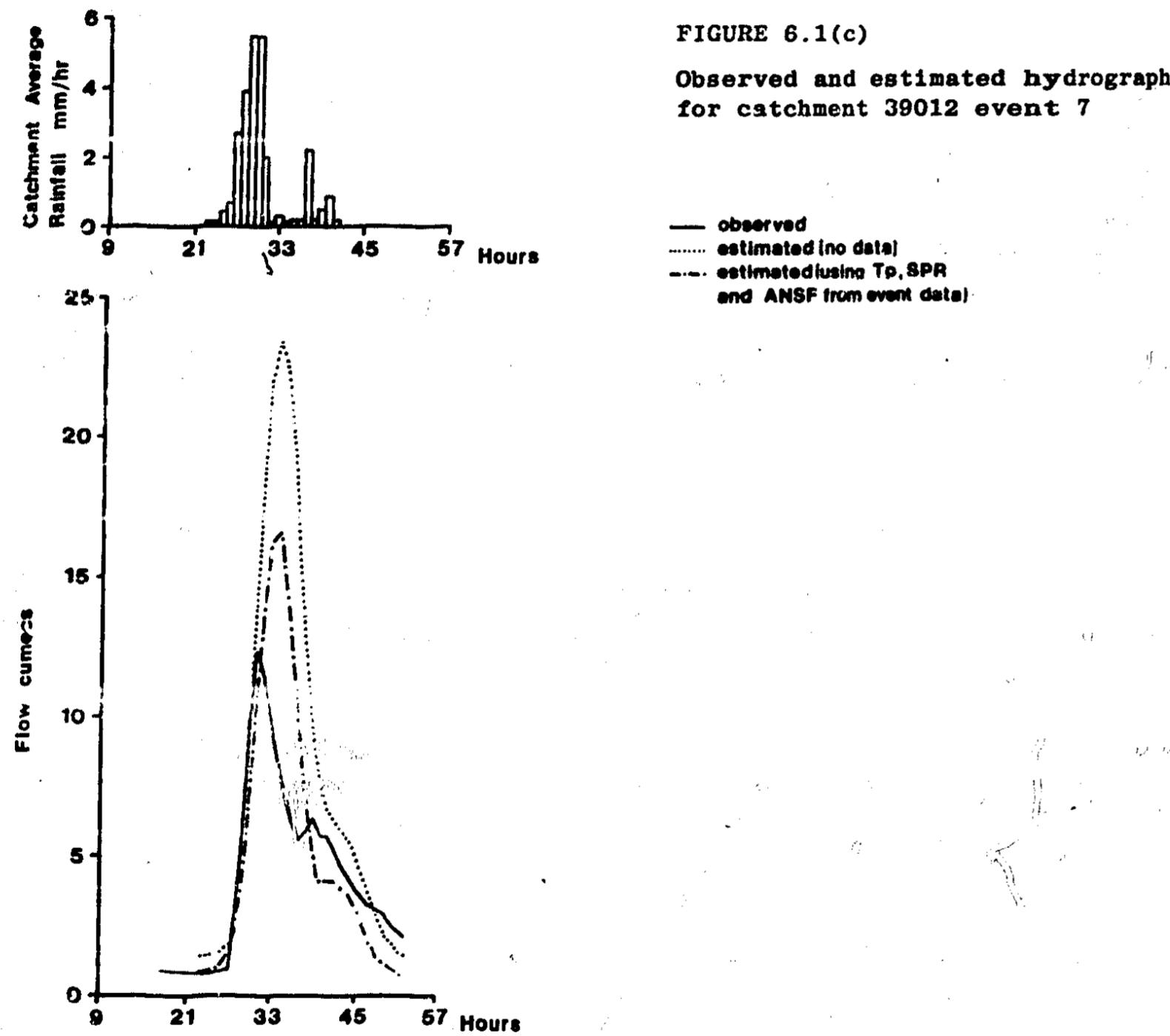
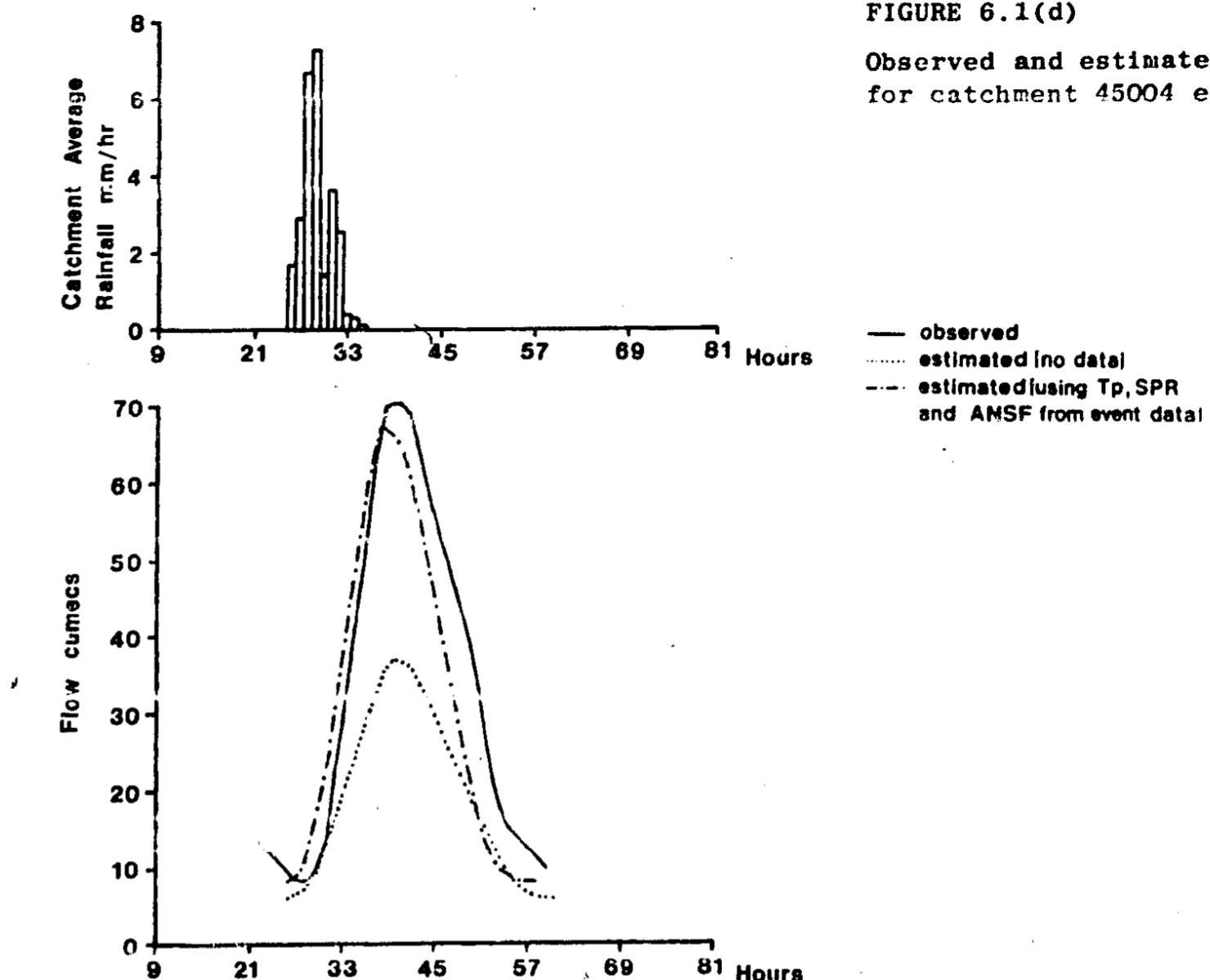


FIGURE 6.1(c)

Observed and estimated hydrographs
for catchment 39012 event 7

FIGURE 6.1(d)

Observed and estimated hydrographs
for catchment 45004 event 5

45004 Axe at Whitford

This is the only catchment in the test data set on which peaks are consistently underestimated, Figure 6.1(d) showing one of the poorer examples. Again, using local data, the SPR from soil of 27% can be replaced by 41% from event data or 39% using BFI and a dramatic improvement in the example event is seen.

54016 Roden at Rodington

The event shown in Figure 6.1(e) shows that the problems encountered on this catchment are rather different. Percentage runoff is estimated quite well (SPR 28% from soil, 31% from BFI and 27% using event data) but there is a considerable baseflow error and the response hydrograph shape is rather symmetrical and hence of shorter duration than that observed. With only three other events for which a unit hydrograph was derivable it is not possible in this case to replace the triangular unit hydrograph with one derived from local data. Boorman and Reed (1981) assess methods of deriving an average unit hydrograph suitable for use in this context. The observed AMSF's can be averaged to give a value to replace the one obtained from the equation. The effect this has on event 2 is shown in the figure.

67010 Gelyn at Cynefael

On this catchment the 'no data' equations have performed reasonably well with an equal number of events over and under estimated. In the example of Figure 6.1(f) the response flow is seen to start too early, to be too great early in the event, and to end more quickly than is observed. On this event the constant proportional loss model is not working well although the main peak is estimated reasonably.

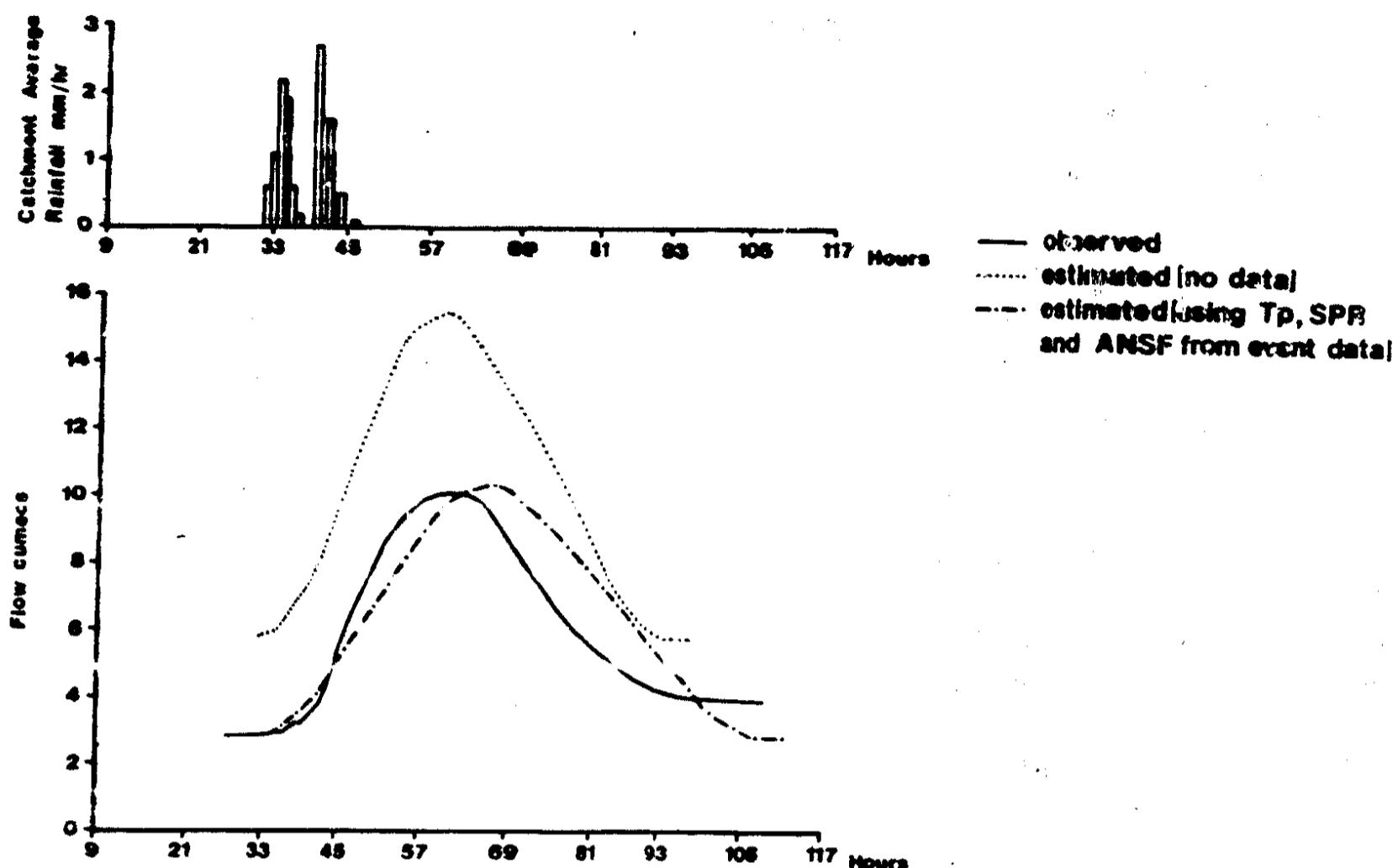


FIGURE 6.1(e) Observed and estimated hydrographs for catchment 54016 event 1

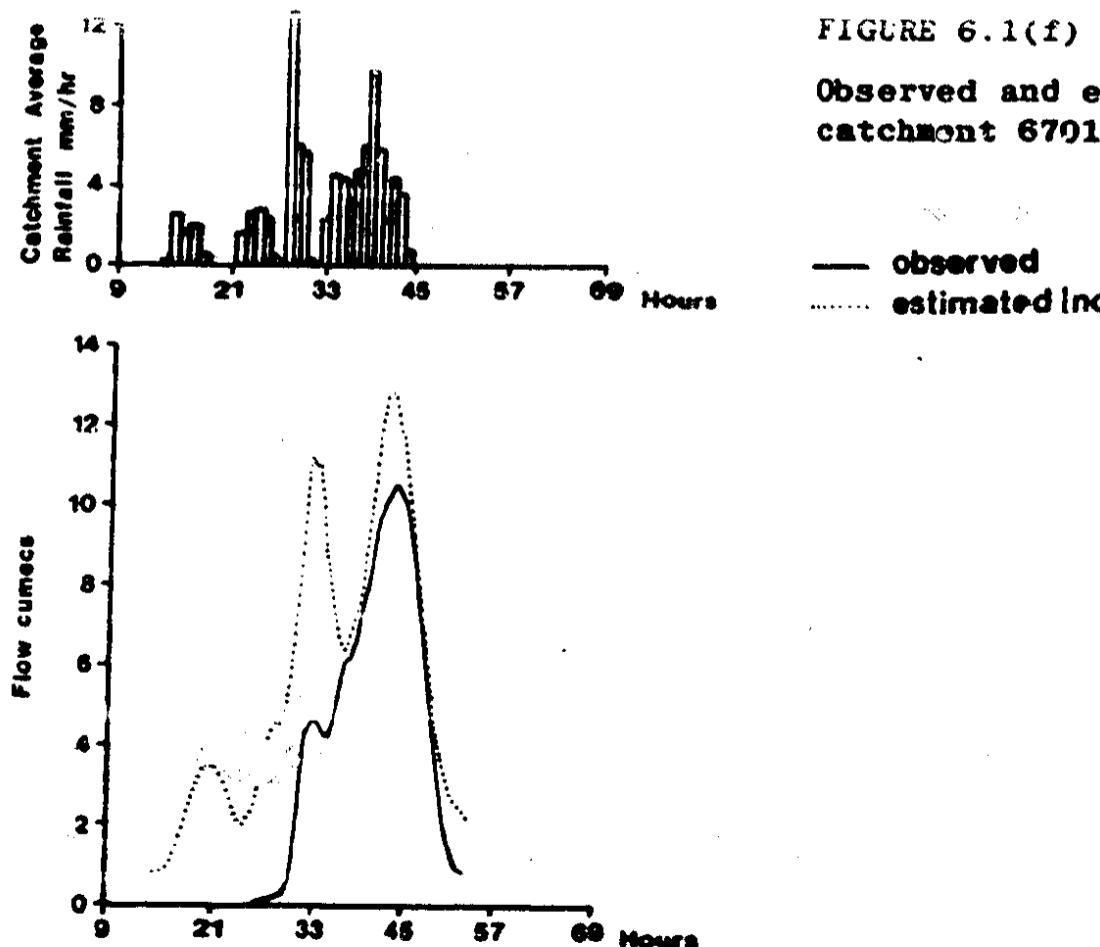


FIGURE 6.1(f)
Observed and estimated hydrographs for catchment 67910 event 13

Summary

The results presented in Table 6.2 represent a severe test of the flood estimation procedure as they are based solely on catchment characteristics. The use of local data to replace regression equation estimates of model parameters has been demonstrated to give a great improvement in the accuracy of the estimations on the six test catchments. A typical application in the United Kingdom will be able to draw on some local data either at the site of interest or from a nearby gauging station. Several types of data can prove beneficial in reducing errors; event data of the type collected for this study are obviously of most use as they can yield both timing and volume of runoff information. Daily mean flows assist in estimating response volumes. The applications given for the test data have revealed many of the problems that may be expected in genuine applications. However, while these problems are revealed in the examples, they will normally remain hidden. The hydrologist or engineer making a flood estimate must remember the uncertainties involved and take steps appropriate to the application to improve the estimate. Applications where errors are likely to be greater include catchments containing significant urbanisation or a large proportion of highly permeable soils.

7 CONCLUSION AND SUMMARY OF RECOMMENDED EQUATIONS

Since the publication of the FSR a considerable amount of new event data has been collected and analysed in line with the procedures of the FSR. The whole data set has been reviewed applying criteria that were more rigorous than those used originally. In addition to the event data review, catchment characteristics were also checked and updated where new maps defining relevant variables had been produced. With the aid of a new computer database system, the Flood Event Data Archive, these data, and key parameters derived from event analyses, are stored in a well organised and easily accessible form. The data set is generally larger than the one available to the Flood Studies team containing many more events from more catchments; it does however have slightly fewer events for which unit hydrographs could be derived.

In the derivation of new model parameter estimation equations consideration has been given to easing problems encountered in characteristic abstraction and application under extreme conditions. The new equations, presented in summary form in the following section, do not represent a major improvement in the standard of flood estimation procedures for ungauged sites. They do however offer a consolidated set of guidelines based on material previously published in a variety of sources (eg FSR, FSRR's). The relatively minor changes that have been introduced are recommended on the basis of an enhanced (and better validated) data set and because of the reviewed choice of dependent and independent variables to assist application. That the FSR parameter estimation equations are not found seriously deficient is a particularly pleasing, and, for all FSR users, a reassuring aspect of this review.

Looking to the future, the most needed enhancement in rainfall-runoff modelling for flood estimation is better definition of runoff volumes. A new soil map containing more classes and at a larger scale is on the horizon and promises the most significant improvement within the framework of the current design package.

SUMMARY OF RECOMMENDED MODEL PARAMETER ESTIMATION EQUATIONS

Summary of Recommended Model Parameter Estimation Equations

Unit Hydrograph Parameters

$$T_p(0) = 283 S1085^{-0.33} (1+URBAN)^{-2.2} SAAR^{-0.54} MSL^{0.23} \text{ (hours)}$$

$$T_p(T) = T_p(0) + T/2 \text{ (hours)}$$

$$Q_p = 220/T_p \text{ (cumecs per 100 sq km)}$$

Percentage Runoff

$$PR_{rural} = SPR + DPR_{CWI} + DPR_{RAIN}(\%)$$

where

$$SPR = 10S1 + 30S2 + 37S3 + 47S4 + 53S5 \text{ (%)}$$

$$DPR_{CWI} = 0.25 \text{ (CWI-125) (%)}$$

$$\begin{aligned} DPR_{RAIN} &= 0.45 (P-40)^{0.7} \text{ (%) for } P > 40 \text{ mm} \\ &= 0 \text{ for } P < 40 \text{ mm} \end{aligned}$$

$$PR = PR_{rural} (1.0 - 0.3 URBAN) + 21.0 URBAN \text{ (%)}$$

If mean daily flow data available to calculate BFI

$$SPR = 72.0 - 66.5 BFI \text{ (%)}$$

Base Flow

$$\Delta NSF = (33(CWI - 125) + 3.0 SAAR + 5.5) \times 10^{-5} \text{ (cumecs/sq km)}$$

Acknowledgement

This work was carried out as part of a research project funded by the River Flood Protection Commission of the Ministry of Agriculture, Fisheries and Food.

The collection, analysis and validation of the data were carried out by a great many members, both past and present, of the catchment response group of the Applied Hydrology section of the Institute of Hydrology. Dr M J Lowing and Dr D W Reed have provided invaluable assistance in the interpretation of analyses and preparation of the report.

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APPENDIX A

CATCHMENT AND EVENT DATA

TABLE A.1 CATCHMENT CHARACTERISTICS FOR BASINS USED IN THE STUDY

NUMBER	NAME	AREA	MSL	S1025	S442	SOILT	SOIL2	SOIL3	SOIL4	SOIL5	URBAN	LAKE	BFI
19001	ALMOND AT CRAIGIE HALL	369.00	42.00	5- 1	914	0.	0.	0.	0.50	0.20	0.11	0.04	0.355
19002	ALMOND AT ALMOND WEIR	43.50	17.90	5- 0.5	1052	0.	0.	0.	1.00	0.	0.07	0.	0.346
19005	ALMOND AT ALMONDELL	229.00	25.20	5- 2.7	968	0.	0.	0.	0.54	0.36	0.10	0.07	0.343
20001	TYNE AT EAST LINTON	307.00	31.90	5- 0.8	759	0.05	0.	0.22	0.72	0.22	0.02	0.02	0.543
21030	MEGGET AT HENDERLAND	56.20	12.40	10- 5.5	1524	0.	0.	0.51	0.	0.49	0.	0.	0.405
23002	DERWENT AT EDDY'S BRIDGE	119.30	21.40	11- 3.0	932	0.	0.	0.	0.20	0.30	0.	0.	0.424
23005	NORTH TYNE AT TARTSET	245.00	36.30	10- 3.5	1255	0.	0.	0.	1.00	0.	0.	0.	0.298
23006	SOUTH TYNE AT FEATHERSTONE	321.90	32.70	10- 5.7	1557	0.	0.	0.	0.	1.00	0.	0.	0.233
23010	TARTSET BURN AT GREENHAUGH	96.00	15.25	10- 1.5	1035	0.	0.	0.	0.	1.00	0.	0.	0.255
24003	WEAR AT STANHOPE	171.90	21.81	14- 5.1	1318	0.	0.	0.	0.02	0.25	0.	0.10	0.350
24005	BROWNEY AT BURN HALL	178.45	31.71	6- 3.0	770	0.	0.	0.	0.99	0.01	0.05	0.	0.519
24007	BROWNEY AT LANCASTER	44.50	11.90	14- 5.0	768	0.	0.	0.	0.93	0.07	0.	0.	0.405
25003	TROUTBECK AT MOOR HOUSE	11.60	5.07	35- 7.2	2182	0.	0.	0.	1.00	0.	0.	0.	2.456
25004	SKERNE AT SOUTH PARK	250.19	44.20	11- 3.2	671	0.	0.	0.	1.00	0.	0.	0.	0.752
25006	GRETA AT RUTHERFORD BRIDGE	86.10	17.59	11- 5.8	1179	0.	0.	0.	0.	1.00	0.	0.	0.533
25011	LANGDON BECK AT LANGDON	13.00	5.21	25- 6.4	1521	0.	0.	0.	1.00	0.	0.	0.	0.215
25012	HARWOOD BECK AT HARWOOD	26.10	6.48	25- 6.4	1440	0.	0.	0.	1.00	0.	0.	0.	0.211
26010	STEELE WEIR AT MOOR HOUSE	0.04	0.16	43- 7.8	2182	0.	0.	0.	1.00	0.	0.	0.	0.
27001	NIDDA AT MUNSINGORE WEIR	484.30	34.64	2- 5.4	993	0.01	0.	0.	0.69	0.30	0.02	0.25	0.506
27010	HODGE BECK AT BRANDSDEALE WEIR	18.90	8.41	14- 8.6	1057	0.	0.	0.	0.40	0.60	0.	0.	0.459
27026	ROTHER AT WHITTINGTON	165.00	15.53	3- 5.3	800	0.03	0.	0.76	0.75	0.06	0.07	0.07	0.454
27027	WARFE AT ILKLEY	443.00	55.10	6- 4.5	1382	0.	0.	0.	0.13	0.88	0.	0.09	0.378
27031	COLNE AT COLNE BRIDGE	245.00	23.65	9- 2.7	1107	0.	0.	0.	0.22	0.31	0.11	0.	0.397
27034	URE AT KILGRAN ROTDFE	512.20	25.12	4- 5.1	1429	0.	0.	0.	0.42	0.55	0.01	0.01	0.345
27035	AIRE AT KILDWICK BRIDGE	282.30	71.70	2- 1.7	1102	0.	0.	0.	0.53	0.47	0.02	0.13	0.426
27051	CRIMPLE BECK AT SURY BRIDGE	3.10	5.75	25- 5.1	531	0.	0.	0.	1.00	0.	0.	0.	0.335
28016	RYTON AT SERBY PARK	237.11	35.60	2- 4.7	631	0.84	0.	0.	0.15	0.	0.04	0.	0.692
28023	WYE AT ASHFORD	154.00	26.41	10- 0.4	1200	0.20	0.	0.	0.	0.20	0.02	0.	0.735
28026	ANKER AT POLESWORTH	368.00	34.10	1- 3.0	697	0.	0.	0.	0.02	0.98	0.	0.07	0.495
28033	DOVE AT HOLLINSCLough	8.05	3.90	33- 3.7	1392	0.	0.	0.	0.	1.00	0.	0.	0.447
28041	HAMPS AT WATERHouses	35.13	14.42	11- 3.1	1064	0.	0.	0.	0.	1.00	0.	0.	0.349
28070	BUCKAGE BROOK AT BURDGE	7.10	5.24	35- 1.5	985	0.	0.10	0.	0.	0.90	0.	0.	0.397
29001	WATHE BECK AT BRIGSLEY	108.30	20.17	3- 3.3	698	0.81	0.	0.	0.19	0.	0.	0.	0.864
29004	ANCHOLME AT BISHOPSBRIDGE	54.65	12.11	1- 9.8	630	0.51	0.08	0.	0.42	0.	0.	0.	0.460
30001	WITHAM AT CLAYPOLE MILL	298.00	45.92	2- 1.3	625	0.41	0.04	0.	0.55	0.	0.02	0.	0.630
30004	PARTNEY LYNN AT PARTNEY MILL	61.64	15.14	3- 2.6	673	0.	0.	1.00	0.	0.	0.	0.	0.770
31005	WELLAND AT TIXOVER	417.00	55.36	1- 4.4	643	0.02	0.	0.	0.98	0.	0.01	0.15	*
31010	CHATER AT FOSTERS BRIDGE	68.90	21.46	5- 2.0	641	0.06	0.	0.	0.94	0.	0.	0.02	0.497
31021	WELLAND AT ASHLEY	250.70	28.98	2- 5.0	653	0.	0.	0.	1.00	0.	0.02	0.	0.443
31023	WEST GLEN AT EASTON WOOD	4.40	2.80	14- 6.0	527	0.	0.	0.	1.00	0.	0.	0.	0.165
32801	FLORE EXPERIMENTAL STATION	6.81	3.26	5- 5.9	654	0.	0.	0.	0.65	0.	0.02	0.	*
33014	LARK AT TEMPLE WEIR	272.00	29.37	2- 2.4	622	0.55	0.	0.	0.	0.03	0.	0.	0.771
33015	GUZEL AT WILFEN WEIR	277.00	29.10	2- 1.7	555	0.12	0.	0.20	0.03	0.	0.	0.	0.547
33029	STRINGSIDE AT WHITE BRIDGE	98.90	7.04	1- 6.3	637	0.75	0.25	0.	0.	0.	0.	0.	0.931
33045	WHITTLE AT QUIDDENHAM	28.30	7.82	3- 2.6	649	0.	0.	1.00	0.	0.	0.	0.	0.694
33080	BURY BROOK AT BURY WEIR	65.30	10.00	1- 6.5	554	0.	0.	0.	0.85	0.15	0.	0.	0.299
34003	BURE AT INGWORTH	165.00	22.36	2- 3.5	639	1.00	0.	0.	0.	0.	0.	0.	0.329
34005	TUD AT COSTESSEY PARK	73.30	22.70	1- 7.3	680	0.08	0.	0.	0.92	0.	0.	0.	0.660
34007	DOVE AT OAKLEY PARK	134.00	16.53	1- 4.8	600	0.	0.	1.00	0.	0.	0.	0.	0.454
34011	WENSUM AT PARENHAM	127.10	17.69	1- 3.2	705	0.80	0.	0.	0.20	0.	0.	0.01	0.321
35006	GIPPING AT STOWMARKET	129.00	14.51	3- 4.1	515	0.	0.10	0.	0.92	0.	0.	0.	0.339
36008	STOUR AT WEST MILL	224.00	37.90	1- 7.1	617	0.	0.07	0.93	0.	0.	0.	0.	0.370
37001	RODING AT REDBRIDGE	303.00	62.60	1- 2.2	655	0.	0.	0.	0.80	0.20	0.	0.10	0.359
37003	TER AT CRABBS BRIDGE	77.80	24.83	2- 4.0	587	0.	0.36	0.54	0.	0.	0.	0.	0.526
37007	WID AT WRITTLE	136.00	26.90	1- 3.5	620	0.	0.03	0.90	0.07	0.	0.13	0.	0.186
37008	CHELMER AT SPRINGFIELD	100.00	43.53	1- 3.5	596	0.	0.10	0.90	0.	0.	0.03	0.	0.542
37331	CROUCH AT WICKFORD	71.90	12.40	2- 5.0	571	0.	0.	0.10	0.90	0.	0.25	0.	*
38001	MIRRAM AT PANSH												

NUMBER	NAME	AREA	MSL	S10RS	SAAR	SOIL1	SOIL2	SOIL3	SOIL4	SOIL5	URBAN	LAKE	BFI	NUMBER	NAME	AREA	MSL	S10RS	SAAR	SOIL1	SOIL2	SOIL3	SOIL4	SOIL5	URBAN	Lake	BFI				
45004	AXE AT WHITFORD	255.50	33.60	3.55	1009	0.50	0.	0.15	0.35	0.	0.01	0.	0.492	74001	DUDDON AT DUDDON HALL	25.66	19.44	13.01	2187	0.	0.27	0.	0.	0.73	0.	0.05	0.249				
45804	BARBLE AT BUSHFORTH	129.00	35.80	6.90	1655	0.	0.65	0.	0.	0.35	0.	0.	*	75006	NEWLANDS BECK AT BRAITHWAITE	33.70	10.54	21.01	2456	0.	0.47	0.	0.	0.53	0.	0.	0.297				
45805	EKE AT PIKTON	147.53	33.76	8.30	1467	0.	0.20	0.	0.	0.10	0.	0.	*	75007	GLENDERMACHIN AT THRELKELD	64.50	15.87	20.17	1900	0.	0.51	0.	0.14	0.32	0.	0.	0.293				
66003	DART AT AUSTINS BRIDGE	248.00	35.20	16.50	1921	0.26	0.24	0.	0.	0.50	0.	0.	0.527	76005	EDEN AT TEMPLE SOWERBY	616.40	56.70	4.05	1219	0.	0.	0.	0.30	0.70	0.	0.	0.322				
66005	EAST DART AT BELLEVILLE BRIDGE	21.30	11.50	22.60	2103	0.	0.	0.	0.	1.00	0.	0.	0.385	76008	ZIRTHING AT GREENHOLME	334.50	55.27	5.82	1049	0.	0.10	0.	0.15	0.	0.75	0.	0.01	0.328			
66802	SWINCOMBE AT SWANCOMBE INTAKE	14.20	3.62	25.90	1966	0.	0.	0.	0.	1.00	0.	0.	0.365	76011	COALBURN AT COALBURN	1.52	2.13	21.95	1143	0.	0.	0.	0.	1.00	0.	0.	0.155				
67007	YEALM AT PUSLINGHAM	5.93	2.83	75.90	2146	0.	0.	0.	0.	1.00	0.	0.	*	76014	EDEN AT KIRKBY STEPHEN	69.49	20.77	14.47	1439	0.	0.	0.	0.16	0.34	0.	0.	0.252				
67008	THRUSHIEL AT TINMAY	54.90	16.60	17.80	1445	0.14	0.50	0.	0.	0.36	0.	0.	0.531	76005	FORCE BECK AT SHAP	4.10	3.10	12.22	1610	0.	0.	0.	0.	1.00	0.	0.	0.273				
67009	PLYM AT CARN WOOD	112.70	18.60	6.81	1259	0.	0.33	0.	0.	0.67	0.	0.	0.355	77002	ESK AT CANONBIE	494.70	53.40	3.69	1550	0.	0.	0.	0.34	0.	0.67	0.	0.04	0.364			
67004	WARLEGGAN AT TRENGOFFEE	79.20	18.30	19.30	1664	0.10	0.40	0.	0.	0.50	0.	0.03	0.28	78002	GARROCK AT DALRY	32.50	17.31	21.77	1742	0.	0.	0.	0.55	0.	0.55	0.	0.02	0.215			
68005	KENWICH AT TRURO	25.33	19.00	17.45	1533	0.	0.25	0.	0.	0.75	0.	0.	0.720	94002	CALDER AT MUIRSHIEL	12.40	7.30	25.78	2329	0.	0.	0.	0.	1.00	0.	0.	0.64				
68009	ST NEOT AT CRAIGSMILL WOOD	22.70	12.17	17.67	1539	0.	0.20	0.	0.	0.30	0.	0.	0.395	84008	ROTTEN CALDER AT RED LEES	51.30	18.90	13.45	1175	0.	0.	0.	0.14	0.75	0.12	0.	0.315				
69003	DE LANK AT DE LANK	21.70	6.70	12.80	1681	0.	0.	0.	0.	1.00	0.	0.	0.485	94012	WHITE CARY WATER AT HAWKHEAD	227.20	51.16	6.52	1266	0.	0.	0.	0.32	0.50	0.19	0.	0.12	0.358			
51002	MORNER WATER AT WEST LUCCOME	20.80	10.73	34.96	1400	0.	0.90	0.	0.	0.10	0.	0.	0.559	34022	DUNEATON AT MAIDENDOTS	110.30	25.90	4.97	1471	0.	0.	0.	0.48	0.	0.52	0.	0.450				
52004	ISLE AT ASHFORD MILL	93.10	14.30	5.10	904	0.	0.09	0.	0.	0.20	0.	0.	0.570	45002	ENDRICK WATER AT GAIDREW	219.90	35.44	9.42	1478	0.	0.10	0.	0.15	0.00	0.15	0.	0.01	0.328			
52105	TEO AT PEN MILL	213.10	10.70	5.60	993	0.18	0.47	0.	0.	0.35	0.	0.	0.570	STOP	OK																
52010	BAUE AT LOVINGTON	135.20	20.35	4.65	909	0.	0.07	0.	0.02	0.63	0.	0.	0.471																		
52016	CURRY POOL AT CURRY FARM	15.70	7.15	20.75	918	0.	0.44	0.	0.	0.56	0.	0.	0.743																		
52020	GALLICA BROOK AT GYNE INTRINSICK	16.40	6.64	13.68	980	0.	0.	0.	0.40	0.40	0.	0.	0.245																		
53005	HIDE BROOK AT MIDFORD	147.40	24.60	3.00	998	0.60	0.	0.	0.10	0.30	0.	0.	0.606																		
53007	FROME AT TELLISFORD	261.60	27.70	2.30	933	0.27	0.03	0.	0.35	0.35	0.	0.	0.536																		
53008	AVON AT GT SOMERFORD	303.00	29.70	2.56	838	0.37	0.	0.	0.38	0.25	0.	0.	0.565																		
53009	WELLOW BROOK AT WELLOW	72.60	16.13	4.15	1925	0.57	0.	0.	0.43	0.	0.	0.	0.606																		
54004	SOKE AT STONEFIELD	245.50	22.00	4.05	907	0.	0.	0.	0.	0.	0.	0.	*																		
54010	STOUR AT RIDDERMINSTER	184.00	35.55	3.07	721	0.	0.14	0.	0.08	0.40	0.	0.	0.730																		
54011	STOUR AT ASCOT PARK	119.30	38.95	2.90	711	0.	0.10	0.	0.40	0.50	0.	0.	0.539																		
54012	SALWARPE AT HARFORD HILL	134.00	26.91	4.35	691	0.	0.42	0.	0.	0.01	0.57	0.	0.	0.655																	
54016	RODEN AT RODDINGTON	259.00	40.20	0.92	721	0.	0.53	0.	0.03	0.	0.	0.	0.507																		
54019	AVON AT STARDETTON	147.00	56.70	1.60	676	0.	0.30	0.	0.	0.70	0.	0.	0.500																		
54020	FERRY AT YEATON	180.50	31.57	2.56	792	0.	0.14	0.	0.07	0.76	0.	0.	0.660																		
54022	SEVERN AT PLUMTHORN WEIR	2.93	6.60	17.70	2449	0.	0.	0.	0.	0.	1.00	0.	0.	0.690																	
54034	DOUBLES BROOK AT DOUBLES	40.90	1.30	9.78	951	0.	0.20	0.	0.	0.50	0.	0.	*	19001	7	13	8.66	41.6	20	14.9	0.4	6.34	1.5	4.0	125	23.5	56.5	22.0	7.8	10.0	
54036	TAHLWYTH AT PLATTFELD	2.92	1.45	12.07	2552	0.	0.	0.	0.	1.00	0.	0.	*	19001	8	1	11	67	39.6	32	105.29	6.5	7.79	0.	0.	125	18.0	45.3	25.5	6.0	7.4
55008	WYF																														

1-HOUR UNIT HYDROGRAPH																		
CATCH NO	EV NO	DATE OF STORM	TOTAL DURN (MM)	PEAK FLOW (CUMECS)	LAG (H)	ANSF (CUMECS)	SMD (MM)	APIS (MM)	CWI (MM)	RUNOFF PER 100 SQ KMS (MM)	TIME TO AT 0.5 PEAK (H)	TIME BASE (H)	1-HOUR UNIT HYDROGRAPH					
24003	1	9 11 63	26.0	16	70.89	4.2	4.80	0.	3.3	128	13.9	51.8	*	*	*	*	*	*
24003	8	25 9 65	42.9	15	72.81	7.6	2.06	1.7	1.0	124	18.1	42.2	*	*	*	*	*	*
24003	11	2 10 66	48.4	34	121.03	5.6	1.85	3.7	3.3	124	20.7	42.8	*	*	*	*	*	*
24003	16	4 9 67	34.9	35	64.67	5.1	3.86	0.	16.1	139	15.7	45.1	*	*	*	*	*	*
24003	17	6 10 67	25.5	18	76.80	6.2	3.30	0.6	4.1	128	15.4	60.6	*	*	*	*	*	*
24005	1	8 12 56	33.6	11	34.99	3.0	4.24	3.2	0.3	122	9.8	29.2	21.0	7.5	10.7	31.6		
24005	2	27 8 56	27.7	10	31.02	10.0	1.87	0.8	1.8	125	7.3	26.4	23.1	9.8	9.7	28.7		
24005	5	13 3 64	36.5	31	27.30	3.5	2.82	0.	3.4	128	8.0	21.9	23.0	5.0	9.5	29.3		
24005	7	23 3 64	30.1	30	44.48	9.1	4.73	0.5	1.6	126	11.0	36.6	29.0	7.5	5.2	21.9		
24005	8	28 9 65	14.9	8	18.80	8.6	1.80	25.3	3.3	103	3.3	21.9	27.0	8.5	8.5	24.2		
24005	9	30 9 65	14.0	12	20.24	2.7	4.26	0.	10.5	135	3.4	22.5	26.1	9.0	8.0	23.6		
24005	10	17 11 65	43.0	46	48.39	6.5	11.43	0.	10.0	134	15.8	36.7	*	*	*	*	*	*
24005	12	9 4 66	21.6	12	42.39	7.7	5.10	1.0	7.7	131	9.5	46.0	31.0	6.0	7.1	21.7		
24005	13	12 8 66	35.4	33	29.76	11.0	1.34	38.8	5.6	91	8.2	23.0	23.0	7.0	8.5	31.3		
24005	14	2 10 66	39.7	37	21.81	13.5	1.18	17.4	2.9	110	8.6	21.5	*	*	*	*	*	*
24005	15	8 6 67	42.8	21	28.38	10.1	0.96	62.0	1.5	64	5.7	13.4	35.6	4.7	5.9	19.4		
24005	16	16 10 67	42.5	16	40.67	8.1	1.34	0.6	1.7	126	11.8	27.7	*	*	*	*	*	*
24005	17	1 11 67	16.0	9	19.10	9.5	1.99	1.2	0.7	124	3.8	23.9	30.0	8.5	8.5	20.1		
24005	18	4 11 67	56.2	22	58.48	9.0	4.29	0.	2.5	127	23.1	61.1	21.0	7.0	15.0	34.0		
24005	19	4 11 67	44.7	22	58.48	9.0	4.29	0.	2.5	127	23.1	61.1	21.0	7.0	15.0	34.0		
24005	21	11 1 69	18.6	17	22.74	11.1	2.49	0.2	0.9	125	5.7	30.7	25.0	6.5	8.8	26.9		
24005	22	2 5 69	19.4	18	26.49	7.7	2.29	8.1	2.4	119	4.3	22.2	32.1	7.7	6.9	20.8		
24005	23	6 5 69	15.5	12	30.78	5.5	4.98	0.6	5.4	129	5.3	34.0	32.8	6.5	6.9	20.1		
24007	2	30 10 68	73.5	94	12.46	11.1	0.60	0.	2.1	127	42.6	57.9	*	*	*	*	*	*
24007	5	11 1 69	18.0	17	8.03	7.3	1.25	0.2	1.0	125	7.0	44.0	32.2	5.7	7.3	10.9		
24007	7	5 5 69	16.2	17	8.27	5.7	0.71	5.8	2.2	121	4.3	26.2	41.0	4.8	5.5	16.1		
24007	8	23 6 69	21.0	21	8.86	6.7	1.48	4.3	9.2	120	5.0	39.5	35.3	4.5	6.7	18.1		
24007	9	17 9 69	20.2	11	8.36	5.0	0.49	11.8	3.5	116	5.7	26.3	35.0	4.0	6.3	19.2		
24007	11	20 1 71	16.7	12	7.24	5.8	0.70	2.7	1.3	123	5.7	33.9	33.0	7.3	7.5	18.7		
24007	13	22 4 71	51.9	33	13.66	12.0	0.66	21.3	0.0	103	18.8	36.2	24.2	8.2	8.0	30.0		
25003	2	21 11 63	35.9	18	12.20	3.0	0.33	0.	11.0	135	20.5	52.7	47.7	2.0	4.4	14.5		
25003	3	8 8 66	36.5	8	16.33	2.8	0.46	83.3	12.1	122	18.3	50.1	49.4	4.0	5.0	12.5		
25003	5	15 9 65	38.6	14	13.52	2.4	0.27	3.3	0.7	122	23.0	39.7	47.2	4.0	5.0	13.6		
25003	11	2 7 66	29.7	8	24.11	2.5	0.92	0.	11.6	136	21.9	73.6	64.7	4.0	3.8	9.6		
25003	13	20 9 68	40.5	11	13.74	3.5	0.67	0.	21.3	140	32.6	80.5	49.6	2.9	5.4	11.6		
25004	3	21 1 59	28.5	32	26.85	4.1	10.28	0.	9.3	134	4.9	17.0	*	*	*	*	*	*
25004	5	14 3 64	26.7	28	24.07	6.2	3.91	2.0	4.2	127	6.9	25.7	*	*	*	*	*	*
25004	8	9 4 66	23.8	13	29.67	12.0	6.86	0.6	7.7	132	7.0	29.6	13.9	10.4	16.8	46.4		
25004	9	16 10 67	43.5	18	32.23	11.1	3.86	1.0	5.3	129	10.9	25.0	*	*	*	*	*	*
25004	10	4 6 11 67	50.1	23	35.50	16.2	3.88	0.2	2.9	127	20.3	40.6	*	*	*	*	*	*
25006	7	22 7 71	50.1	30	50.13	4.4	0.94	21.0	0.8	104	4.32	53.9	*	*	*	*	*	*
25006	4	20 11 71	24.3	9	29.11	4.4	1.63	0.2	1.9	126	9.8	40.3	*	*	*	*	*	*
25011	3	16 3 72	35.4	5	15.16	1.5	0.66											

1-HOUR UNIT HYDROGRAPH											
CATCH NO	EV NO	DATE OF STORM	TOTAL DURN (H)	PEAK FLOW (CUMECS)	LAG (H)	ANSF	SMD (MM)	APIS (MM)	CWI (MM)	RUNOFF PER 100 SQ KMS.	TO AT 0.5 TIME BASE
29001	11	6 10 74	53.1	35	1.28	12.5	0.14	84.4	4.5	0.9	1.8
29004	2	2 6 69	26.9	16	5.38	7.5	0.68	9.4	2.2	117	4.6 17.1 20.2 5.8 12.6 29.8
29004	3	27 7 69	50.8	11	7.60	9.7	0.06	94.0	1.1	32	5.9 11.7 22.9 8.0 10.9 26.8
29004	4	16 11 69	31.6	11	8.62	9.9	0.69	42.2	2.1	54	8.2 25.8 16.9 9.3 11.3 35.2
29004	5	12 4 70	33.1	17	7.43	11.2	0.55	0.7	0.4	126	8.9 26.9 17.1 10.2 11.6 41.8
29004	6	8 3 72	11.2	12	6.05	5.7	1.74	0.	8.9	133	4.1 36.6 *
29004	7	15 2 73	58.6	20	9.23	13.7	0.60	45.0	1.4	81	12.6 21.4 14.1 9.4 15.8 47.3
29004	8	10 74	53.1	25	6.72	13.6	0.69	79.0	6.9	50	16.9 31.8 *
29004	9	18 4 75	10.4	14	6.66	7.2	0.95	0.	3.2	128	5.3 27.1 22.0 6.0 10.9 28.7
30001	1	29 10 60	17.6	14	16.82	22.7	4.78	0.	2.9	127	5.3 30.1 9.0 21.2 24.3 75.0
30001	2	3 12 60	35.2	20	23.37	32.9	4.99	0.	0.2	125	13.2 37.4 9.0 19.7 25.5 72.6
30001	3	18 12 60	35.9	47	23.87	19.0	4.57	0.	0.1	125	9.8 27.2 10.0 18.8 24.8 61.6
30001	4	28 11 65	25.9	16	17.27	20.8	3.41	0.	2.2	127	5.9 22.8 10.0 19.0 19.6 72.0
30001	5	9 12 65	20.2	18	18.70	22.1	4.93	0.	4.2	125	6.5 32.0 9.2 19.0 24.6 71.7
30001	6	18 12 65	16.7	18	16.80	19.4	7.90	0.	4.1	126	7.7 17.1 20.0 20.0 17.0 *
30001	7	15 2 67	47.1	41	23.38	35.7	2.08	3.6	5.4	126	15.3 32.0 *
30001	8	1 11 68	36.5	19	26.35	26.8	2.90	0.	5.5	130	11.7 32.2 9.0 20.5 26.5 70.6
30001	9	5 5 69	27.7	10	19.29	22.4	2.26	15.0	0.8	110	6.0 21.8 10.6 21.0 19.7 65.5
30001	10	22 1 71	24.0	23	13.90	17.5	2.57	29.0	1.8	97	10.0 24.0 26.0 59.2
30001	13	8 3 75	35.8	18	33.34	19.2	3.25	0.	2.0	127	11.1 30.9 11.0 16.0 21.0 59.1
30004	1	19 12 62	15.5	10	3.02	0.9	0.52	0.	0.6	125	2.3 14.7 18.5 8.2 12.9 36.4
30004	3	28 11 55	34.0	16	11.05	10.0	0.98	0.	3.1	128	10.1 27.4 19.0 11.5 11.0 35.0
30004	4	17 12 65	18.0	15	5.45	13.4	1.25	0.	3.4	133	5.1 27.2 *
30004	5	5 11 67	15.2	16	3.72	10.1	0.86	9.9	6.6	121	3.3 21.9 18.2 11.1 12.5 36.1
30004	6	10 7 63	105.5	24	13.34	12.2	0.36	57.2	3.1	70	15.1 15.7 *
30004	7	7 8 68	33.8	7	5.09	5.7	0.67	32.3	2.3	94	2.9 26.0 6.0 9.0 20.3
30004	8	15 9 68	30.1	29	6.58	6.9	0.74	2.8	1.9	124	6.6 22.0 22.2 10.0 13.5 23.1
30004	9	1 11 68	48.7	26	10.17	10.0	0.92	0.	6.4	131	12.2 25.0 15.5 10.7 12.5 35.1
30004	10	5 2 74	11.9	2	7.33	7.3	0.42	0.	5.2	130	6.4 20.1 23.0 10.0 10.0 28.2
30004	11	6 10 74	27.7	8	7.88	10.7	1.95	50.4	5.4	79	6.7 24.3 20.0 11.6 12.0 31.6
30004	12	18 4 75	22.0	10	3.64	5.5	1.12	0.	4.3	131	4.9 22.4 27.5 5.0 8.6 23.2
31005	1	27 2 67	17.3	18	22.98	52.0	2.90	0.	1.0	126	7.6 44.1 *
31005	2	25 11 65	16.5	22	20.33	49.3	2.28	0.	0.8	125	9.3 56.5 *
31005	5	11 3 69	27.8	42	39.51	44.3	3.89	4.3	0.5	121	16.2 58.2 5.7 42.0 46.4 103.3
31005	6	9 1 70	0.6	24	16.03	44.3	1.85	40.0	4.1	89	8.5 90.8 *
31005	7	22 1 71	24.3	22	33.46	37.9	4.47	24.2	1.1	102	11.2 42.4 *
31005	8	5 12 72	20.3	14	22.93	25.9	3.29	47.7	5.0	82	5.6 27.7 8.2 26.5 26.5 83.4
31005	9	19 11 74	22.0	26	32.33	61.1	6.46	1.7	7.0	130	0.7 47.0 *
31005	10	3 7 75	19.7	22	104.47	51.7	4.41	0.	2.4	127	37.0 72.0 *
31010	1	10 7 63	72.8	22	20.03	14.3	0.55	60.0	2.4	67	16.8 23.1 *
31010	2	1 11 68	26.0	15	12.39	15.7	1.07	0.	5.7	130	10.6 40.9 *
31010	3	5 5 69	38.8	11	16.26	12.8	0.77	27.7	0.3	97	12.4 32.1 *
31010	5	6 12 72	16.3	7	5.61	18.7	0.67	38.5	6.7	93	5.7 35.2 *
31010	6	20 11 74	19.0	24	7.60	17.3	1.13	2.6	4.6	127	8.8 44.3 16.2 14.1 13.4 51.5
31010	7	5 3 75	32.9	23	15.63	12.6	0.79	0.	3.0	127	14.2 43.1 *
31010	8	18 4 75	22.2	15	15.02	11.9	1.73	0.	5.0	129	11.3 51.1 *
31021	4	1 12 72	27.0	21	19.20	16.0	1.01	59.8	3.1	68	6.1 22.6 *
31021	6	19 6 73	52.0	21	13.88	13.2	0.49	66.8	0.	38	3.7 7.0 *
31023	2	21 7 73	20.3	14	1.17	3.5	0.06	35.9	4.8	93	5.1 25.2 4.3 5.9 16.3
32001	1	13 10 66	19.4	6	1.60	5.4	0.19	4.8	5.5	125	6.0 30.7 4.4 5.5 20.2
32001	2	1 12 66	9.2	10	1.14	2.4	0.28	0.	4.8	129	3.9 42.9 40.3 5.6 7.7 14.0
32001	3	9 12 66	21.3	10	2.51	5.5	0.19	0.	3.7	128	14.5 66.6 28.6 5.6 7.7 23.3
32001	5	7 9 68	71.9	26	2.92	9.3	0.08	28.8	1.2	97	19.3 26.0 25.9 6.4 9.1 24.8
32001	6	1 11 68	29.2	16	3.06	5.2	0.16	0.	3.3	128	13.1 44.9 36.1 4.3 5.3 20.1
32001	7	15 1 69	9.7	7	1.21	2.1	0.24	0.	1.9	126	3.2 32.6 64.7 2.8 3.3 10.6
32001	9	5 5 69	30.4	13	1.25	5.0	0.09	33.7	0.1	91	4.9 16.0 36.5 3.8 5.8 18.9
32001	10	30 5 69	26.7	13	4.27	6.8	0.16	3.2	2.7	124	13.7 51.3

1-HOUR UNIT HYDROGRAPH
UM PEAK TIME WIDTH UM
CATCH NO. EW DATE OF STORM TOTAL DURN (HRS) (CUMEC) PEAK FLOW (CUMCS) LAG (H) ANSF (MM) (MM) (MM) RUNOFF PER 100 MM (MM) % SO RNS? (MM) (MM) (MM) TO AT 0.5 TIME BASE (MM)

CATCH	NO.	EW	DATE OF STORM	TOTAL DURN	(HRS)	RAINFALL (CUMEC)	PEAK FLOW (CUMCS)	LAG (H)	ANSF (MM)	(MM)	(MM)	SMD	APIS	CHI	RUNOFF PER 100 MM (%)	% SO RNS?	(MM)	(MM)	(MM)	TO AT 0.5 TIME BASE (MM)
37331	6	23	2 67	10.2	12	0.48	4.5	0.28	0.	2.6	127	2.8	27.1	*	*	*	*	*	*	
37331	7	27	2 67	12.5	17	0.16	5.8	0.61	0.	3.4	128	4.1	33.0	*	*	*	*	*	*	
37331	9	9	4 67	19.9	10	5.84	6.5	0.23	14.5	1.0	111	2.5	12.3	*	*	*	*	*	*	
37331	11	18	12 67	18.6	13	7.45	7.1	0.29	9.6	0.3	115	4.4	23.9	*	*	*	*	*	*	
37331	13	8	9 68	13.0	9	4.76	7.6	0.20	81.1	7.0	50	2.6	19.6	*	*	*	*	*	*	
38003	1	2	5 61	8.4	6	1.64	6.3	0.98	4.7	1.3	121	0.1	6.4	*	*	*	*	*	*	
38003	2	12	6 61	27.6	19	1.65	5.9	0.49	81.3	0.	43	0.2	0.9	37.2	4.9	6.0	17.9	*	*	
38003	3	6	7 67	25.7	10	1.69	5.9	0.52	36.1	1.9	92	0.3	1.0	48.0	4.8	4.8	13.6	*	*	
38003	6	25	6 67	30.0	5	2.56	5.5	0.72	51.3	8.1	81	0.3	1.0	*	*	*	*	*	*	
38003	7	22	7 67	25.3	0	2.78	6.0	0.63	88.4	0.7	37	0.2	0.9	*	*	*	*	*	*	
38003	9	15	9 68	6.6	15	3.61	3.1	0.48	37.0	5.8	93	1.0	1.5	*	*	*	*	*	*	
38007	1	20	6 58	39.1	24	13.27	5.5	0.32	14.2	5.6	116	18.7	47.7	32.3	3.6	6.9	20.7	*	*	
38007	2	1	7 58	36.1	30	14.04	3.6	0.26	6.3	2.0	120	20.5	56.7	35.0	3.6	6.7	18.4	*	*	
38007	3	19	9 60	21.4	15	8.57	3.0	0.25	44.6	4.6	65	11.7	54.8	44.2	4.6	5.1	15.0	*	*	
38007	4	8	10 60	19.6	11	8.38	4.4	0.50	9.6	6.3	119	10.2	36.9	35.5	4.9	6.6	18.0	*	*	
38007	5	30	10 60	14.2	6	7.61	3.7	0.76	0.	10.3	135	7.3	52.5	47.8	3.6	6.9	13.4	*	*	
38007	6	25	11 60	15.0	9	7.17	4.1	0.47	0.	5.2	130	10.5	66.2	39.5	5.8	5.2	17.7	*	*	
38007	9	13	7 62	13.3	6	2.10	4.6	0.09	106.3	2.5	21	1.4	10.4	*	*	*	*	*	*	
38007	11	31	8 93	15.1	9	2.02	1.8	0.04	62.9	1.6	63	1.3	8.6	67.0	2.3	3.0	10.5	*	*	
38007	12	17	11 63	13.3	7	2.80	5.5	0.21	0.	1.0	126	3.4	25.3	37.8	5.4	5.2	18.9	*	*	
38007	14	21	7 66	30.6	4	8.46	3.3	0.27	66.8	1.3	59	7.0	17.7	*	*	*	*	*	*	
38007	15	19	7 65	22.6	8	5.89	2.5	0.21	82.1	0.5	43	2.3	10.3	*	*	*	*	*	*	
38007	16	17	11 65	11.3	7	3.84	2.9	0.37	23.4	3.4	105	2.9	25.9	55.2	3.2	4.0	12.1	*	*	
38007	18	22	6 66	33.5	0	3.03	2.4	0.34	75.5	0.5	30	4.4	13.3	37.0	2.7	3.1	25.9	*	*	
38007	19	27	2 67	14.4	12	4.34	4.8	0.66	0.	3.7	128	8.0	55.3	26.7	3.3	6.4	26.0	*	*	
38007	20	25	6 67	23.6	5	4.37	1.7	0.20	58.5	2.3	68	2.7	10.4	70.0	2.3	3.6	8.7	*	*	
38007	22	13	7 68	25.8	7	7.60	2.1	0.31	68.4	1.8	58	4.8	18.7	39.9	3.6	4.3	10.0	*	*	
38007	23	7	10 68	21.1	22	6.27	9.0	0.19	7.9	0.3	117	7.7	36.4	34.6	3.3	6.3	19.1	*	*	
38007	24	28	10 68	19.1	8	4.85	2.6	0.35	15.9	0.7	109	3.1	20.9	34.9	3.4	5.2	9.9	*	*	
39004	6	16	6 65	12.8	9	1.53	1.6	0.02	71.6	1.2	54	0.1	0.8	145.7	1.8	1.8	4.1	*	*	
39004	7	7	7 65	10.1	11	1.84	4.1	0.76	20.0	2.2	47	1.2	1.2	*	*	*	*	*	*	
39004	8	22	7 65	15.2	12	1.73	2.2	0.01	88.1	6.3	43	0.2	1.6	*	*	*	*	*	*	
39004	9	2	9 65	18.0	10	2.37	1.9	0.07	95.5	1.7	31	0.2	1.0	82.2	3.3	3.3	7.0	*	*	
39004	10	3	9 65	58.4	14	3.72	2.3	0.08	82.7	14.2	56	0.8	1.3	56.6	1.8	3.0	13.6	*	*	
39004	12	22	11 65	27.3	18	2.47	3.8	0.01	0.	4.9	2.7	122	0.2	1.1	100.3	1.8	2.3	6.4	*	*
39004	13	22	6 66	29.1	6	3.07	1.8	0.13	82.7	6.7	43	0.3	3.1	24.3	2.4	2.8	8.7	*	*	
39004	14	25	6 67	28.9	7	3.84	7.1	0.29	50.6	7.1	37	0.4	1.3	91.3	1.3	2.3	7.3	*	*	
39004	15	22	7 67	20.4	7	2.98	2.2	0.19	97.2	0.6	34	0.3	1.6	75.1	1.3	2.6	9.6	*	*	
39004	16	11	11 67	20.7	7	2.84	7.7	0.17	0.8	9.1	133	0.4	1.7	75.1	1.3	2.6	9.6	*	*	
39004	17	17	4 68	9.3	2	3.06	0.8	0.25	8.0	2.6	119	0.1	1.1	10.1	1.8	2.1	6.6	*	*	
39004	18	4	5 68	13.0	9	2.65	2.2	0.18	7.5	0.4	117	0.2	1.3	102.9	1.8	2.1	6.6	*	*	
39004	20	13	7 68	17.6	6	3.52	2.3	0.23	35.3	2.2	91	0.2	1.3	95.9	2.0	2.5	6.5	*	*	
39004	21	28	8 68	16.4	4	3.94	1.3	0.23	56.6	2.2	70	0.2	1.1	138.6	2.0	2.7	4.6	*	*	
39004	22	7	1 68	27.7	22	1.72	0.29	0.29	58.8	2.6	88	2.1	1.2	108.8	1.3	2.1	6.0	*	*	
39004	25	28	7 69	39.6	15	5.92	2.2	0.13	101.0	0.4	24	0.7	1.5	*	*	*	*	*	*	
39004	26	2	8 69	27.6	9	4.33	1.0	0.14	81.2	4.3	48									

1-HOUR UNIT HYDROGRAPH

CATCH NO	EV NO	DATE OF STORM	RAINFALL		PEAK FLOW (CUMECES)	LAG (H)	ANSF (CUMECES)	SND (MM)	APIS (MM)	CWI	RUNOFF PER 100 MM X SQ RMS (MM)		TIME BASE	UM PEAK TO AT 0.5 TIME		
			(MM)	(CM)							(MM)	(MM)			(MM)	(MM)
39814	18	19 8 70	12.2	9	0.94	1.2	0.04	138.3	8.8	-6	5.4	64.2	122.1	1.5	2.0	5.1

1-HOUR UNIT HYDROGRAPH																
CATCH NO	EV NO	DATE OF STORM	RAINFALL		PEAK FLOW (CUMEC)	LAG (H)	ANSF (%)	SND (MM)	APIS (MM)	CWI (MM)	RUNOFF PER 100 MM (MM)	TIME TO PEAK (H)	TIME AT 0.5 MAX (H)	UM TIME BASE		
			TOTAL DURN (MM)	(H)												
41022	1	20 11 70	25.5	19	9.99	3.5	0.62	0.	1.0	125	13.3	32.0	19.0	12.0	11.5	35.5
41022	4	5 3 72	11.0	0	6.79	0.4	1.28	0.	12.1	137	6.0	50.2	22.5	7.5	11.5	26.4
41022	5	12 12 72	14.7	1	5.64	5.6	2.16	4.	5.0	120	0.3	43.0	21.0	9.8	15.0	27.0
41022	7	21 11 76	25.0	14	27.45	7.8	9.29	0.	11.2	138	11.7	66.6	36.0	7.6	8.0	16.7
41022	8	19 1 75	34.3	18	29.94	7.2	1.62	35.6	5.1	94	21.8	63.6	33.0	7.6	8.0	17.7
41022	10	8 3 75	14.3	10	10.09	5.2	1.66	0.4	5.6	130	6.6	46.5	25.0	4.8	10.5	23.5
41022	11	18 4 75	20.7	15	10.38	7.9	0.77	3.5	2.9	126	10.5	69.8	19.0	4.0	14.0	30.5
41022	12	1 12 75	32.7	18	16.49	6.4	0.69	12.5	3.3	115	14.0	42.7	25.0	4.6	9.5	25.5
41023	2	10 2 72	14.6	38	8.62	17.9	1.19	0.2	1.0	126	7.9	53.9	•	•	•	•
41023	3	4 3 72	20.6	19	17.50	22.8	1.12	0.	3.1	130	13.1	63.6	•	•	•	•
41023	5	12 12 72	13.5	17	11.99	21.9	3.56	0.	7.2	132	5.9	43.9	•	•	•	•
41023	7	16 2 74	25.6	30	23.68	15.1	2.39	0.2	5.0	129	16.9	66.2	•	•	•	•
41023	10	26 12 76	23.4	26	20.14	20.5	1.94	0.	6.1	129	14.5	61.9	•	•	•	•
41023	12	1 12 75	26.8	17	21.90	19.2	0.51	14.1	3.8	114	16.5	61.1	•	•	•	•
41028	1	13 1 65	20.1	13	5.39	12.2	0.67	2.9	3.0	125	10.6	52.8	21.0	9.5	11.4	30.2
41028	2	19 11 65	43.6	16	8.93	7.1	0.53	0.	3.3	128	17.6	40.4	22.5	6.8	11.5	26.4
41028	3	25 11 65	27.4	21	1.55	5.2	2.45	0.	3.2	127	13.1	47.8	21.5	7.0	10.7	30.3
41028	4	8 12 65	35.3	26	10.40	11.1	0.39	0.2	1.4	126	20.1	56.8	•	•	•	•
41028	5	22 12 65	25.8	14	7.50	6.6	0.82	0.	2.1	127	12.8	53.7	27.0	7.6	9.0	23.2
41028	7	20 2 67	16.0	9	6.17	3.0	0.50	0.	8.6	133	8.7	54.6	29.0	8.5	8.2	27.3
41028	8	27 2 67	26.8	17	5.63	7.4	0.37	0.	2.6	127	13.7	51.2	20.8	8.7	12.0	29.5
41028	9	3 11 67	45.5	24	7.70	12.0	0.72	1.4	12.1	135	21.6	47.6	19.4	10.0	15.0	27.3
41028	10	11 10 68	26.2	13	6.79	6.8	0.84	0.	8.6	133	10.2	53.5	9.0	9.5	26.4	27.4
41028	11	1 11 68	25.0	12	6.79	6.8	0.84	0.	8.6	133	12.5	47.8	21.2	9.0	10.3	27.4
41028	14	21 11 69	19.4	20	3.69	9.0	0.18	0.	16.0	137	9.9	50.8	29.0	9.1	8.4	21.5
41028	16	21 1 71	29.3	18	8.11	9.2	0.75	0.2	3.9	130	14.0	47.6	21.5	10.4	12.6	26.5
41028	17	10 1 72	31.2	15	4.27	11.8	0.16	0.	2.5	127	11.6	37.3	18.8	11.3	12.0	35.1
41028	18	10 2 76	42.9	19	8.48	9.1	0.69	0.	3.9	130	23.1	53.9	21.4	10.1	12.7	26.6
41028	19	21 11 76	40.9	18	13.62	9.5	3.08	0.	10.9	133	14.5	55.6	27.3	6.3	8.6	23.5
41028	20	1 1 75	31.7	26	8.27	8.5	0.66	0.	10.0	134	14.5	55.5	27.0	6.0	7.9	21.0
41001	3	12 2 67	19.7	26	0.51	5.2	0.82	0.	2.0	126	12.6	52.4	•	•	•	•
41001	4	12 3 67	18.2	17	0.94	5.3	0.07	0.	6.5	131	7.0	58.3	43.5	3.6	6.0	13.6
41001	6	4 7 69	43.6	19	1.74	6.3	0.33	47.8	1.2	94	3.5	18.0	•	•	•	•
41001	7	20 7 69	31.2	13	0.75	3.4	0.01	106.5	0.	98	4.5	16.5	55.6	4.0	4.1	11.7
41001	8	1 8 69	45.3	5	1.31	7.9	0.02	92.1	1.6	36	2.7	17.6	64.6	3.2	3.6	11.2
41001	10	11 11 70	25.6	30	2.77	6.7	0.77	90.1	0.4	55	1.7	17.5	56.2	3.6	4.6	11.0
41001	12	13 6 71	50.8	34	2.93	6.8	0.02	59.0	4.7	20	19.8	33.2	38.5	6.1	6.3	15.4
41001	15	6 11 69	20.8	12	1.68	4.1	0.05	15.1	6.6	116	12.9	41.8	39.2	3.1	3.5	19.9
41001	16	6 12 72	17.5	9	2.48	4.1	0.04	44.3	8.8	89	5.2	41.9	65.0	4.1	8.6	8.6
41001	17	21 3 73	24.1	6	2.66	2.3	0.03	23.1	8.2	103	3.9	20.3	•	3.8	9.3	15.1
41001	19	21 11 76	50.2	17	3.02	3.8	0.15	0.	11.6	136	17.3	57.2	66.4	3.3	3.5	20.7
41001	20	20 11 75	20.5	8	2.84	3.9	0.14	14.4	8.7	119	10.7	36.2	33.0	3.5	4.7	25.1
41002	1	24 6 63	23.3	16	37.80	10.2	10.14	2.9	3.9	126	4.6	19.8	•	•	•	•
41002	2	12 11 65	60.7	25	37.80	10.2	10.14	2.9	3.9	126	4.6	19.8	•	•	•	•
41002	3	12 1 65	21.1	22	14.22	7.2	1.35	0.	1.9	126	3.0	14.3	28.1	0.0	7.3	25.0
41002	4	12 11 66	30													

1-HOUR UNIT HYDROGRAPH											
CATCH NO	EV	DATE	STORM	RAINFALL		PEAK FLOW		TIME WIDTH		UM	
				OF TOTAL QUN	(MM) (CUMECS)	LAG (H)	ANSF (MM)	SMD (MM)	API5 (MM)	CWI	RUNOFF PER 100 MM (MM)
											X SQ KMS (K)
47007	5	17	12	65	48.1	41	20.81	9.6	5.80	0.	76.6 141
47007	6	22	12	65	26.1	14	21.50	6.1	5.03	0.	9.0 133
47007	7	28	12	65	33.1	17	21.78	5.7	3.12	0.	0.8 125
47007	8	24	12	65	38.6	15	19.11	6.3	2.01	0.	1.9 126
47007	10	2	3	66	26.7	16	21.88	5.0	3.80	0.	9.6 136
47007	11	5	5	66	53.3	21	21.95	4.7	0.91	2.6	4.8 127
47007	12	22	10	67	44.1	11	22.00	5.2	2.76	0.8	3.0 127
47007	13	20	22	67	26.3	11	20.48	4.3	4.57	0.	15.3 146
47007	15	24	6	67	31.3	17	19.06	6.9	1.83	0.	14.5 139
47007	16	27	6	67	31.0	11	21.31	7.1	4.86	0.4	4.0 144
47008	2	12	3	72	23.6	9	30.42	5.2	6.17	0.	16.6 142
47008	4	30	12	72	36.8	25	38.90	4.1	3.61	0.	5.6 130
47008	6	22	5	73	19.4	19	15.32	7.6	0.85	0.8	1.3 116
47008	7	15	12	73	10.1	18	11.07	5.0	2.78	0.	2.6 127
47008	8	29	12	73	9.1	9	9.96	5.0	2.23	0.	1.1 126
47008	10	25	1	74	29.4	28	41.01	5.1	3.15	0.	4.2 129
47008	14	13	9	73	37.4	15	5.21	5.9	7.47	0.4	1.9 66
47011	1	18	6	74	16.3	12	18.35	3.7	1.1	1.1	1.1 111
47011	2	15	10	74	26.2	20	16.96	6.7	0.67	5.3	7.7 125
47011	4	18	12	74	38.1	31	18.46	4.9	1.66	0.	0.2 125
47011	5	23	5	74	26.4	9	31.73	3.1	3.40	1.1	1.5 126
47011	6	14	6	74	30.3	18	22.89	7.1	2.48	1.7	3.6 126
47011	7	14	4	74	27.1	17	17.14	5.1	2.23	0.	1.1 127
47011	8	1	4	74	13	13	15.45	6.5	1.20	2.4	1.5 123
47011	9	4	8	74	20.4	42	26.35	12.2	0.99	2.3	27.9 99
47011	12	3	1	74	45.7	27	40.40	3.2	4.69	0.	5.1 178
47011	14	26	2	74	39.1	22	22.25	4.9	3.46	0.	1.1 127
47011	15	12	10	74	49.7	15	18.19	4.1	2.76	0.	2.1 127
47011	16	21	12	74	4.3	10	27.10	6.8	4.09	0.	9.0 136
47011	17	21	12	74	10.4	10	26.13	5.3	3.66	0.	5.1 130
48004	1	27	3	75	20.2	22	16.00	8.2	2.03	0.	13.2 138
48004	2	11	2	75	26.9	76	8.03	6.9	1.48	0.	3.2 120
48004	3	20	5	75	45.4	27	7.23	13.8	0.56	35.6	12.1 101
48004	4	6	11	75	31.0	26	42.6	10.3	0.52	0.	5.7 133
48004	5	17	6	75	39.8	32	3.74	0.2	0.25	28.6	1.1 123
48004	7	11	6	75	34.0	33	6.01	13.6	1.69	1.4	3.1 127
48004	8	30	11	75	34.6	32	6.08	7.0	1.05	0.	5.2 130
48004	9	37	11	75	41.8	29	14.03	11.0	0.39	0.	1.9 126
48004	10	26	9	75	40.8	29	14.77	10.2	1.33	0.	2.1 130
48004	11	17	10	75	40.4	29	17.98	8.9	0.01	0.	3.2 128
48004	12	12	11	75	30.2	14	11.32	6.4	1.47	0.	0.5 134
48005	2	25	6	76	13.7	13	11.29	6.2	0.19	3.0	1.28
48005	3	28	7	76	6.1	11	7.41	6.2	2.03	0.	1.3 128
48005	5	18	12	76	17.5	7	1.56	5.1	0.31	0.	1.1 126
48005	6	7	8	76	22.6	12	0.74	2.0	0.15	44.6	6.3 88
48005	9	19	1	76	42.0	10	0.74	2.0	0.15	44.6	6.3 88
48005	10	12	1	76	22.6	12	0.74	2.0	0.15	44.6	6.3 88
48005	13	30	1	76	12.4	11	4.42	4.3	1.17	0.	7.0 131
48005	14	16	8	76	16.7	11	2.43	1.6	0.29	0.	2.9 127
48005	15	16	8	76	20.3	10	1.19	4.0	0.13	9.6	2.1 102
48005	16	13	9	76	33.1	20	3.62	3.2	0.15	76.8	4.4 82
48009	2	29	11	77	17.9	25	8.70	1.79	0.	10.9 134	
48009	4	25	1	77	32.5	25	8.70	1.79	0.	10.9 134	
48009	5	31	1	77	32.5	25	8.70	1.79	0.	10.9 134	
48009	6	11	6	77	39.3	36	4.13	13.8	0.75	0.	5.3 122
48009	8	4	8	77	30.3	36	4.13	13.8	0.75	0.	5.3 122
48009	9	17	10	77	21	7.47	5.9	0.97	0.	3.5 128	
48009	10	12	11	77	32.1	19	10.35	3.4	1.68	0.	10.3 128
49003	1	20	12	66	35.8	21	14.10	5.9	1.56	3.	16.4 139
49003	2	22	1	67	35.9	9	14.32	5.0	1.05	0.	0.7 134
49003	3	27	2	67	42.0	22	16.74	4.0	0.95	0.	2.5 129
49003	4	15	12	67	16.9	20	16.89	5.8	1.52	0.	13.3 130
49003	6	17	12	67	21.6	19	17.70	8.3	0.46	0.	7.4 128
49003	7	21	12	68	25.1	19	10.14	5.6	0.78	0.	4.4 129
49003	8	23	12	68	7.3	21	15.93	13.9	0.86	0.	9.1 134
49003	9	27	12	69	15.5	21	15.52	9.1	0.78	0.	1.1 134
49003	10	16	1	70	47.5	29	9.0	9.0	0.87		

RAINFALL												1-HOUR UNIT HYDROGRAPH																			
CATCH		EV		DATE OF STORM		TOTAL DURN (H)		FLOV (CUMECS)		LAG (H)		ANSF (MM)		SMO (MM)		APIG (MM)		CWI (MM)		RUNOFF (MM) X SQ KMS		TO AT 0.5 PEAK (H)		UM TIME							
NO	NO	NO	NO	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	MM	PEAK (H)	BASE (H)	PER 100	PEAK							
54004	8	22	72	65	20.1	20	23-65	10-7	5.47	0.	1.6	126	10.0	49.6	9.3	13.0	22.0	73.6	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	1.6	9.0			
54004	9	18	22	66	40.5	63	32-18	20-2	4.23	1.0	0.3	126	22.0	54.3	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.1	7.0			
54004	10	29	66	42.6	19	25-33	3-2	5.38	62.2	0.0	0.8	126	7.6	17.5	0.	0.	0.	0.	0.	0.07	4.6	7.2	127	88.5	75.9	98.9	1.3	1.7	7.0		
54004	11	9	12	66	15.7	19	24-36	12-8	7.25	0.	4.0	129	6.5	41.3	11.0	13.0	20.2	60.7	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	1.7	7.0			
54004	12	8	3	67	23.3	22	22-31	12-1	4.23	0.	2.0	127	8.8	37.1	10.5	11.0	22.5	60.9	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.3	10.9			
54004	13	10	7	66	56.4	19	42-31	19-0	3.36	10.7	4.8	119	21.1	38.7	0.	0.	0.	0.	0.	0.05	97.1	2.8	30	54.0	75.9	71.1	1.2	2.3	4.8		
54004	14	12	3	67	29.2	26	35-35	17-2	3.04	5.0	3.3	125	16.3	55.9	10.0	13.0	23.0	65.2	22.0	0.07	80.1	4.9	49	33.6	34.6	129.6	1.0	1.7	5.1		
54004	15	5	6	67	36.2	12	34-79	13-6	3.55	20.6	0.5	105	12.9	35.7	10.8	15.0	23.0	57.0	22.0	0.07	0.	0.	0.	4.1	129	47.1	64.0	91.6	1.7	1.9	8.4
54006	9	10	7	68	36.4	21	19-37	10-4	2.82	10.0	3.8	120	5.7	15.8	0.	0.	0.	0.	0.	0.07	0.	0.	0.	0.	0.	0.	0.	0.	0.		
54006	10	12	3	69	28.3	23	20-07	24-5	2.58	0.	2.4	127	6.7	23.7	9.7	24.7	22.1	70.4	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	1.5	9.0			
54006	11	5	5	69	35.7	13	21-61	20-0	2.83	19.5	4.5	110	6.1	17.0	10.9	21.0	22.0	58.0	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.8	12.3			
54006	12	2	8	69	28.7	22	18-04	19-3	2.95	63.8	9.9	69	5.2	18.1	0.	0.	0.	0.	0.	0.07	0.	0.	0.	0.	0.	0.	0.	0.	0.		
54006	14	27	9	50	44.9	25	70-57	24-0	5.30	0.2	5.8	130	13.1	29.1	5.8	26.2	35.0	93.5	22.0	0.07	0.	0.	0.	0.	0.	0.	0.	0.	0.		
54010	3	21	1	59	22.1	26	37-02	19-7	9.54	0.	8.7	133	9.2	41.6	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	1.6	9.0			
54010	4	23	1	60	34.0	35	49-61	13-6	5.06	0.	6.8	129	13.1	38.6	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.0	7.1			
54010	5	27	4	50	27.0	28	47-56	15-3	5.28	0.	3.9	128	11.0	40.6	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.6	12.3			
54010	6	16	11	60	18.3	30	34-49	19-3	4.88	0.	1.9	126	7.8	41.6	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.5	10.5			
54010	10	14	5	67	29.3	32	43-56	16-1	1.92	7.3	6.4	126	13.1	44.8	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.7	7.1			
54010	13	12	3	69	24.9	33	33-86	21-0	3.52	1.2	3.3	127	9.8	39.4	12.0	21.3	18.0	56.7	22.0	0.07	0.	0.	0.	0.	0.	0.	0.	0.	0.		
54011	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
54011	5	24	9	65	25.3	37	11-83	21-8	2.70	0.	5.2	130	5.8	22.9	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	1.6	9.0			
54011	6	23	11	65	18.5	35	15-07	15-6	1.09	0.	2.1	127	7.1	38.2	12.6	14.0	19.4	49.5	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.0	7.1			
54011	7	8	12	65	20.2	39	10-27	11-9	2.05	0.	1.8	126	11.0	54.5	0.	0.	0.	0.	0.	0.05	10.1	40	60.2	67.0	91.2	1.3	2.6	12.3			
54011	8	22	12	65	13.5	22	11-70	14-9	1.62	0.	1.4	126	4.9	31.4	14.0	11.3	16.8	45.8	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.5	10.5			
54011	9	31	12	65	17.7	31	16-37	12-8	2.19	0.	2.4	127	7.1	40.0	14.5	12.5	16.0	46.7	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.7	7.1			
54011	10	11	20	7	66	20.8	31	12-06	22-9	0.96	6.4	4.7	122	4.1	19.5	19.3	12.3	12.5	33.3	22.0	0.05	10.1	40	60.2	67.0	91.2	1.3	2.7	7.1		
54011	11	20	2	67	11.0	21	15-20	11-5	1.17	0.	1.1	126	6.8	73.9	4.0	44.6	17.0	14.0	12.5	39.8	22.0	0.05	10.1	40	60.2	67.0	91.				

CATCH NO	EV NO	DATE OF STORM	1-HOUR UNIT HYDROGRAPH												1-HOUR UNIT HYDROGRAPH																					
			RAINFALL (MM)			PEAK FLOW (CUMECs)			LAG (H)			ANSF (MM)			SMD (MM)			APIS (MM)			CWI (MM)			RUNOFF PER 100 MM X 50 KM3 (MM)			TO AT 0.5 TIME BASE (H)			UH PEAK (CUMECs)			TIME WIDTH (H)			
			TOTAL DURN (H)	DURN (H)	FLOW (CUMECs)	LAG (H)	CUMECs	LAG (H)	ANSF (MM)	SMD (MM)	APIS (MM)	CWI (MM)	RUNOFF (MM)	PER 100 MM X 50 KM3 (MM)	TO AT 0.5 TIME BASE (H)	UH PEAK (CUMECs)	TIME WIDTH (H)	UH PEAK (CUMECs)	TIME TO AT 0.5 (H)	UH PEAK (CUMECs)	TIME WIDTH (H)	UH PEAK (CUMECs)	TIME TO AT 0.5 (H)	UH PEAK (CUMECs)	TIME WIDTH (H)	UH PEAK (CUMECs)	TIME TO AT 0.5 (H)	UH PEAK (CUMECs)	TIME WIDTH (H)							
56005	12	6 11 70	57.3	20	31.32	4.8	5.43	1.2	1.8	125	15.0	26.1	22.7	6.2	14.2	26.6	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56005	14	24 1 75	31.7	11	34.86	5.8	6.97	0.	10.8	135	9.6	30.3	37.5	4.1	2.6	18.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56005	15	14 2 76	42.5	16	36.35	4.8	6.44	0.	10.3	135	11.0	28.0	30.7	5.0	8.6	19.0	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56005	16	10 10 76	30.6	11	46.86	4.1	7.58	14.3	6.3	117	12.5	40.7	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
56005	17	13 10 76	30.9	14	40.35	6.0	8.10	6.4	7.1	125	15.6	49.8	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56005	18	17 10 76	41.7	17	49.59	6.8	9.93	2.9	5.8	127	18.9	45.6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56006	1	11 12 64	81.7	39	193.65	8.2	12.40	0.	9.1	134	52.6	66.4	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56006	2	12 1 65	44.8	16	226.53	2.8	18.70	0.	13.7	138	28.6	65.9	67.6	4.5	3.8	8.8	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	3	8 12 65	59.1	48	148.38	4.8	8.76	0.	3.2	128	35.1	50.8	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56006	4	16 12 65	144.6	52	223.56	5.5	17.02	0.	9.2	134	106.8	73.9	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56006	5	24 2 66	53.0	15	193.12	3.3	16.18	0.	8.1	133	30.5	57.5	60.0	3.5	4.0	10.5	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	6	27 2 67	64.0	18	239.32	5.3	13.57	0.	8.2	133	42.0	65.6	68.5	6.0	5.3	18.3	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	7	16 10 67	81.0	18	242.66	3.6	17.05	0.	14.2	139	48.0	59.3	66.0	5.8	5.6	14.1	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	8	10 11 74	57.8	25	142.00	3.5	33.30	0.	19.7	144	27.2	47.1	50.0	3.5	4.6	13.0	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	9	12 11 74	58.9	29	160.68	6.5	16.95	0.	16.7	141	33.9	49.2	38.0	5.7	6.0	17.3	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	10	13 11 74	61.1	21	76.93	6.7	6.53	0.	2.6	127	15.9	38.7	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*		
56006	11	2 2 77	40.3	20	89.61	3.7	14.38	0.	3.6	128	11.8	29.2	36.5	5.8	4.0	19.6	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56006	12	3 2 77	30.1	15	100.01	4.1	4.67	0.	14.0	139	12.9	42.2	40.0	4.6	5.8	16.2	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56007	1	30 11 75	47.8	26	26.33	9.6	1.14	0.	2.4	127	19.2	40.2	22.3	4.0	8.2	33.5	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56007	2	11 2 76	24.8	18	10.96	8.9	1.50	0.	3.8	128	7.7	31.0	18.3	10.5	12.5	35.8	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56007	3	12 3 77	40.3	31	25.03	2.8	3.14	0.	7.8	132	9.6	23.9	38.4	6.2	6.0	17.0	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9	5.6	10.9	135	18.2	44.9	25.2	7.7	9.6	24.9		
56007	4	13 4 77	47.5	37	34.88	9.4	4.68	0.	10.0	135	25.6	44.3																								

CATCH	EV	RAINFALL			PEAK FLOW	1-HOUR UNIT HYDROGRAPH										UM TIME	
		NO	DATE OF STORM	TOTAL DURN (MM)	(CUMECs)	LAG (H)	ANSF (CUMECs)	SMD (MM)	API5 (MM)	CWI (MM)	RUNOFF (CUMECs)	TO 100 SQ KMS (H)	PEAK (H)	PER 100 PEAK (H)	TIME AT 0.5 BASE (H)	UM TIME	
65001	13	14	9 65	123.3	28	52.78	9.1	0.35	3.2	4.5	126	52.9	42.9	25.5	3.4	9.8	24.0
65001	14	28	12 65	71.6	17	43.81	5.0	1.32	0.	0.7	125	25.3	35.3	31.3	3.7	7.0	21.5
65001	15	26	6 66	106.8	24	51.09	8.2	0.80	2.4	7.7	130	45.2	42.3	22.0	3.9	10.3	30.5
65001	21	22	3 68	117.2	34	56.63	11.0	1.52	0.	17.5	137	65.7	56.0	*	*	*	*
65801	2	28	6 72	86.3	17	17.60	4.0	0.44	20.0	1.0	105	38.9	45.1	45.0	2.1	5.1	14.5
65801	8	11	11 72	65.0	25	23.50	5.9	0.86	0.	11.9	136	53.5	82.8	49.2	6.7	5.1	12.4
65801	16	12	12 72	55.8	10	26.00	3.1	1.99	0.	36.5	161	44.3	79.3	69.0	2.4	3.0	10.1
66002	2	24	5 68	40.5	32	36.44	10.7	3.36	14.0	0.1	111	7.9	19.6	*	*	*	*
66002	3	30	6 68	66.3	51	52.51	9.1	3.46	40.0	2.8	87	12.0	18.0	29.0	4.8	5.6	27.1
66002	5	5	5 69	29.9	21	35.71	5.2	4.82	3.6	3.7	125	4.6	15.1	35.1	7.4	5.6	20.5
66002	10	15	7 73	76.8	25	42.74	13.3	0.05	102.7	21.0	43	8.0	10.5	28.6	1.8	9.2	20.5
66004	1	22	8 70	57.7	31	2.54	9.2	0.57	102.7	4.3	26	2.7	6.6	*	*	*	*
66004	2	11	6 71	21.5	10	1.23	5.5	0.54	54.4	8.0	78	0.6	2.7	*	*	*	*
66004	3	25	7 71	0.4	9	2.64	4.1	0.47	44.4	15.0	74	*	*	*	*	*	*
66004	7	14	7 73	20.1	66	3.03	11.0	0.66	77.6	0.7	48	2.6	4.6	*	*	*	*
66004	7	14	7 73	67.2	36	2.37	19.9	0.52	83.3	7.7	49	1.7	2.8	*	*	*	*
66006	1	15	7 73	74.4	29	63.92	73.8	3.19	101.9	18.6	61	17.5	23.6	24.3	6.5	6.7	32.4
66006	3	10	11 74	23.4	16	50.57	6.2	6.21	0.3	2.6	127	7.8	33.1	35.1	4.6	7.1	17.5
66006	7	2	9 76	30.4	12	81.79	0.4	16.47	8.3	17.9	136	12.8	42.2	*	*	*	*
66006	9	13	10 73	84.6	48	141.17	12.3	8.90	5.7	5.4	121	57.6	66.3	24.2	9.3	0.1	29.8
66011	1	7	7 64	71.6	15	236.74	5.4	6.51	44.4	0.1	80	19.3	27.0	*	*	*	*
66011	2	13	11 64	41.2	13	241.45	5.5	10.39	0.	1.1	133	21.1	51.9	36.9	4.3	6.6	17.3
66011	4	11	22 64	191.5	34	535.23	5.9	26.01	0.	22.3	147	147.5	77.0	*	*	*	*
66011	5	6	10 63	42.6	10	333.01	4.2	20.64	2.0	9.8	132	22.5	52.7	*	*	*	*
66011	9	7	10 66	40.6	17	301.86	5.3	15.83	0.	13.1	138	23.2	57.2	*	*	*	*
66011	10	30	11 66	73.4	43	335.68	1.0	27.90	0.	11.0	135	45.0	58.9	45.0	3.5	5.6	11.5
66011	11	12	10 67	41.5	17	309.48	2.7	17.4	0.	1.1	129	38.9	63.0	49.6	5.2	4.8	12.8
66011	12	26	6 67	71.8	19	320.77	6.8	35.88	0.	7.6	132	57.9	80.6	46.0	2.7	5.5	14.3
66011	13	1	10 67	56.8	14	462.82	5.7	25.15	0.	9.8	134	42.1	74.1	35.7	5.0	7.0	17.1
66011	16	10 67	71.6	23	396.79	3.8	43.22	0.2	16.3	141	35.3	77.4	35.0	4.6	7.4	17.0	
66011	15	22	12 67	57.9	18	376.91	6.0	19.00	0.	2.8	127	44.9	77.6	*	*	*	*
67003	1	22	3 68	50.2	35	43.39	2.8	1.60	0.	10.3	135	38.0	75.7	*	*	*	*
67003	3	19	6 70	47.1	30	15.31	3.8	0.54	101.1	1.2	25	31.2	66.2	40.0	6.8	7.0	13.8
67003	4	28	2 71	17.7	19	7.42	7.7	0.52	1.4	2.2	125	11.3	66.5	42.0	4.8	5.0	16.5
67003	5	9	8 71	45.8	24	14.81	10.2	0.27	74.0	1.0	52	24.5	53.5	66.0	4.8	4.6	16.5
67003	6	30	2 72	46.2	27	21.80	7.4	0.17	62.5	0.6	43	35.1	74.1	76.5	7.0	7.0	16.5
67003	7	7	7 73	26.1	6	14.37	1.7	1.10	71.0	0.5	52	11.3	41.0	48.0	3.0	3.5	12.2
67005	7	22	2 67	40.7	21	36.14	6.2	7.73	0.	7.1	132	12.3	30.1	22.5	5.8	8.6	32.2
67005	8	27	2 67	48.2	28	30.85	6.8	7.97	0.	5.4	130	14.5	30.1	20.3	5.0	8.5	37.8
67005	9	15	13 69	53.0	50	27.28	3.7	5.89	0.	9.6	134	15.0	27.9	*	*	*	*
67005	10	28	10 67	26.4	13	24.47	3.4	9.20	0.8	13.9	138	6.7	25.5	25.3	5.4	9.2	25.6
67008	13	24	5 68	38.6	30	16.60	8.2	2.06	14.0	0.3	111	6.0	15.5	*	*	*	*
67008	15	1	11 68	24.7	20	12.17	12.6	2.20	0.	1.1	128	4.9	19.8	11.5	9.6	24.0	48.7
67008	17	25	4 69	19.4	11	12.13	7.8	2.56	5.7	1.1	120	2.4	12.2	22.0	7.4	9.0	32.5
67008	18	29	5 69	23.3	19	21.23	5.2	6.00	5.6	3.5	122	3.9	16.8	20.0	6.1	13.5	28.6
67008	22	9	8 71	59.0	31	17.44	13.9	4.98	1.4	1.7	125	3.3	17.4	21.2	6.0	9.0	34.5
67008	23	20	11 71	28.5	21	29.84											

1-HOUR UNIT HYDROGRAPH												1-HOUR UNIT HYDROGRAPH																													
CATCH		EV		DATE OF STORM		TOTAL DURM (HR)		PEAK FLOW (CUECS)		LAG (H)		UM PEAK (CUECS)		TIME WIDTH (H)		UM (CUECS)		TO AT 0.5 SQ KMS		RUNOFF PER 100 MM (H)		PEAK (H)		TIME BASE (H)		UM PEAK (CUECS)		TIME WIDTH (H)		UM (CUECS)		TO AT 0.5 SQ KMS		RUNOFF PER 100 MM (H)		PEAK (H)		TIME BASE (H)			
71801	4	18	3 58	62.0	34	122.02	4.8	12.27	0.	6.4	131	41.7	67.3	23.6	7.4	9.5	28.1	74001	1	19 12 68	45.1	16	59.77	5.4	3.96	0.	4.3	129	28.5	63.7	0.	0.	0.	0.	27.5	56.7	50.8	3.6	5.0	11.9	
71802	1	30	10 48	54.0	50	133.47	5.8	9.64	1.0	7.9	131	36.7	68.0	32.5	8.6	8.8	16.6	74001	9	20 1 69	85.6	13	119.12	6.1	5.04	0.2	3.6	128	10.1	77.1	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
71802	9	30	3 69	52.0	23	142.41	7.9	6.26	0.	5.0	130	29.8	57.3	0	*	*	*	74001	10	13 12 69	60.1	20	107.78	5.7	3.62	0.2	0.9	125	37.9	51.0	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
71804	1	5	7 60	108.5	39	25.16	1.9	0.53	29.6	7.2	52	64.5	59.4	87.4	2.3	2.6	7.6	74001	12	18 1 72	55.7	19	154.82	6.3	6.34	0.	14.6	139	39.8	70.2	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
71804	2	3	8 61	43.1	18	33.73	2.4	0.43	45.9	1.1	80	21.0	35.3	66.6	1.8	3.1	10.9	74006	2	10 11 74	53.7	16	43.10	4.5	4.77	0.	1.8	141	30.0	59.6	42.1	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3
71804	3	22	8 62	60.5	23	27.47	0.5	1.08	20.0	9.2	114	18.4	30.4	120.0	1.8	1.8	5.8	74006	6	13 1 75	56.6	14	37.19	4.6	6.99	0.	1.8	141	30.0	65.8	42.7	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3
71804	4	25	9 63	63.6	25	24.15	2.4	0.57	0.	16.0	141	18.7	29.4	80.2	2.0	2.1	9.7	74006	5	26 1 75	18.2	25	41.05	4.8	4.76	0.	2.3	148	55.1	66.7	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
71804	5	20	11 03	59.3	15	21.85	1.6	0.87	0.	10.6	135	11.2	17.6	95.7	2.1	2.6	7.7	74006	9	2 1 76	60.2	12	27.00	4.9	7.34	0.	13.0	139	21.9	54.3	42.7	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3
71804	6	8	12 64	73.7	29	22.84	3.3	0.98	0.	32.5	157	27.6	37.4	56.8	2.6	3.0	15.6	74006	10	27 11 76	35.9	8	42.13	3.1	6.21	0.	22.0	147	22.7	52.3	48.5	3.6	3.6	10.3	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	2	16	12 62	27.0	26	99.48	6.9	4.69	0.	3.4	128	15.7	58.1	26.5	5.8	8.5	22.0	75007	1	9 1 74	28.0	16	40.84	3.7	4.32	0.	11.2	116	12.6	44.8	49.0	3.6	4.4	13.5	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	3	25	9 63	38.2	14	131.13	8.0	8.42	0.	13.5	138	16.9	44.1	29.6	5.8	7.9	21.8	75007	2	10 11 74	44.1	17	65.16	5.8	6.12	0.	18.6	143	27.4	48.6	4.0	4.0	13.6	0.	26.6	53.8	50.8	3.6	4.3	11.3	
72002	4	2	10 63	31.5	11	138.87	0.1	10.21	0.	6.4	131	17.2	56.5	31.6	5.6	7.6	20.2	75007	3	23 11 74	48.6	26	67.76	6.5	3.05	0.	1.1	130	21.3	64.4	40.3	3.9	4.1	13.5	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	5	20	11 63	31.2	13	118.24	5.8	9.83	0.	6.9	131	16.5	52.9	29.9	5.6	8.2	20.8	75007	4	28 12 74	31.6	17	43.82	7.7	2.72	0.	10.5	135	16.8	53.1	66.6	3.8	6.0	15.0	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	6	10	5 56	36.8	18	136.79	6.0	5.87	3.6	6.7	128	20.7	56.3	28.0	4.8	8.8	22.1	75007	5	13 1 75	27.6	14	33.26	8.4	6.34	0.	10.2	135	19.1	50.9	51.7	4.0	4.7	11.9	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	7	8	12 66	39.8	15	142.71	9.1	13.60	0.	16.8	141	24.5	61.7	26.6	6.3	8.5	20.5	75007	6	24 1 75	41.3	12	43.03	7.5	4.24	0.	13.1	134	17.5	42.3	4.4	4.4	11.3	0.	26.6	53.8	50.8	3.6	4.3	11.3	
72002	8	9	9 65	61.6	43	120.75	8.7	9.09	0.	11.3	136	41.6	67.8	23.5	7.0	9.0	29.3	75007	7	24 9 75	63.7	12	62.31	5.3	4.47	0.	19.2	139	20.5	38.8	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
72002	9	4	10 65	29.6	18	145.56	6.4	12.33	0.	10.2	135	22.5	73.8	29.5	6.0	8.0	22.9	75007	8	26 9 75	34.6	15	42.39	4.1	4.01	0.	13.8	132	18.3	53.5	67.0	4.0	4.1	11.1	0.	26.6	53.8	50.8	3.6	4.3	11.3
72002	10	7	12 65	38.0	61	117.2	1.1	6.32	0.	6.6	131	25.0	65.9	28.0	5.8	8.8	22.1	76005	1	22 1 70	33.5	24	205.54	12.0	20.79	0.	3.7	128	25.4	65.9	0.	0.	0.	0.	26.6	53.8	50.8	3.6	4.3	11.3	
72002	11	16	12 65	41.0	67	123.7	0.9	8.16	0.																																

CATCH NO	EV NO	DATE OF STORM	RAINFALL		SLR	LLG	ANSF	SMO	APIS	CHI	RUNOFF	1-HOUR UNIT HYDROGRAPH	
			TOTAL INCH	14MM								(IN)	(CM)
84021	8	12-17	24.0	10.8	10.8	4.2	6.82	0.	16.7	1.7	12.6	65.0	*
84022	9	11-12	32.9	13	13.92	5.0	3.71	0.	6.7	1.9	66.2	74.0	*
84022	10	23-24	18.6	9	10.8	2.9	6.12	0.	5.6	1.2	2.7	7.5	*
84022	11	25-26	76.5	37	56.31	5.3	1.15	0.	5.7	9.0	23.8	36.1	*
84022	12	12-13	6.7	3.4	25.0	5.2	7.62	1.2	4.5	125	4.8	20.0	47.3
85002	8	13-15	17.1	12	10.82	4.2	6.93	0.	6.6	131	11.1	35.7	31.8
85002	9	1-3	10.3	3.9	106.30	7.3	47.10	0.	10.8	135	26.3	65.6	39.0
85002	10	20-21	37.2	21	80.47	5.9	17.79	0.	1.3	122	16.2	30.6	28.3
85002	11	20-21	31.3	22	100.64	5.6	10.46	0.	10.6	135	17.4	33.7	29.8
85001	1	16	72.16	22.0	18	79.85	22.1	18.12	0.	3.1	128	8.8	39.4
85001	2	17	72.35	39.8	29	101.23	24.2	26.55	0.	3.6	130	30.8	51.5
85001	3	18	62.9	13.8	99.07	23.3	20.21	0.	2.8	127	32.2	51.3	46.3
85001	4	19	5.6	42.0	95.93	17.4	12.41	1.0	3.1	129	20.0	46.7	9.7
85001	5	20	32.0	12.6	132	70.46	17.0	26.19	2.	9.1	134	22.9	43.3
85001	6	21	3.6	74.6	476	80.09	28.2	8.73	1.0	3.0	126	37.9	50.9
85001	7	22	66	48.5	78	402.51	29.0	77.79	0.	4.8	129	23.3	51.6
85001	8	23	68	32.1	42	94.90	20.9	21.85	0.	12.6	137	16.3	44.7
85001	10	24	69	104.5	240	124.13	37.4	16.42	0.	0.4	125	62.6	59.9

THE KHEDDA HOURS 5-LEVEL DIFFERS FROM ONE HOUR. THE UNIT HYDROGRAPH PARAMETERS HAVE BEEN ADJUSTED TO THE 1-HR UN

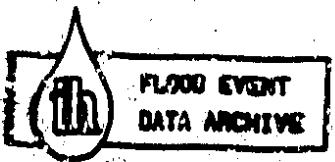
APPENDIX

THE FLOOD EVENT DATA ARCHIVE

The data used for this study are stored on a computer data base called the Flood Event Data Archive. Data held for each event include flow, catchment average rainfall, raw (collated but not processed) rainfall, antecedent rainfall, soil moisture deficit and text comments. While analysis programmes can access these data directly two standard retrieval options are available to list and plot the data. Examples of these are given in Figures B.1 and B.2. In addition to the event data, information is held that relates directly to catchments. Examples of these are catchment characteristics, digitised data, lists of daily and recording gauges and text comments. The archive contents for a particular catchment are summarised on another standard retrieval as shown in Figure B.3. The lists of daily and recording rainfalls held against each catchment greatly aid the collation of event rainfalls by speeding the interface with peripheral archivers containing data from these gauges.

FIGURE 8

14.00	41.00	74.00	7.00	6.00	5.00
20.00	46.00	47.00	7.00	5.00	3.25
21.00	45.00	101.00	2.00	7.00	0.
22.00	49.00	114.00	7.00	3.50	0.
23.00	46.00	117.00	1.00	1.00	0.
0.	46.00	207.00	2.00	0.00	0.25
1.00	46.00	247.00	2.00	0.	0.25
2.00	52.00	253.00	3.00	2.00	1.00
3.00	51.00	357.00	2.00	2.00	0.
4.00	52.00	291.00	2.00	2.00	0.
5.00	53.00	295.00	2.00	2.00	0.
6.00	54.00	293.00	2.00	2.00	0.
7.00	55.00	265.00	2.00	2.00	0.
8.00	56.00	245.00	2.00	2.00	0.50
9.00	57.00	212.00	2.00	2.00	0.
10.00	58.00	213.00	2.00	2.00	0.
11.00	59.00	214.00	2.00	2.00	0.
12.00	60.00	206.00	2.00	2.00	0.
13.00	61.00	194.00	2.00	2.00	0.
14.00	62.00	195.00	2.00	2.00	0.
15.00	63.00	177.00	2.00	2.00	0.
16.00	64.00	170.00	2.00	2.00	0.
17.00	65.00	163.00	2.00	2.00	0.
18.00	66.00	157.00	2.00	2.00	0.
19.00	67.00	152.00	2.00	2.00	0.
20.00	68.00	147.00	1.50	2.00	0.75
21.00	69.00	142.00	1.00	2.00	0.
22.00	70.00	139.00	0.50	2.00	0.
23.00	71.00	134.00	2.00	2.00	0.25
0.	72.00	131.00	2.00	2.00	0.
1.00	73.00	127.00	2.00	2.00	0.
2.00	74.00	126.00	2.00	2.00	0.
3.00	75.00	125.00	2.00	2.00	0.
4.00	76.00	117.00	2.00	2.00	0.
5.00	77.00	114.00	2.00	2.00	0.
6.00	78.00	112.00	2.00	2.00	0.25
7.00	79.00	107.00	2.00	2.00	0.25
8.00	80.00	103.00	2.00	2.00	0.
9.00	81.00	105.00	2.00	2.00	0.
10.00	82.00	97.00	2.00	2.00	0.
11.00	83.00	93.00	2.00	2.00	0.
12.00	84.00	90.00	2.00	2.00	0.
13.00	85.00	89.00	2.00	2.00	0.
14.00	86.00	83.00	2.00	2.00	0.
15.00	87.00	81.00	2.00	2.00	0.
16.00	88.00	77.00	2.00	2.00	0.
17.00	89.00	73.00	2.00	2.00	0.
18.00	90.00	70.00	2.00	2.00	0.
19.00	91.00	67.00	2.00	2.00	0.
20.00	92.00	64.00	2.00	2.00	0.
21.00	93.00	61.00	2.00	2.00	0.
22.00	94.00	58.00	2.00	2.00	0.
23.00	95.00	55.00	2.00	2.00	0.
0.	96.00	52.00	2.00	2.00	0.
1.00	97.00	49.00	2.00	2.00	0.
2.00	98.00	46.00	2.00	2.00	0.
3.00	99.00	43.00	2.00	2.00	0.
4.00	100.00	40.00	2.00	2.00	0.
5.00	101.00	37.00	2.00	2.00	0.
6.00	102.00	34.00	2.00	2.00	0.
7.00	103.00	31.00	2.00	2.00	0.
8.00	104.00	28.00	2.00	2.00	0.
9.00	105.00	25.00	2.00	2.00	0.
10.00	106.00	22.00	2.00	2.00	0.
11.00	107.00	19.00	2.00	2.00	0.
12.00	108.00	16.00	2.00	2.00	0.
13.00	109.00	13.00	2.00	2.00	0.
14.00	110.00	10.00	2.00	2.00	0.
15.00	111.00	7.00	2.00	2.00	0.
16.00	112.00	4.00	2.00	2.00	0.
17.00	113.00	1.00	2.00	2.00	0.
18.00	114.00	0.00	2.00	2.00	0.
19.00	115.00	0.00	2.00	2.00	0.
20.00	116.00	0.00	2.00	2.00	0.
21.00	117.00	0.00	2.00	2.00	0.
22.00	118.00	0.00	2.00	2.00	0.
23.00	119.00	0.00	2.00	2.00	0.
0.	120.00	0.00	2.00	2.00	0.
1.00	121.00	0.00	2.00	2.00	0.
2.00	122.00	0.00	2.00	2.00	0.
3.00	123.00	0.00	2.00	2.00	0.
4.00	124.00	0.00	2.00	2.00	0.
5.00	125.00	0.00	2.00	2.00	0.
6.00	126.00	0.00	2.00	2.00	0.
7.00	127.00	0.00	2.00	2.00	0.
8.00	128.00	0.00	2.00	2.00	0.
9.00	129.00	0.00	2.00	2.00	0.
10.00	130.00	0.00	2.00	2.00	0.
11.00	131.00	0.00	2.00	2.00	0.
12.00	132.00	0.00	2.00	2.00	0.
13.00	133.00	0.00	2.00	2.00	0.
14.00	134.00	0.00	2.00	2.00	0.
15.00	135.00	0.00	2.00	2.00	0.
16.00	136.00	0.00	2.00	2.00	0.
17.00	137.00	0.00	2.00	2.00	0.
18.00	138.00	0.00	2.00	2.00	0.
19.00	139.00	0.00	2.00	2.00	0.
20.00	140.00	0.00	2.00	2.00	0.
21.00	141.00	0.00	2.00	2.00	0.
22.00	142.00	0.00	2.00	2.00	0.
23.00	143.00	0.00	2.00	2.00	0.
0.	144.00	0.00	2.00	2.00	0.
1.00	145.00	0.00	2.00	2.00	0.
2.00	146.00	0.00	2.00	2.00	0.
3.00	147.00	0.00	2.00	2.00	0.
4.00	148.00	0.00	2.00	2.00	0.
5.00	149.00	0.00	2.00	2.00	0.
6.00	150.00	0.00	2.00	2.00	0.
7.00	151.00	0.00	2.00	2.00	0.
8.00	152.00	0.00	2.00	2.00	0.
9.00	153.00	0.00	2.00	2.00	0.
10.00	154.00	0.00	2.00	2.00	0.
11.00	155.00	0.00	2.00	2.00	0.
12.00	156.00	0.00	2.00	2.00	0.
13.00	157.00	0.00	2.00	2.00	0.
14.00	158.00	0.00	2.00	2.00	0.
15.00	159.00	0.00	2.00	2.00	0.
16.00	160.00	0.00	2.00	2.00	0.
17.00	161.00	0.00	2.00	2.00	0.
18.00	162.00	0.00	2.00	2.00	0.
19.00	163.00	0.00	2.00	2.00	0.
20.00	164.00	0.00	2.00	2.00	0.
21.00	165.00	0.00	2.00	2.00	0.
22.00	166.00	0.00	2.00	2.00	0.
23.00	167.00	0.00	2.00	2.00	0.
0.	168.00	0.00	2.00	2.00	0.
1.00	169.00	0.00	2.00	2.00	0.
2.00	170.00	0.00	2.00	2.00	0.
3.00	171.00	0.00	2.00	2.00	0.
4.00	172.00	0.00	2.00	2.00	0.
5.00	173.00	0.00	2.00	2.00	0.
6.00	174.00	0.00	2.00	2.00	0.
7.00	175.00	0.00	2.00	2.	



OBSEVED HYDROGRAPH AND RAINFALL

CATCHMENT NO. 1007
EVENT NO. 11
DATE 14/SEP/1988
AREA 403.30 SQ.KM

INDIVIDUAL RAINGAUGE
PROFILE(S)

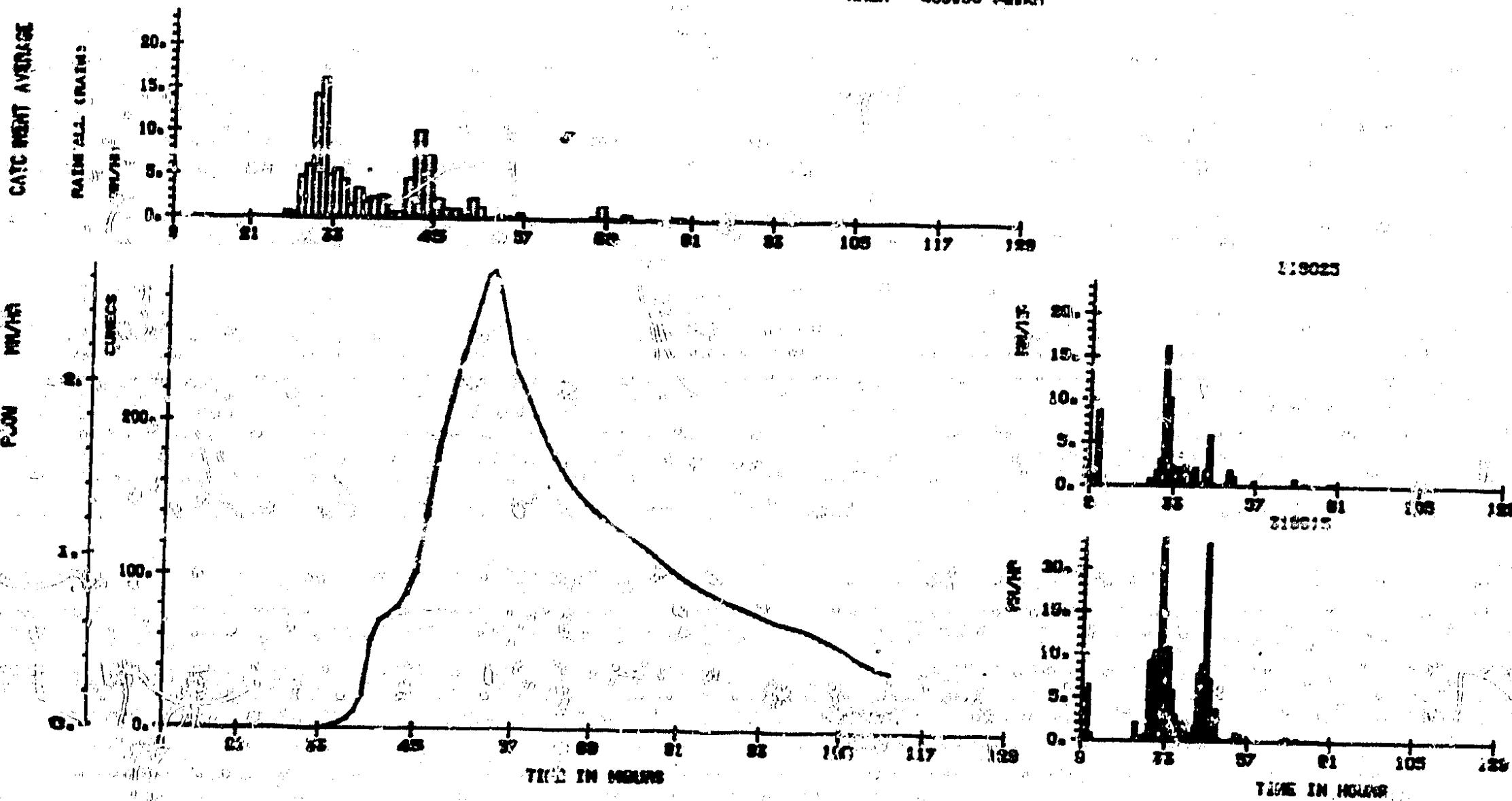


FIGURE D.2

FIGURE B.3

INSTITUTE OF HYDROLOGY
FLOOD EVENT DATA ARCHIVE

CATCHMENT NAME: LULWELL PARK MM 10
CATCHMENT NUMBER: 10007
NUMBER OF EVENTS: 13

DATE OF FORM: 21-04-94
INDICATE LATEST ENTRY

CATCHMENT DESCRIPTORS FROM FILE CATCHERS
ENTERED: 19-6-94

START OF RECORD NOT KNOWN

END OF RECORD CURRENT

SPUD REF OF RECORDING STATION: 5037 7 10

SPUD REF OF CATCHMENT CENTROID: 5037 7 10

CATCHMENT AREA: 403.90 MM 10

MAX STREAM LENGTH: 66.77 KM

ACTUAL LENGTH: 51.14 KM

ALTITUDE OF STATION: 140.4

ALTITUDE OF WATERSHED: 271.0 KM

MAXIMUM ALTITUDE: 296.0 KM

USGS SLOPE (SL1095): 1.89 KM/KM

TAYLOR-SCHWARTZ SLOPE: 1.95 KM/KM

NASH OVERLAND SLOPE: 4.7 KM/KM

DRY VALLEY FACTOR: 0.013

INTER QUAD USED FOR RAINGAUGE SITE PONES:
5075 1105
5275 1205
5155 1403
5075 1707

FLOW STATISTICS

AVERAGE DAILY FLOW: CUMECES ANNUAL AVERAGE RAINFALL: 755.0 MM
MEAN ANNUAL FLOOD (FSK41F): 2.0 CUMECES R50D: 31.7 MM
MAP FROM ANNUAL MAXIMA: 94.9 CUMECES: 15-20
TEN YEAR FLOOD (RECORDED): 126.0 CUMECES: EST MAX 2HR RAINFALL: 173.0 MM
MAXIMUM RECORDED FLOOD: 291.0 CUMECES: EST MAX 24HR RAINFALL: 230.0 MM
YEAR OF BIGGEST FLOOD: 1967 JENKINSON R: 0.350

BASE FLOW INDEX
RESPONSE FLOW INDEX

SOILS

WRAP SOIL INDEX: 0.44
TYPE 1 FRACTION: 0.04
TYPE 2 FRACTION: 0.
TYPE 3 FRACTION: 0.
TYPE 4 FRACTION: 0.24
TYPE 5 FRACTION: 0.

URBAN FRACTION: 0.010
LAKE INDEX: 0.06
STREAM FREQUENCY: 1.55 JUNCTIONS/SC KM
SHAPE FACTOR: 1.223

RAINFALL INDICES

SMDAR
V.20

LIST OF DAILY RAINGAUGES

ENTERED: 23-6-94

RAINGAUGE NUMBER	GRID REF	PERIOD OF RECORD	ON/OFF
250057	EAST NORTH	1979-1984	OFF
250229	5040 1300	1984-1990	OFF
252263	5076 1307	1984-1991	OFF
252250	5061 1392	1984-1991	OFF
252232	5095 1405	1984-1991	OFF
252233	5113 1441	1984-1991	OFF
310053	5233 1236	1984-1991	OFF
314200	5124 1265	1984-1991	OFF
314235	5213 1271	1984-1991	OFF
316065	5255 1319	1984-1991	ON
316153	5225 1326	1984-1991	ON
316213	5165 1309	1984-1991	ON
316220	5117 1301	1984-1991	ON
316232	5174 1317	1984-1991	ON
316232	5102 1304	1984-1991	ON
316233	5160 1302	1984-1991	ON
316230	5176 1387	1984-1991	ON
316261	5190 1363	1984-1991	ON
316343	5180 1321	1984-1991	ON
316347	5176 1321	1984-1991	ON
316484	5141 1-05	1984-1991	ON
316524	5146 1397	1984-1991	ON
316602	5113 1321	1984-1991	ON
316650	5092 1331	1984-1991	ON
316653	5042 1361	1984-1991	ON
316597	4954 1353	1984-1991	ON
316720	4978 1310	1984-1991	ON
316744	5003 1371	1984-1991	ON
3170713	5016 1342	1984-1991	ON
317084	5040 1315	1984-1991	ON
3171305	5007 1269	1984-1991	ON
317220	4923 1292	1984-1991	ON
317238	4940 1315	1984-1991	ON
317241	4927 1293	1984-1991	ON
317334	4990 1273	1984-1991	ON
317454	5015 1270	1984-1991	ON
317550	5007 1222	1984-1991	ON
317551	5053 1227	1984-1991	ON
315541	4882 1311	1984-1991	OFF
315575	4954 1275	1984-1991	OFF

LIST OF RECORDING RAINGAUGES

ENTERED: 23-6-94

RAINGAUGE NUMBER	GRID REF	PERIOD OF RECORD	ON/OFF
281057	4979 1325	1984-1991	OFF
292234	5041 1392	1984-1991	OFF
310233	5160 1302	SEP 1984-FEB 1991	ON
310343	5180 1321	JUL 1984-JUN 1991	ON
310454	5141 1-05	APR 1984-FEB 1991	ON
310524	5146 1397	APR 1984-JUN 1991	ON

LIST OF EVENT DATA

EVENT ENTRY NUMBER	QUALITY CODE	FLOW DATA	RAIN DATA	SNOW DATA	API DATA	COMMENTS	RAWRAIN DATA	NEW PATH DATA	NEW API DATA	FS RESULT	EVENT DATE
1	11	6312144	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	NO DATE UNSATISFACTORY
2	11	12122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	3 DEC 1990 SNOW
3	11	52121	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	27 JAN 1991 SNOW
4	11	11121	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	27 FEB 1991 SNOW
5	11	11122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	3 MAY 1991 SNOW
6	11	11121	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	16 NOV 1990 SNOW
7	11	11122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	13 APR 1990 SNOW
8	11	21122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	28 NOV 1990 SNOW
9	11	11122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	31 MAY 1990 VARIABLE RAIN
10	11	11121	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	22 OCT 1990 VARIABLE RAIN
11	11	23122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	14 SEP 1990 VARIABLE RAIN
12	11	21121	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	15 DEC 1990 VARIABLE RAIN
13	11	23122	23-6-92	21-6-92	21-6-92	23-6-92	23-6-92	NO ENTRY	NO ENTRY	NO ENTRY	11 APR 1990 VARIABLE RAIN

EVENT BREAKDOWN :-

NO ENTRY: 7
SNOW: 7
VARIABLE RAIN: 7
UNINTERESTING: 1
UNSATISFACTORY: 1
SNOW: 1

APPENDIX C

A LOCALISED REINTERPRETATION OF THE WRAP MAP

During the preliminary analysis of the percentage runoff data a group of catchments stood out as having very much larger percentage runoffs than would be expected from their SOIL classification. One catchment, located entirely on type 1 soil was giving percentage runoffs well over 50% (the highest being 75%). The catchments all fell partly or wholly on a single soil association. While this soil is correctly classed as type 1 by the scheme given in FSK I.4.2.3 (see also Farquharson et al, 1978), there is evidence from the current study and elsewhere (Gustard, 1981) that its hydrological response is as expected from a type 5 soil. Table C.1 lists the nine catchments occurring on this soil association and gives two measures of their hydrological response; percentage runoff and base flow index. Reinterpreting this soil as type 5 gives a consistent improvement across all the catchments and so it is recommended that the soil map should be amended accordingly. Figure C.1 shows the newly interpreted WRAP map for this area.

TABLE C.1 PERCENTAGE RUNOFF, BFI AND SOIL VALUES FOR ANOMALOUS CATCHMENTS

Catchment	Percentage Runoff	BFI	Soil type 1 (FSR)	Soil type (FSR)	SPR from soils (now)
76014	30-80	0.25	0.51	29.9	52.0
71802	60-80	0.26	0.09	45.8	53.0
72006	50-80	-	0.21	40.7	53.0
76805	50-70	0.27	1.00	15.0	53.0
27034	50-70	0.34	0.03	45.4	49.8
27027	30-90	0.38	0.16	40.1	52.7
76005	65	0.39	0.48	30.3	51.2
27035	23-50	0.43	0.12	41.5	49.8
73008	20-50	0.50	0.08	27.5	31.8

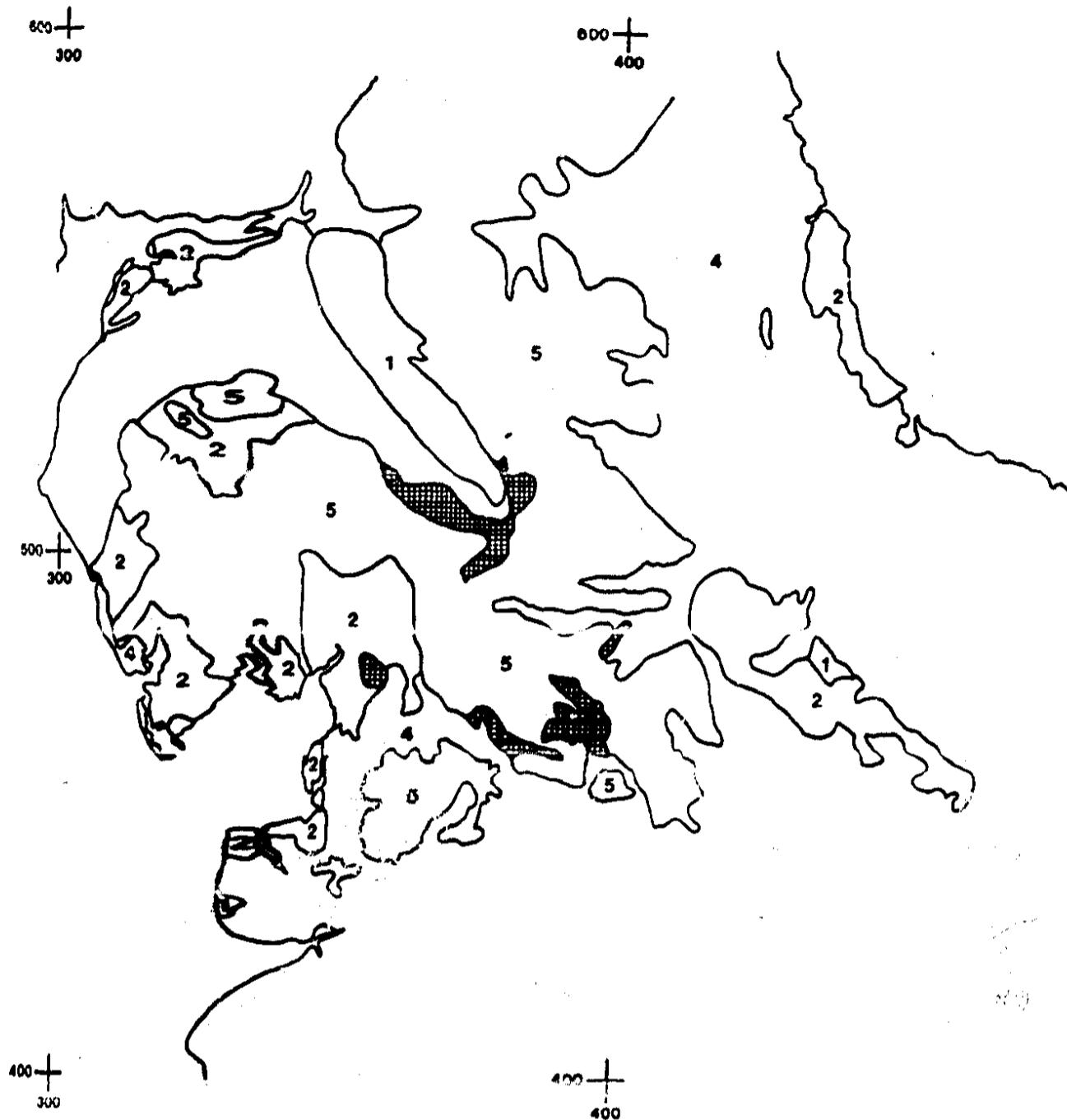


FIGURE C.1 Areas of Soil 1 reinterpreted as Soil 5

It is important to appreciate that such an interpretation of the WRAP map requires careful consideration of the distribution of the soil association under examination, and the veracity and extent of relevant hydrological data. Users of the estimation procedures are advised against making such a reassessment themselves but should use local data to refine FSR estimates as described in Section 4.

References

- Farquharson, F.A.-K., Mackney, D., Newson, M.D. and Thomasson, A.J. 1978 'Estimation of runoff potential of river catchments from soil surveys'. Special Survey No. 11. Soil Survey of England & Wales.
- Gustard, A. 1981 'The hydrological response of two upland catchments: implications for flood estimation' PhD Thesis, Lancaster University.